CITY OF RAYMORE

100 Municipal Circle · Raymore, MO. 64083 Phone · 816-892-3045 · Fax · 816-892-3093



ADDENDUM NO. 4 Hawk Ridge Park Improvements Project #18-253-201

All plan holders are hereby notified and agree by signature below, that the bid includes consideration of the following changes, amendments, and/or clarifications and costs associated with these changes and are included in the bid.

Addendum No. 4 - Questions and Clarifications, Geotech Report, Revised Bid Tab

A. The Owner will purchase the pre-fab bathroom and will be delivered to the site. Contractor is responsible for stubs, connections, water, electricity to the building.

- 1. Who is responsible for the road to the site for the building? Answer: Contractor
- 2. Is the contractor responsible for Johnston Drive? Answer: No
- **3. When is the building anticipated to arrive? Answer:** The building will be ordered by the City. Date of delivery should be available when the contractor is selected.
- 4. What is the allowable time for unloading and waiting time? Answer:
- 5. Who is responsible for the crane rental?Answer: The owner will purchase the building as a package, delivery with crane will be included.

B. North Playground trails. Color coded drawing to show the trail details is attached. A heater will be added to one of the bathrooms. The interiro walls have been changed to prevent air from coming in. There is a requirement that the contractor install a vent according to code.

6. The interior walls are not noted on the drawings. Answer: Interior walls are detailed on sheet A220

C. Regarding the Amphitheater, the contractor will need to remove unsuitable fill and must be replaced by engineered fill.

- 7. Will the fill be required just under the footings? Answer: Yes.
- 8. Will bearing capacity be needed under the entire Amphitheater? Answer: It is estimated that the North corner, ¹/₃ of the amphitheater will need to get sound material. Fly ash/rock stabilization will be needed under the building only whee unsuitable material is present.
- 9. Can the limestone seating and retaining wall use random lengths of limestone?

Answer: Yes as long as they are uniform, stable, and should not move at all.

D. There will be a concrete sidewalk connecting the existing sidewalk from Johnston Drive.

E. Please clarify the Bid Tab as there is confusion to the Asphalt listed for the N. Playground.

10. The bid tab lists APWA Type 3 4" and APWA Type 1 2". Then on a separate line it lists APWA Type 1 2". Why?

Answer: These are two separate line items. The first item is for the full parking lot (4" base, 2" surface). The second line Type 1 2" is for the asphalt paving around the parking lot. The bid tab has been coreected and a revised one is in the addendum.

- **11. Is recycled asphalt allowed for the base? Answer:** Yes. 30% maximum for base only.
- 12. Are there specifications for Soding slopes? There is no item on the bid tab for Sod.

Answer: No, it is a seed only project, with seed blanket if we exceed 6:1 slope. **13. Any 4-1 or greater slopes?**

- **Answer:** There should not be any slopes that exceed the 6:1 point in the project.
- **14.** For site excavation, is the cut quanity listed? **Answer:** Yes, in the bid tab.
- **15.** Are there fill quantities listed? **Answer:** No. Make sure it meets Engineering fill requirements where indicated.
- **16.** Is there a place to put the excess dirt? **Answer:** Yes, on the west side of the park.
- **17.** The bid quantities are as listed on the bid tabs? **Answer:** Yes.
- **18. Will all base bids be taken into consideration? Answer:** Yes. One contractor will be selected.
- **19. Erosion control is not listed under #6.Answer:** Erosion control has been added to amphitheater.
- 20. Is testing for the Engineering fill required?Answer: Yes. Contractor must have Geotech on site for testing and approval.
- 21. All testing is on the General Contractor? Answer: Yes.
- 22. Is there special testing for borings, foundations, and soil borings? Answer: Yes.
- 23. When is the anticipated NTP? Answer: July 2018
- **24.** Can the completion date be changed to one single date? Answer: Yes. The date of completion will be December 31, 2018.
- **25. Will there be a problem with delay in the biological plantings? Answer:** No as that is weather permitted.

26. Having problems obtaining fees from KCPL.Answer: Fees will be paid by the owner and the bid item will be removed from bid tab. Owner will coordinate work with contractor.

27. Is the sub-structure metal?

Answer: Galvinized steel

28. Can the lake be lowered?

Answer: The whole lake can NOT be lowered; however, lowering the lake is permittable in work zone only.

29. Helical piers

Answer: The City has provided design criteria to be able to bid the helico piers. The City will get shop drawing with the selected contractors vendor with final structural numbers for the loading. The piers can be installed in the lake without draining the lake or making any provision for the water. They can be installed with extended crane from the shore or installed pier by pier as you build out into the water.

30. Is the parking lot still light and heavy duty pavements?

Answer: The parking lot is now all one design, heavy duty.

31. The revised bid form "X"ed out the Montrose Building line item since the City is providing the building. According to the specifications I am to prepare a gravel sub base for the building that is a minimum of 6"thickness of $\frac{3}{4}$ " granular fill. I think we either need to add a line item for this work or eliminate

the "X"'s off the bid form so I can put the cost for the base on this line item. I will also need to put in the cost for a temporary road to get the crane and truck access to the site and this cost would go on the same line item.

Answer: An additional line item has been added for the AB3 sub base for the building. The access to the Montrose site is the road and parking lot. The weight of one half of the building is approximately 76,000 pounds and the second half is 67,000 pounds.

- F. Asphalt Location Map attached.
- G. Geotech Report attached.
- H. Second Revised Bid Proposal Form E 4/27/18 attached.
- I. Second Revised Appendex A 04/27/18 attached

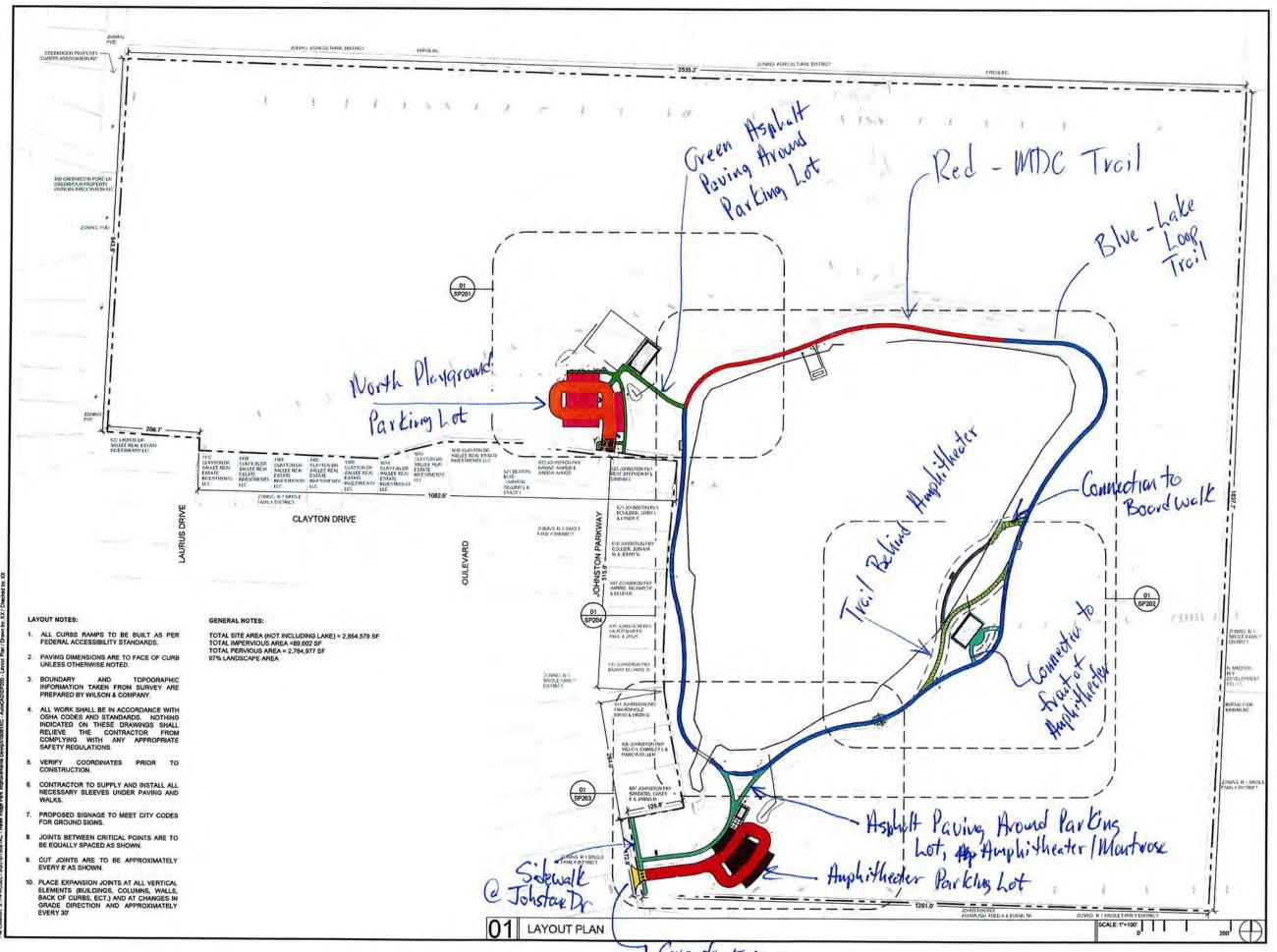
J. Full set of plans attached.

Any other questions regarding this proposal shall be submitted to Kim Quade, CPPB by e-mail at kquade@raymore.com or by phone at (816) 892-3045. There will be no questions allowed after May 4, 2018 at 5 p.m.

I hereby certify that the above have been considered and associated costs have been included in this bid.

Company Name:	
By:	
Title:	
	Phone: er:

ADDENDUM MUST BE SUBMITTED WITH BID



2 Concrete Entrance

CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER. STE 200 KANSAS CITY, MISSOURI 64131 PH: 818.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64 108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169





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CONFLUENCE PROJECT NO: 16081KC

SP200



1701 State Avenue Kansas City, KS 66102

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AOGeotech.com

GEOTECHNICAL ENGINEERING REPORT

HAWK RIDGE PARK

JOHNSTON PARKWAY RAYMORE, MISSOURI (AOG 17-304E)

Date:

August 2, 2017

Submitted to: Mr. Hank Moyers, ASLA Confluence 525 17th Street Des Moines, Iowa 50309

Submitted by: ALPHA-OMEGA GEOTECH, INC.

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Appendix A – SITE SKETCH - Site & Boring Location Plans Appendix B – LABORATORY TEST RESULTS Appendix C – BORING LOGS



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August 3, 2017

1701 State Avenue Kansas City, KS 66102

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Mr. Hank Moyers, ASLA Confluence 525 17th Street Des Moines, Iowa 50309

AOGeotech.com

HAWK RIDGE PARK

Johnston Drive, North of Hampton Drive Raymore, Missouri (AOG 17-204E)

Dear Mr. Moyers,

Alpha Omega Geotech, Inc. (AOG) has completed its geotechnical engineering investigation for the above-referenced project.

Attached are the following items that were utilized in the analysis and evaluation of the subsurface conditions at this site: a sketch giving the approximate location of the thirty-one (31) auger borings completed during this investigation with reference to the existing site features; detailed laboratory results of twenty (20) moisture contents (ASTM D2216), twelve (12) dry densities (ASTM D7263), one (1) set of Atterberg limits (ASTM D4318), and five (5) unconfined compression (ASTM D2166) tests; twelve (12) calibrated pocket penetrometer readings; and eight (8) auger boring (ASTM D1452) logs that describe the materials encountered, their approximate thicknesses, and the sampling depths where Shelby tube, thin-walled steel, samplers (ASTM D1587) and Standard Penetration (ASTM D1586) tests were performed.

Representatives of AOG located each of the selected borings by measuring from the existing site features, and these measurements should be considered accurate only to the extent implied by the method of measurement. Elevations were not determined in the field at the time of drilling. Each of the borings was completed by AOG using a CME 55 high-torque drill rig.

1.0 PROJECT AND SITE DESCRIPTION

At the time of this investigation, AOG was provided with a general site plan showing the proposed location of the new park structures. Based on this information, AOG understands the project consists of developing the new Hawk Ridge Park around the perimeter of the existing pond located off Johnston Parkway in Raymore, Missouri. Developing the new park will include construction of two small retaining walls, two signs, two shelters, a walking path and associated paving along the perimeter of the pond. It is anticipated that the structures will be lightly loaded, slab on grade shelter areas.

Currently, the proposed project site has an existing pond surrounded by trees, foliage, and open fields. Residential houses are located along Johnston Parkway which travels parallel with the west side of the pond. No grading plan was available at the time of this investigation, but AOG anticipates the existing grades in the areas of the proposed shelters are within two (2) feet, +/-, of the proposed construction grades.

2.0 SUBSURFACE INVESTIGATION

This subsurface exploration and the services documented, herein, were provided in accordance with the scope of work described in Alpha Omega Geotech's proposal number PW-5850 dated June 30, 2017 and authorized by Mr. Hank Moyers of Confluence.

Based on the information provided by Confluence, LLC and conversations with the same at the time of this exploration, AOG drilled eight (8) auger borings at the proposed site. The borings were advanced to their planned depths, or auger refusal, whichever occurred first. Four (4) borings with planned depths of twenty (20) feet were drilled within the footprints of the proposed shelter structures. Additionally, four (4) borings with planned depths of ten (10) feet were drilled at the approximate location of the proposed signs and retaining walls.

It should be understood that the depth of boring, split-spoon refusal or auger refusal reported, herein, applies to the type of drilling equipment used. As such, it might be possible to extend some of these borings deeper using different drilling equipment and/or techniques. <u>Conversely, residual sandstone, shale and limestone materials through which AOG's drill rig penetrated, without achieving refusal, may be difficult to excavate depending upon the equipment being used.</u> As such, Alpha-Omega Geotech, Inc. shall not be responsible, for the determination of Others, regarding the rippability, or ease of excavation, of the in-situ subgrade, bedrock and/or geo-intermediate materials.

Above the depth at which auger refusal or boring termination occurred, predominantly fat clay soils were encountered in the borings. Thin-walled, steel, Shelby tube samplers (ASTM D1587) were used to collect relatively undisturbed samples from these borings for laboratory analysis. Standard Penetration tests (SPT) (ASTM D1586) were also used to sample and evaluate the consistency of the in-situ subgrade materials encountered in these test borings. Standard Penetration Tests are conducted by advancing a hollow, split spoon sampler into the base of the auger hole by means of dropping a 140-pound hammer a distance of 30 inches onto the drill rods. Each drop of the hammer is one blow, and these blow counts are recorded for each of three, 6-inch advances of the sampler. The first 6-inch advance is the seating drive, and the summation of the blow counts of the final two, 6-inch advances is taken as the standard penetration resistance. The standard



Confluence AOG 17-304E August 3, 2017 HAWK RIDGE PARK Raymore, Missouri Page | 4

penetration resistance, or N-value, as it is known, along with the soil classification, can be used to estimate the density, shear strength and other engineering properties of the materials encountered.

The N-values obtained from each of the SPT's completed in these borings using a CME automatic hammer are included on the boring logs and summarized in the Summary of Laboratory Testing sheet found in Appendix B. Samples retrieved during drilling efforts were returned to AOG's laboratory for testing and evaluation.

3.0 LABORATORY TESTING PROGRAM

Laboratory testing on materials collected during drilling was performed on samples selected by AOG. Results from these tests can be found in Appendix B and on the boring logs in Appendix C. The following laboratory tests were performed by qualified AOG personnel in accordance with ASTM specifications to determine pertinent engineering properties of the soils:

- Visual classification (ASTM D2488)
- Moisture content tests (ASTM D2216)
- Atterberg limits tests (ASTM D4318)
- Dry Unit Weight (ASTM D7263)
- Unconfined compression tests on soil (ASTM D2166)

The dry unit weights of specimens cut from the Shelby tube samples were found to be moderate, ranging from 83.3 pounds per cubic foot (pcf) to 110.5 pcf. Depending upon the material composition and depth below existing grade, the moisture content of the specimens cut from these tube samples ranged from 14.1 to 32.1 percent. The unconfined compressive strength of the specimen cut from the Shelby tube sample ranged from 1128 to 9046 pounds per square foot (psf). Calibrated pocket penetrometer readings ranging from 1.0 tons per square foot (tsf) (2000 psf) to >4.5 tsf (9000 psf) were obtained on the recovered Shelby tube samples. However, it should be noted that the pocket penetrometer values tend to over-estimate the strength of insitu subgrade materials relative to the actual unconfined compressive strength test.

The Atterberg consistency limits were determined for one, generally, representative sample taken at a relatively shallow depth from within the proposed northwest shelter footprint. Based on the Atterberg limits, the sample was classified in accordance with the Unified Soil Classification System (USCS) as a fat clay, i.e. CH classification materials. The results of these laboratory analyses are presented in the following table:

Table #1: Atterberg Limits Results

		AITTERBE	RGUIMIN	an sister	
Sample	Depth (ft)	Liquid Limit	Plastic Limit	Plasticity Index	USCS Classification
B2 ST-1	2-4	53	24	29	FAT CLAY (CH)

Based on the Atterberg limits, it is anticipated the majority of the onsite soil materials generally possess a high swelling potential. The swelling potential of a clay soil is an indication of the volume changes that may take place with variations in the soil moisture content.



Confluence AOG 17-304E August 3, 2017

Except for the samples for which the Atterberg limits were determined, all of the other soil classifications given throughout the laboratory test data, as well as, the boring logs, were made using the visual and tactile techniques described in ASTM D2488. As a result, additional analyses could reveal other soil types of different classification and potentially higher plasticity and swelling potential both onsite and within the nearby vicinity.

4.0 GROUNDWATER

Free water at depths ranging from nine (9) to eighteen (18) feet beneath existing grade (fbeg) was observed in Borings B1, B2, and B8 at the time of drilling. The remained borings remained dry and no free water was observed at the time of drilling. However, a twenty-four-hour water level was not established in any of the borings due to time restrictions, as well as, potential safety hazards associated with open bore holes.

Although the ground water levels given on the boring logs reflect the conditions observed at the time the borings were made, they should not be construed to represent an accurate or permanent condition. There is uncertainty involved with short-term water level observations in bore holes especially in clay soils of relatively low permeability. The groundwater level should be expected to fluctuate with variations in precipitation, site grading and drainage conditions. In addition, it is also possible that seasonal perched ground water may be encountered within these soil deposits and bedrock formations at different depths during other times of the year based on drainage conditions, seasonal snowmelt and rainwater infiltration.

5.0 GEOTECHNICAL CONSIDERATIONS

The following considerations are given based on observations made by AOG at the time of drilling, during reconnaissance trips, and based on the project requirements and description as stated above:

- 1) Undocumented Fill: Undocumented fill, in general, consists of foreign materials with unknown densities and consistencies. Undocumented fill is unsuitable beneath structures unless measures are taken to stabilize the materials prior to loading. Undocumented fill beneath foundations and slabs should be addressed in accordance with Section 6.3, "Undocumented Fill," this report.
- Expansive Materials: Expansive clays were encountered during this exploration. Expansive clays are known to experience significant volume changes with changed in moisture. Expansive clays located beneath any slabs on grade should be removed in accordance with Section 8.0, "SLABS ON GRADE," of this report.
- 3) Compressible Soils: During this exploration, compressible soils with strain rates in excess of 10% were observed in Boring B4, which is located within the footprint of the southeast shelter. Any soft, compressible areas identified on the proposed project site must be corrected in accordance with Section 6.1, "Site Preparation," of this report.



6.0 SITE DEVELOPMENT

6.1 Site Preparation

Based on the information provided, AOG anticipates minimal amounts of cut and fill, less than two (2) feet from the current elevation, within the proposed structure footprint, will be required to achieve finish floor elevations. It is possible that additional cuts and fills may be required to obtain improved surface drainage.

Appropriate erosion control measures, such as proper site contouring during grading activities, as well as, silt fences, should be maintained to help keep any eroded materials **o**nsite.

Within the footprint of the proposed new structure and associated paving, it is recommended that any topsoil, vegetation, utility backfill, and other deleterious material (i.e. concrete slabs, relic foundations, utilities, etc.) or pavements should be stripped and removed prior to the placement of any fill required to achieve the finished floor elevation. In accordance with the local building code, this should be verified by a representative of Alpha-Omega Geotech, Inc. prior to the placement of fill.

Once initial site stripping operations have been completed and prior to the placement of any engineered fill in this area, it is recommended that the exposed subgrade be moisture conditioned and recompacted, as needed, and be thoroughly evaluated by means of a proof-roll with a fully loaded, tandem-axle dump truck to locate any soft, compressible areas within the proposed project site. <u>Any soft, compressible areas identified on the proposed project site must be corrected by over-excavation to a suitable subgrade and replaced with an acceptable material.</u> Compressible soils with strain rates over 10% were observed in Boring B4, which is located within the boundaries of the southeast shelter footprint. This compressible zone is, likely, the result of a combination of the material composition and a, relatively, high moisture content. Some material removal and moisture conditioning should be anticipated during construction.

Although it is not anticipated that any extensive removal and replacement would be necessary, it is likely that some effort may be required to develop a stable platform on which to place the necessary fill material and address any other existing site conditions that become known during construction. It is generally anticipated that the extent of these efforts would strongly depend upon the ground moisture conditions at the time the site work begins. In the event that the ground is generally dry, it is possible that only a minimal amount of stabilization would be required, which may be possible to accomplish by simple moisture conditioning and recompaction efforts. <u>Nevertheless, it is recommended that a representative of Alpha-Omega Geotech, Inc. should be onsite to witness this proof-rolling and offer recommendations, as needed, to correct any problem areas identified.</u>

6.2 Undocumented Fili

Undocumented fill is a foreign material, of which no records of testing or evaluation by a qualified professional during the time of placement exist. The risks associated with supporting foundations and floor slabs on undocumented fill include total and differential settlements in excess of tolerable limits. Undocumented fill was encountered in the upper 8.5 feet of the planned northwest shelter and in the upper four (4) feet of the planned southeast shelter during this exploration. If undocumented fill is encountered during construction, it should be addressed in accordance with this report.



HAWK RIDGE PARK

Raymore, Missouri

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Undocumented fill is, generally, unsuitable beneath structures and pavements, and, if encountered during development, should be completely removed and replaced with engineered fill.

Partial fill remediation in pavement areas can be considered with an increased risk, which is accepted by the owner, for pavement distress. If partial fill remediation is desirable, AOG can provide additional recommendations based on observations at the time of construction.

6.3 Engineered Fill Placement

It is assumed that any fill material needed will come from cut areas and, if necessary, on-site or nearby borrow sources of similar material. It is recommended that unweathered shales should NOT be used to construct any of the necessary fill within either the new building or paved portions of the site. Assuming they are properly moisture conditioned and compacted, it generally appears that the clean clay soils encountered in the borings that are free of rubble, trash, concrete, asphalt, and other debris would be acceptable for use as controlled fill. However, due to their very high swelling potential, detailed recommendations for the placement of a non-expansive subbase are provided in Section 8.0, SLABS ON GRADE of this report.

Any imported fill materials for use as structural fill should be tested by Alpha-Omega Geotech, Inc. to determine if they are acceptable for the intended use. Any ground water seeps that are encountered must be diverted prior to placing fill.

In addition, no compaction of soil fill material should be performed during freezing weather. Nevertheless, as weather conditions dictate, it may be possible to substitute crusher-run limestone in lieu of soil fill to allow placement of engineered controlled fill material to continue during the cold fall and winter months. However, any frozen fill material must be stripped prior to placing subsequent lifts.

All general fill within the area of the new structures (except for the upper 22-inches, as discussed in Section 8.0, SLABS ON GRADE of this report, should be placed in lifts not exceeding 6 inches in thickness, and compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content.

As required by the local building code, the compaction of any structural fill beneath the new buildings, pavements, and any other areas where settlement control is necessary, as well as, any slopes that are steeper than 4:1 (H:V) should be tested lift-by-lift by a representative of Alpha-Omega Geotech, Inc.

6.4 Drainage Considerations

Fluctuations of the ground water level can occur due to seasonal variations in the amount of rainfall and other climatic factors that were not evident at the time the borings were made. The possibility of ground water level fluctuations should be considered when developing the design and construction plans for the project. In spring and late fall, soil moisture contents may be abnormally high and drying of the soils that are exposed and/or undercutting may be required to develop a suitable base for the placement and compaction of engineered fill. Disking and aeration of the exposed soils may be sufficient to develop a stable base. However, if site grading begins during the summer or early fall, moisture contents may be abnormally low and the plastic clay soils encountered during this exploration may undergo significant volume changes with subsequent.

increases in their moisture content. Therefore, when these conditions exist, disking and moisture conditioning of the exposed subgrade soils may be required.

It is important to consider drainage and construction elements that will help to inhibit future slab on grade problems, foundation cracks, as well as, intolerable settlements due to volume changes of the onsite soils. The surface drainage must be designed to prevent ponding and effectively move water away from both the new and existing buildings, pavements and other structures. It is also very important to place all materials under carefully controlled conditions of moisture and density to inhibit significant soil volume changes. Shrubs and trees with deep root systems and requiring large quantities of water should not be planted within 20 feet of the building lines. Any planters located near the building should have impermeable bases with weep holes to discharge water away from the wall lines. Down spouts should be connected to subsurface drains to carry the water to safe exits beyond the building lines, retaining walls, pavements, slopes and other site features or structures that could be adversely affected by water seepage.

In addition to controlling surface drainage, it is recommended that a gravity drainage system, such as a French drain or similar, designed to intercept free water prior to contact with foundations be installed in areas where the topography will direct water toward the proposed structure. foundation drainage systems should, also, be considered to prevent any free water accumulation and/or ingress at the foundations where shallow groundwater was encountered. Any basement or below grade slabs should have a permanent dewatering system, such as a sump pump or similar type system, installed to alleviate and water accumulation.

6.5 General

Permanent slopes should not be steeper than 3:1 (H:V) to help ensure their future stability and accommodate normal mowing equipment. The responsibility for excavation safety and stability of temporary construction slopes should lie solely with the contractor and should follow the OSHA regulations given in 29 CFR Part 1926.650 - .652, Subpart P. The stability of open excavations is dependent upon a number of factors including but not limited to the presence of gravel, sand and/or silt seams, ground water seepage, strength characteristics of the soil layers, slickensides and other unique geological features, the slope and height of the cut, surcharge loading and vibrations during construction, weather conditions, as well as, the length of time the excavation is left open. Alpha-Omega Geotech, Inc. does not assume any responsibility for construction site safety or the contractor's or other parties' compliance with all local, state and federal safety or other regulations including imprudent excavating practices that results in any damage to nearby structures, roadways, utilities, as well as, onsite or offsite improvements.

7.0 FOUNDATIONS

7.1 Foundation Recommendations

Based on the finding during this geotechnical exploration and AOG's understanding the proposed project, it is AOG's opinion that a shallow foundation system consisting of either earth-formed trench or spread footings may be used as economical foundation elements for the proposed structures. Footings should bear on native clay soils and/or engineered controlled fill. Undocumented fill observed in the areas of the proposed shelters should be removed in accordance with Section 6.3, "Undocumented Fill," of this report.



Based on the subsurface conditions that have been identified, Site Class C conditions (IBC 2012) may be assumed for seismic consideration.

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Perimeter footings, and any footings in unheated areas, should be placed at least 3 feet below final exterior grade to provide adequate frost protection and place them in a more stable moisture environment. Under heated areas, the interior footings can be founded at shallower depths of at least 18 inches below the finished floor elevation. The footing excavations should be carried to undisturbed, inorganic soil or engineered fill.

7.2 Allowable Bearing Pressure

Provided all design and inspection recommendations as given in this report are closely followed and good construction practices are exercised, it is recommended an allowable bearing value of 2,500 psf may be used for design purposes to proportion the spread/wall footings. A twenty-percent increase, i.e. 3,000 psf, may be used for individual column footings. These allowable bearing capacity values, which are based on shear strength alone and not on settlement, incorporate a factor of safety of 3.0. <u>The actual bearing capacity of all subgrade supporting the foundation elements must be confirmed by a representative of Alpha-Omega Geotech, Inc. as the excavations for the load-bearing wall and column footings are completed and prior to placement of reinforcing steel and concrete. For transient loading conditions, such as unsustained wind and earthquake, a 33 percent increase may be applied to the above-referenced allowable bearing capacity values.</u>

7.3 Anticipated Settlement

Uniform bearing conditions should be provided beneath the footings to minimize differential settlements. If any soft or otherwise unsuitable material, including undocumented fill, is encountered in the footing excavations, it will have to be removed and replaced with engineered controlled fill. Recommendations for the over-excavation and replacement with engineered controlled fill can be made when the footing excavations are inspected during construction, if needed. <u>A representative of Alpha-Omega Geotech, Inc.</u> should inspect all of the footing excavations to verify that uniform and competent bearing material is present beneath all of the foundation elements prior to the placement of any reinforcing steel and concrete.

For spread footings designed and constructed in accordance with this report, it is anticipated that settlements will be limited to 0.5 inches of differential and .75 inches in total.

7.4 General

If possible, the over-dug footing excavations should not be left open for more than 24 hours to help reduce excessive sloughing, softening or drying of the exposed subgrade material. The base of the footing excavations should be free of water and loose soil prior to placing reinforcing steel and concrete. No groundwater is expected in the footing excavations since groundwater was not encountered in any of the borings that were made at the time of drilling. However, if groundwater is encountered within the expected depth of excavation for the footings, it is anticipated that it can be removed by the use of sumps and pumps. Based on the subsurface conditions that have been identified, it is anticipated that earth-formed trench footing excavations may be used effectively on this project. A minimum width of 12 inches should be used for trenched wall footings to allow for steel placement and inspection. Minimum widths of 16 and 24 inches should be used for formed wall and column footings, respectively.



8.1 Slab Thicknesses

Slabs on grade that will be subjected to repeated wheel loads, such as passenger vehicles, should be at least 6 inches in thickness. Slabs that are <u>not</u> exposed to repeated wheel loads, should be at least 4 inches in thickness. Slabs in storage areas may need to be thicker due to shelving post and other concentrated floor loads.

8.2 Low Volume Change (LVC)

The following recommendations provided to help protect the slabs from damage caused by volume changes within the underlying subgrade, and should be implemented in conjunction with Section 7.0, FOUNDATIONS of this report:

- 1) Cut the subgrade a minimum of 22-inches beneath the base of slab elevation to allow placement of an 18-inch subbase and a 4-inch base course beneath the slab-on-grade.
- Scarify and recompact the upper 9 inches of exposed subgrade to within 95 to 100 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content wet of the optimum moisture content 0 to 3 percent.
- 3) For the 18-inch granular subbase, place crusher-run limestone or rock dust in two (2), 9-inch lifts and compact to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density. The moisture content of this material at the time of placement must be sufficient to achieve the specified level of compaction.
- 4) Place a 4-inch base course of clean, open-graded crushed limestone. This granular base course should be compacted with a suitable vibratory steel wheel roller.

Alternatively, it would be possible to consider constructing the 22-inch subbase by stabilizing the onsite fat clay soil material with Type C flyash, blended at 15 percent by weight using a large Bomag Tiller. However, due to the amount of dust that is generated, the use of flyash stabilization may not be a viable alternative for this project site. In addition, it should also be noted that flyash stabilization is, generally, only effective when the ground temperature is a minimum of 50° to 60°F. Nevertheless, if this alternative is utilized, the flyash stabilized subbase should be placed in two (2), 9-inch lifts and compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content. Compaction of the flyash-supplemented soil should be completed within one hour after incorporation. Additional compaction after two hours could cause degradation of the soil strength. Please note, when constructing in areas where fat clays are present, the owner should recognize there is an inherent risk of distress associated with volume changes of the soil, even with subgrade removal and/or treatment.

8.3 General

It is recommended that under-slab utility trenches should be backfilled with impermeable clay soil (*), flowable fill or lean concrete to help reduce the potential of these trenches acting as aqueducts transmitting groundwater beneath the new building, pavements, retaining walls and other structures.



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(*) If impermeable clay soil is used as backfill, it should be placed in lifts not exceeding 6 inches in thickness and compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content, which should be verified lift-by-lift during placement by a representative of Alpha-Omega Geotech, Inc. Although clay soil may be less costly than flowable fill or lean concrete, the OSHA excavation safety regulations given in 29 CFR Part 1926.650 - .652, Subpart P must be followed in the event that clay soil is used to backfill any utility trenches.

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Finally, it should be noted that the recommendations given, herein, regarding placement of low-volume change fill to help protect the slabs on grade from volume changes associated with fluctuations within the moisture content of the underlying subgrade materials, would still apply.

Plumbing lines and other water leaks occurring beneath the structure's slab-on-grade floor can induce volume changes within the underlying subgrade materials. Therefore, it is recommended that all water supply and waste water lines should be tested for leaks prior to backfilling the utility trenches. In addition, it is also recommended that every effort should be made to maintain the plumbing in good working order and prevent or minimize water leaks and discharges.

It is assumed the concrete will be reinforced with properly placed steel reinforcement, such as #4 bars, and control joints will be cut during or shortly after finishing (to be designed by the project structural engineer). Properly placed wire mesh may be used as secondary reinforcement. Fiber reinforcement may also be considered to help control shrinkage cracking and the use of other admixtures may be considered to enhance the workability and performance of the concrete. Suitable construction and sawed joints should be used to control cracking of the slab. In addition, it is recommended that the slump and temperature of the concrete at the time of placement should be limited to standard American Concrete Institute (ACI) guidelines. Furthermore, it is also recommended that proper concrete curing techniques should be utilized and the addition of jobsite water to the concrete be avoided or very closely controlled to within acceptable parameters. Nevertheless, it should be noted that cracking of concrete used for slabs on grade is a normal occurrence and should be expected.

If a 18-inch thick subbase layer of crusher-run limestone (AB-3) or rock dust is used, as recommended, a modulus of subgrade reaction of 150 pci may be assumed for reinforcement and thickness design to support surface loads. If a higher modulus of subgrade reaction were desired, we would be pleased to work with the project's structural engineer to develop recommendations for alternate bases and/or subbases to achieve a higher modulus of subgrade reaction.

9.0 EARTH PRESSURE COEFICIENTS

A coefficient of sliding friction over the in-situ clay soils at this site may be taken as 0.32. A minimum factor of safety of 1.5 should be used when considering sliding resistance.

Active, passive and at-rest earth pressure coefficients of 0.25, 4.2 and 0.4 may be assumed for backfills of clean, open-graded crushed limestone.



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Active, passive and at-rest earth pressure coefficients of 0.5, 1.9 and 1.0 may be assumed for the in-situ clay soils at this site.

However, the in-situ soils encountered during this exploration are classified as a Fat Clay and possess a high swelling potential, and, as such, should not be used as backfill since considerable lateral loads may develop with the addition of water.

If deflection of extended foundation walls or retaining walls is not tolerable, as rest earth pressures should be assumed.

These earth pressure coefficients do not include the effect of surcharge loads, hydrostatic loading or a sloping backfill nor do they incorporate a factor of safety. Also, these earth pressure coefficients do not account for high lateral pressures that may result from volume changes when expansive clay soils are used as backfill behind walls with unbalanced fill depths. In addition, any disturbed soils that are relied upon to provide some level of passive resistance should be placed in lifts not exceeding 6 inches in thickness and compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content. It is recommended that a representative of Alpha-Omega Geotech, Inc. should verify the compaction of any such materials relied upon to provide passive pressure lift-by-lift during placement.

10.0 RETAINING WALLS

No specific information has been provided regarding the construction of the retaining walls near the proposed building and parking on the south end of the site. However, in the event that Mechanically Stabilized Earth (MSE) walls are utilized that will directly or indirectly support the foundations of the new building and parking, it should be recognized these passive wall systems that utilize geogrid reinforcement must undergo some amount of strain to develop resistive strength, which may result in settlement along the adjacent building line.

If MSE walls are planned within close proximity (within 20-feet of the reinforced zone) to any of the building or other areas that cannot undergo settlement, except for a drainage layer directly behind the face blocks, it is recommended that crusher-run limestone such as AB-3 should be used within the entire reinforced zone. The geogrid reinforcement should be placed and overlapped as needed in accordance with the manufacturer's recommended that the geogrid layers should be carefully stretched and staked firmly into position prior to placement of the crusher-run limestone.

During the construction of any critical retaining walls that will directly or indirectly support the structures or other areas where settlement behind the top of the wall cannot to tolerated, it is recommended that full-time construction observation, monitoring and testing should be implemented. This would include subgrade preparation beneath the wall alignment, proper installation of the geogrid layers, verification of lift thickness and the compaction of the fill within the reinforced zone.

In addition, it is recommended that a global stability analyses should be submitted with the wall design to help insure the proposed retaining wall design and geogrid lengths are sufficient to prevent a deep-seated slope failure extending into the underlying subgrade materials from compromising the stability of the adjacent



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buildings. Please note, once the initial retaining wall design parameters (height, length, location, etc.) are established by Others, AOG can, and would be happy to, assist with a global stability analysis at an additional cost.

AOG would be pleased to work with the designer of any such MSE wall system to respond to any questions or geotechnical engineering related issues that may arise once more detailed information regarding any necessary retaining walls is available.

11.0 PAVEMENTS

11.1 Subgrade Preparation

Please note, a formal pavement design is beyond AOG's scope of service. Standard asphaltic concrete and concrete pavement designs for a given service life requires evaluation of the soil by means of a California Bearing Ratio (CBR) test or other methods, estimates of traffic volumes and axle weights, drainage requirements, and the desired level of maintenance. As such, some standard pavement design options based on assumptions made for materials of this nature are included in this section.

Without stabilization or treatment, the subgrade soils at this site are considered to be poor subgrade materials for the support of pavements. California Bearing Ratio (CBR) values we have obtained rarely exceed 5, soaked, for these materials. Pavements, either total strength flexible or rigid, do not usually perform well when they are placed directly on highly expansive, poor soil subgrades. Soft areas can develop during wet periods and differential shrinkage can occur during dry periods. As a result, no pavement can avoid damage from wheel loads under these circumstances.

<u>Unless the subgrade is stabilized with Class C flyash, the subgrade for all pavements should consist of at least</u> <u>9 inches of properly compacted soil, which will require tilling and recompacting in cut sections.</u> It is recommended that any untreated aggregate base or flyash stabilized subgrade layers should extend at least 2 feet beyond the pavement and curb lines. The subgrade should be compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content. Any additional fill that is required to develop the paved areas should also be placed in loose lifts not exceeding 8 inches in thickness and compacted in accordance with these recommendations. The subgrade should be proof-rolled with a loaded tandem-axle dump truck after the final subgrade elevation has been established throughout the paved area. A representative of Alpha-Omega Geotech, Inc. should witness this proof-rolling.

Please note, if asphaltic pavements are used, annual maintenance including, but not limited to, crack sealing, fog sealing, and possible patch with overlay should be anticipated. In addition, the quality of the aggregates and overall composition of the asphalt or concrete mix, as well as, drainage conditions can have a profound effect upon the durability of the pavement section.

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11.2 Pavement Sections

Table 2: Recompacted Subgrade Section

RECOMP. PAVEMENT MATERIALS	CAR PARKING	DRIVE LANES	HEAVY DUTY AREAS (i.e. Dumpster pads, approach lanes, etc.)
Asphaltic Surface Course	2	2	NA
Asphaltic Base Course	3	5.5	NA
Portland Cement Concrete	5	7	8
Crushed Stone (3/4-inch minus)	4	4	4

*Reference Section 10.3, "Recompacted Subgrade Sections"

Table 3: Recommended Thicknesses with Flyash Subgrade Stabilization

FLYASH SUBGE	RADE STABILIZATION.	SECTIONS (INCHI	S)
PAVEMENT MATERIALS	CAR PARKING	DRIVE LANES	HEAVY DUTY AREAS (i.e. Dumpster pads, approach lanes, etc.)
Asphaltic Surface Course	2	2	NA
Asphaltic Base Course	2	4	NA
Flyash Stabilization	12	12	NA
Portland Cement Concrete	4	6	7
Crushed Stone (3/4-inch minus)	4	4	4
Flyash Stabilization	12	12	12

*Reference Section 10.4, "Subgrade Stabilization Sections"

Table 4: Recommended Thicknesses with Geogrid Reinforcement & Baserock

GEOGRID REINFORGEMENT AND	BASEROCK SUBGRAD	E STABILIZATION	NSECTIONS (INCHES)
PAVEMENT MATERIALS	CAR PARKING	DRIVE LANES	HEAVY DUTY AREAS (i.e. Dumpster pads, approach lanes, etc.)
Asphaltic Surface Course	2	2	NA
Asphaltic Base Course	2	4	NA
Crushed Stone (3/4-inch minus)	6	6	NA
Portland Cement Concrete	4	6	7
Crushed Stone (3/4-inch minus)	6	6	6

*Reference Section 10.4, "Subgrade Stabilization Sections"



11.3 Recompacted Subgrade Sections

11.3.1 Flexible Pavements Sections

From an initial cost perspective, flexible asphaltic concrete pavement is the most economical pavement section. However, treating the subgrade with Class C flyash or using a geogrid reinforced base course can provide a higher quality pavement section, having a much longer service life. Nevertheless, if the subgrade is untreated and asphaltic pavement is used, areas used exclusively for automobile parking should consist of at least 5.0 inches of asphaltic concrete (2.0 inches of surface mix and 3.0 inches of base mix). <u>Drives should be constructed of at least 7.5 inches of asphaltic concrete (2.0 inches of surface and 5.5 inches of base mix)</u>. The above-referenced pavement section represents minimum design thicknesses and, as such, periodic maintenance should be anticipated. If an increased pavement performance is desired, as described in Section 10.4, "Subgrade Stabilization," flyash stabilization or the use of a layer of base rock and geogrid reinforcement may be considered. Asphaltic cement concrete should NOT be used in areas where heavy truck loads/concentrations are expected.

It is also recommended that an asphalt binder grade of PG 64-28 should be considered to help reduce the potential of thermal cracking based on the climatic conditions of this region. However, for base mix asphalt placed at least 4 inches below the surface, an asphalt binder grade of PG 64-22 should be sufficient.

11.3.2 Rigid Pavement Sections

As an alternative, rigid Portland Cement concrete with a 4-inch thick base course of crushed limestone may also be used with minimum thicknesses of 5.0 and 7.0 inches for automobile parking areas and drive lanes, respectively. The above-referenced pavement section represents minimum design thicknesses and, as such, periodic maintenance should be anticipated. If a better pavement is desired, as described in Section 10.4, "Subgrade Stabilization," flyash stabilization or the use of a layer of base rock and geogrid reinforcement may be considered.

The crusher-run limestone base course should be compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content sufficient to achieve the specified level of compaction.

For areas where heavy truck loads/concentrations are anticipated, Portland Cement concrete is recommended. Portland cement concrete slabs having a thickness of 8 inches over a 4-inch, minimum, compacted, crusher-run limestone base should be used for dumpster stations, parking lot entrances, areas where a high concentration of heavily loaded trucks are anticipated, as well as, any areas where trucks accelerate/decelerate and execute sharp turning maneuvers.

11.4 Subgrade Stabilization Sections

Alternate pavement sections utilizing flyash, geogrids, granular base and/or subbase courses should be considered. Treating the subgrade with Class C flyash or using a geogrid reinforced base course can provide a pavement section having a much longer service life.



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If specific pavement performance standards are to be met, AOG would be pleased to be of further assistance once the actual design loading conditions, service-life and maintenance expectations have been defined.

11.4.1 Flyash

<u>The use of flyash is usually not effective during cold winter months.</u> Notwithstanding this weather limitation, assuming the flyash is thoroughly and uniformly mixed with the subgrade, flyash stabilization can greatly reduce the swelling potential and improve the strength of the subgrade soil.

If the subgrade is stabilized with Class C flyash to a depth of 12 inches, full depth asphalt pavements with thicknesses of 4.0 and 6.0 inches for parking and drive lanes, respectively, can be used. Likewise, if the subgrade is stabilized with flyash, the Portland cement concrete pavement sections over a 4-inch thick base course of crushed limestone may also be reduced to 4.0 and 6.0 inches, respectively. The crushed limestone base course should be compacted to the specifications given in Section 10.3, "Rigid Pavement Sections," of this report.

Based on experience with similar projects, adding more flyash does not always increase the stiffness of the subgrade. In fact, too much flyash in the subgrade may cause excessive brittleness, which may result in reflective cracking problems to develop. It is usually cost effective to determine the optimum amount of flyash necessary by laboratory testing; however, it usually ranges from about 12 to 15 percent by weight. The Class C flyash should be thoroughly mixed with the subgrade soil by means of a Bomag tiller or other similar equipment specifically designed for such procedures and compacted to a minimum density of 95 percent of the Standard Proctor (ASTM D698) maximum dry density at a moisture content within ± 3 percent of the optimum moisture content.

11.4.2 Geogrid Reinforcement & Base Rock

Soft areas can develop even when the subgrade is stabilized with Class C flyash. An even better pavement section can be developed by the use of a tri-axial geogrid over a properly compacted subgrade, as discussed in this report, and a layer of untreated crushed limestone base rock under either flexible or rigid pavements. The purpose of the geogrid is to help span soft spots that will inevitably develop in the subgrade. The geogrid helps to confine the base rock and acts as a "snowshoe," distributing the loads over the subgrade in a tri-axial direction. The layer of base rock, which is placed over the geogrid, must be thick enough to support construction traffic and paving equipment so the geogrid does not become exposed. In general, the crushed limestone base rock should not be less than approximately 6 inches in thickness. If this option is chosen, it is recommended that Tensar TX-140, which is a tri-axial polypropylene geogrid, be used. The geogrid reinforcement should be placed and overlapped as needed in accordance with the manufacturer's recommendations, which should be verified by a representative of Alpha-Omega Geotech, Inc.

Asphaltic concrete thicknesses of 4.0 and 6.0 inches for parking areas and drive lanes, respectively, can be used if geogrid and base rock stabilization are used. Similarly, the Portland cement concrete sections can be reduced to 4.0 and 6.0 inches for the respective areas. Although these thicknesses are the same as given if the subgrade is treated with Class C flyash, the use of a tri-axial geogrid and base rock usually represents the most effective, reasonable pavement section.



11.5 General

If asphaltic pavements are used, periodic maintenance including, but not limited to, crack sealing, fog sealing, and possible patch with overlay should be anticipated. In addition, the quality of the aggregates and overall composition of the asphalt or concrete mix, as well as, drainage conditions can have a profound effect upon the durability of the pavement section.

Where engineered controlled fill is placed beneath paved areas, it is recommended the compacted fill should extend a minimum distance of two (2) feet beyond the pavement edge or curb line, or a distance equal to the depth of the fill, whichever is greater.

Asphalt mixes meeting KDOT BM-2 and BM-2B specifications may be used for surface and base mixes, respectively. Compaction testing of each pavement layer is recommended to help ensure compliance with the mix design specifications.

For areas where heavy truck loads/concentrations are anticipated, Portland Cement concrete is should be used. It is recommended that load-transfer devices should be installed where construction joints are required. For dumpster stations, the concrete slabs should be large enough to accommodate the dumpster and at least the rear wheels of the disposal vehicle. Rigid pavements should have No. 4 bars on at least 2-foot centers and positioned in the upper third of the slab. Joints should be tooled or cut within 4 hours of hardening to a depth of at least one fourth of the thickness.

The subgrade should be moistened prior to placement of concrete. Fresh concrete should be properly cured as recommended by the American Concrete Institute (ACI). To help provide resistance to damage caused by alternating cycles of freezing and thawing, it is recommended that any exposed concrete should be properly air entrained; typically at 5 to 7 percent. In addition, it is also recommended the outer edges of pavement slabs should be thickened to help resist cracking associated with heavy wheel loads near these unrestrained areas.

If full-depth pavement is used, it is important the moisture content of the subgrade should be kept as constant as possible from the time of recompacting until the pavement is laid. However, if the subgrade becomes dry, it should be moistened for at least 72 hours prior to paving, but it should not be saturated. In all cases, pavements should be sloped to inhibit ponding and provide rapid surface drainage. If water is allowed to pond on or adjacent to the pavement, the subgrade could become saturated and lose its bearing capacity which would contribute to premature pavement deterioration under a single cycle of heavy wheel loads or a number of cycles of lighter wheel loads.

12.0 TESTING AND INSPECTION RECOMMENDATIONS

Unless Alpha-Omega Geotech, Inc. is retained to provide the construction observation, monitoring and testing services for this project, we cannot accept any responsibility for any conditions that deviate from those identified in this subsurface investigation nor for the performance of the foundations, pavements and other structures including any retaining walls that are a part of this project. Alpha-Omega Geotech, Inc. is accredited by AASHTO and we are experienced in construction quality control and have a fully-equipped soil, concrete,



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aggregate, rock and asphalt testing laboratory, as well as, qualified field technicians to provide these field services.

It is not economically practical to perform enough exploratory borings on any site to identify all subsurface conditions. Some conditions affecting the design and/or construction may not become known until the project is underway. The boring logs, field SPT and laboratory test results depict subsurface conditions only at the specified locations and depths at the site. The boundaries between soil and rock layers indicated on the boring logs are based on observations made during drilling and an interpretation of the laboratory testing results. The exact depths of these boundaries are approximate and the transitions between soil and rock types may be gradual rather than being clearly defined. Also, due to the prior development at this site, as well as, the natural conditions of the formation of soils and rock, it is possible that unanticipated subsurface conditions may be encountered during construction. Monitoring of the subsurface conditions that are revealed during construction is needed to verify that subsurface conditions are consistent with those conditions identified in this preliminary geotechnical investigation. If variations in subsurface conditions are encountered, it will be necessary for Alpha-Omega Geotech, inc. to re-evaluate the recommendations that have been made in this report.

Special Inspections should be performed in accordance with the local building code under which the project is designed, as adopted by Raymore, Missouri.

Prior to filling, it is recommended that a representative of Alpha-Omega Geotech, Inc. should verify that the site has been properly stripped of all topsoil and other deleterious material, benched as needed and prepared for the placement of fill. The compaction of any structural fill beneath the new building, pavements, and any other areas where settlement control is necessary should be tested lift-by-lift by a representative of Alpha-Omega Geotech, Inc. as it is being placed. This should include the prepared subgrade layers beneath the building's slab-on-grade, as well as, any other fill material relied upon to provide passive resistance. Also, in accordance with the local building code, any fill that is used to construct slopes steeper than 4:1 (H:V) must be placed as engineered controlled fill and the compaction tested lift-by-lift during placement.

Assuming that uniform fill material is used, nuclear density gauges (ASTM D2922/D3017) should be used to test compaction wherever necessary. However, if fill material of non-uniform consistency is used, other evaluation methods may be required. Such methods may include, but not be limited to, the use of a GeoGauge Stiffness meter, Dynamic Cone Penetrometer (DCP), proof-rolling or other visual inspection techniques.

Any geotextile fabric and geogrid reinforcement that is utilized should be placed and overlapped as needed in accordance with the manufacturer's recommendations, which should be verified by a representative of Alpha-Omega Geotech, Inc. Proper placement of the reinforcing steel for drilled piers, grade beams, pier caps, foundation walls and other structural elements including any necessary wing walls and retaining walls should be verified prior to the placement of concrete. The subgrade under the slabs on grade and pavements should be checked to verify they are in compliance with the density and moisture requirements. Wherever possible, in addition to compaction testing, cut and fill areas should be proof-rolled with a loaded tandem-axie dump truck to identify soft areas that will need to be corrected. A representative of Alpha-Omega Geotech, Inc. should observe this proof-rolling. Checks should also be made of the subbases, concrete and any pavement materials.

Finally, the inspection and testing services listed herein are given as a minimum and it should be understood that additional inspection and testing services might also be required or otherwise beneficial.

13.0 LIMITATIONS

This report is presented in broad terms to provide a comprehensive assessment of the interpreted subsurface conditions and their potential effect on the adequate design and economical construction of the proposed Hawk Ridge Park project located in Raymore, Missouri, as discussed herein. This report has been prepared for the exclusive use of our client for specific application to the project discussed herein and has been prepared within our client's directive and budgetary constraints and in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made.

It should be noted that the concept of risk is an important aspect of the geotechnical engineering evaluation and report since the recommendations given in this report are not based on exact science but rather analytical tools and empirical methods in conjunction with engineering judgment and experience. Therefore, the recommendations given herein should not be considered risk-free and, more importantly, are not a guarantee that the interaction between the soil materials and the proposed structures will perform as planned. Nevertheless, the geotechnical engineering recommendations presented herein are Alpha-Omega Geotech, Inc.'s professional opinion of those measures that are necessary for the proposed structures to perform according to the proposed design based on the information provided to Alpha-Omega Geotech, Inc., the referenced information gathered during the course of this investigation and our experience with these conditions.

Any significant structural changes to the proposed new structure or its location on this site relative to where these test borings were completed shall be assumed to invalidate the conclusions and recommendations given in this report until we have had the opportunity to review these changes and, if necessary, modify our conclusions and recommendations accordingly. It is also strongly suggested that Alpha-Omega Geotech, Inc. should review your plans and specifications dealing with the earthwork, foundations, as well as, any pavements prior to construction to confirm compliance with the recommendations given herein. Particular details of foundation design, construction specifications or quality control may develop, and we would be pleased to respond to any questions regarding these details.

If Alpha-Omega Geotech, Inc. is not retained to review the project plans and specifications, address to the proposed building and parking structure or their location on the site relative to where these test borings were completed, provide the recommended construction phase observation, monitoring and testing services and respond to any subsurface conditions that are identified during construction to evaluate whether or not changes in the recommendations given in this report are needed, we cannot be held responsible for the impact of those conditions on the project or the future performance of the buildings, pavements and/or structures that may be involved.

The scope of our services did not include any environmental assessment or investigation for the presence of hazardous or toxic materials in the soil, surface water, ground water or air, either on, below or adjacent to this site. In addition, no determination regarding the presence or absence of wetlands was made. Furthermore, it should be understood that the scope of geotechnical services for this project does not include either



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specifically or by implication any biological (i.e. mold, fungi or bacteria) assessment of the site or the proposed construction. Any statements in this report or included on the boring logs regarding odors, colors and unusual or suspicious items or conditions are strictly for informational purposes only.

We appreciate the opportunity to be of service to Confluence, as well as, the project developers and look forward to working with you throughout the construction process. We are prepared to provide the Special Inspection services that will be required by the local building code under which this project is designed, as adopted by the City of Raymore, Missouri, as well as, the other necessary construction observation, monitoring and testing services discussed in this report. If you have any questions concerning this report, or if we may be of further assistance, please call us at (913) 371-0000.

Sincerely, ALPHA-OMEGA GEOTECH, INC.

Jacob Engler, P.E. Geotechnical Engineer

Enclosures

Garic Abendroth, P.E.

Garic Abendroth, P.E. Engineering Department Manager

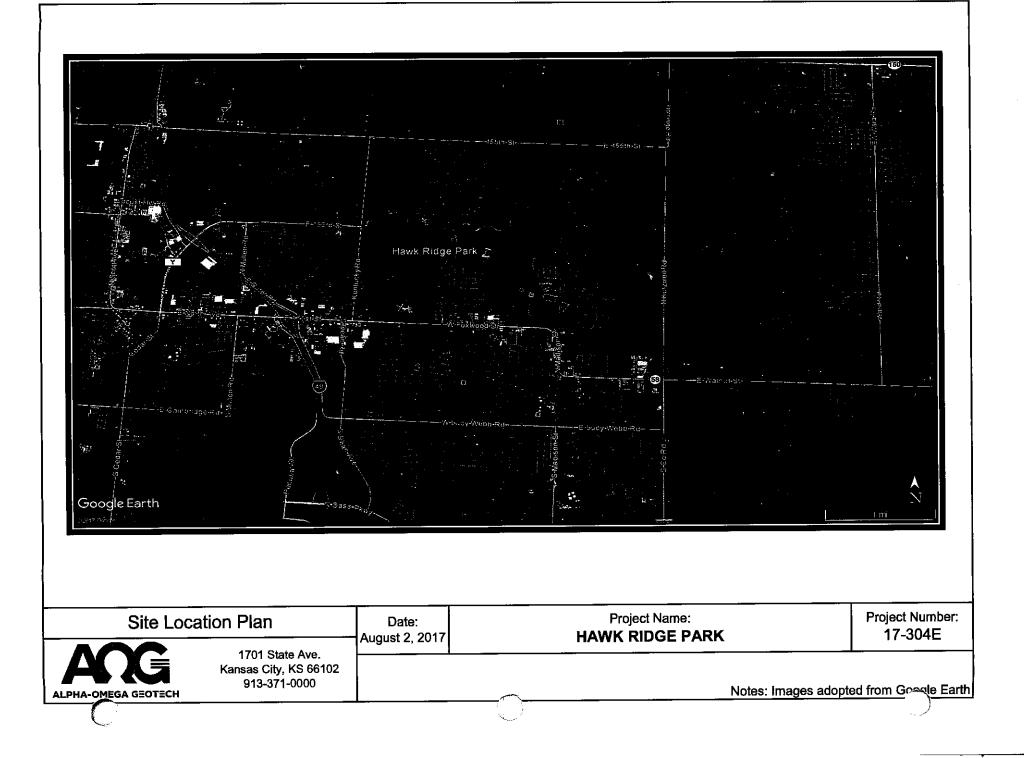


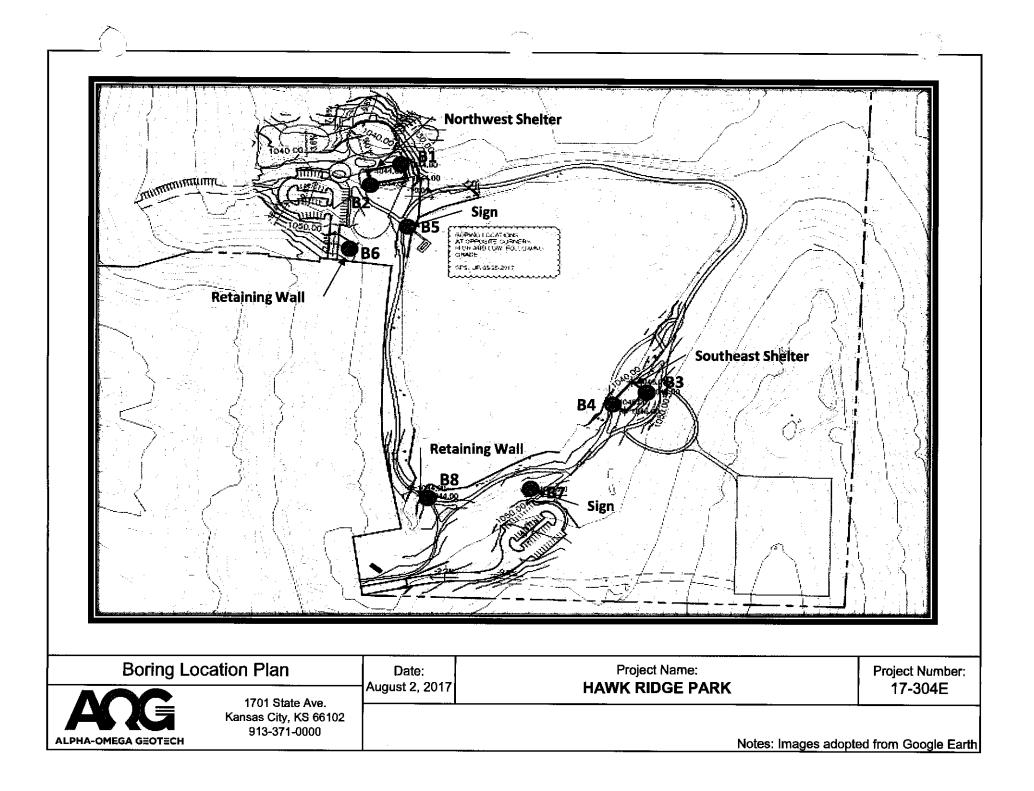
Appendix Section A

SITE SKETCH

Site and Boring Location Plans

Specialty Simplified. Real World Confidence.





Appendix Section B

LABORATORY TEST RESULTS



Alpha-Omega Geotech, Inc. 1701 State Avenue Kansas City, KS 66102 Office: (913) 371-0000 Fax: (913) 371-6710 Website: www.aogeotech.com

PROJECT NAME: PROJECT LOCATION:			Hawk Ridge Raymore, M			BER:	17-304E 7/20/2017							
Boring Number	Sample Number	Depth or Elevation	Description	Natural Moisture (%)	Dry Unit Weight (pcf)	LL	Atterberg Limits PL	PI	USCS/ Visual Class.	% Passing No, 200	Unconfined Compression (psf)	%е	% Swell	Remarks
B-1	ST-1	2'-4'	Brown, mottled reddish brown and dark brown FAT CLAY with trace of slickened sides	24.9	97.9	217-387			Сн	140. 200	<u>(psiy</u> 3939	3.3		PP = 2.0 tsf
B-1	ST-2	4'-6'	Brown, mottled dark brown, spotted reddish brown LEAN/FAT CLAY with trace of sand, gravel and organics (finger roots) (probable fill)	22.2	98.6				CL/CH					PP = 1.75 tsf
B-1	SS-1	6'-7.5'	Light brown LEAN CLAY with sand	20.2					CL					N = 19
B-1	\$ \$- 2	8.5'-10'	Reddish brown LEAN/FAT CLAY with sand and trace of gravel						CL/CH					N = 23
B-1	\$S-3	13.5'-15'	Gray weathered shale with sand						SH					N = 50/4"
B-1	SS-4	18.5'-20'	Gray shale						SH					N = 50/5"
B-2	ST-1	2'-4'	Brown FAT CLAY with trace of sand and gravel (probable fill)	18.5	95.1	53	24	29	Сн					PP =>4.5 tsf
B-2	ST-2	4'-6'	Brown LEAN/FAT CLAY with trace of sand and gravel	14.4	110.5				CL/CH		7358	2.2		PP = 4.5 tsf



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SLT 22205

Alpha-Omega Geotech, Inc. 1701 State Avenue Kansas City, KS 66102 Office: (913) 371-0000 Fax: (913) 371-6710 Website: www.aogeotech.com

PROJEC PROJEC	T NAME T LOCA	-	 Hawk Ridge I Raymore, M 			PROJECT NUMBER: DATE:				304E /2017				
Boring Number	Sample Number	Depth or Elevation	Description	Natural Moisture (%)	Dry Unit Weight (pcf)	LL	Atterberg Limits PL	РІ	USCS/ Visual Class.	% Passing No. 200	Unconfined Compression (psf)	%е	% Swell	Remarks
В-2	SS-1	6'-7.5'	Brown FAT CLAY with trace of sand and gravel (probable fill)	14.3					СН					N = 5
B-2	SS-2	8.5'-10'	Light brown, spotted gray sandy LEAN CLAY						CL					N = 22
B-2	SS-3	13.5'-15'	Gray and brown LEAN CLAY with sand (weathered shale)						CL					N = 50/4"
B-2	SS-4	18.5'-20'	Gray shale						SH					N = 50/3"
B-3	ST-1	2'-4'	Dark brown FAT CLAY with trace of organics	22.2	100.4			-	СН		9046	5.9		PP = >4.5 tsf
B-3	ST-2	4'-6'	Brown, speckled reddish brown LEAN/FAT CLAY with trace of organics	22.9	99.9				CL/CH					PP = 4.5 tsf
B-3	SS-1	6'-7.5'	Brown, mottled gray, spotted reddish brown FAT CLAY	28.1					СН					N = 8
B-3	SS-2	8.5'-10'	Brown, spotted reddish brown and gray FAT CLAY						СН					N = 10
B-3	SS-3	13.5'-15'	Brown, spottd gray FAT CLAY						СН					N = 7



Alpha-Omega Geotech, Inc. 1701 State Avenue Kansas City, KS 66102 Office: (913) 371-0000 Fax: (913) 371-6710 Website: www.aogeotech.com

PROJEC PROJEC				Hawk Ridge Park Raymore, MO				PROJE DATE:	CT NUM	BER:	17-304E 7/20/2017			
Boring Number	Sample Number	Depth or Elevation	Description	Natural Moisture (%)	Dry Unit Weight (pcf)	LL	Atterberg Limits PL	PI	USCS/ Visual Class.	% Passing No. 200	Unconfined Compression (psf)	%e	% Swell	Remarks
B-3	SS-4	18.5'-20'	Brown, mottled gray LEAN/FAT CLAY (weathered shale)						CL/CH					N = 48
B-4	ST-1	2'-4'	Brown LEAN CLAY with trace of organics (finger roots)(probable fill)	31.5	83.3				CL					PP = 1.25 tsf
B-4	ST-2	4'-6'	Dark brown, speckled reddish brown LEAN/FAT CLAY	29.9	90.4				СН		112 8	15.5		PP = 1.0 tsf
B-4	SS-1	6'-7.5'	Brown, mottled gray and reddish brown FAT CLAY	24.8					Сн					N = 7
B-4	SS-2	8.5'-10'	Brown, mottled gray and light reddish brown FAT CLAY						Сн					N = 8
B-4	SS-3	13.5'-15'	Brown, mottled gray FAT CLAY						СН					N == 8
B-4	SS-4	18.5'-20'	Brown, mottled gray LEAN/FAT CLAY (weathered shale)						CL/CH					N = 56 ×
B-5	ST-1	2'-4'	Brown, spotted reddish brown FAT CLAY	26.6	95.6				СН					PP = 3.5 tsf
B-5 •	SS-1	5'-6.5'	Reddish brown FAT CLAY with sand	19.9					Сн					N = 9
B-5	SS-2	8.5'-10'	Brown LEAN CLAY with sand and trace of gravel						CL					N = 42

SLT 22205

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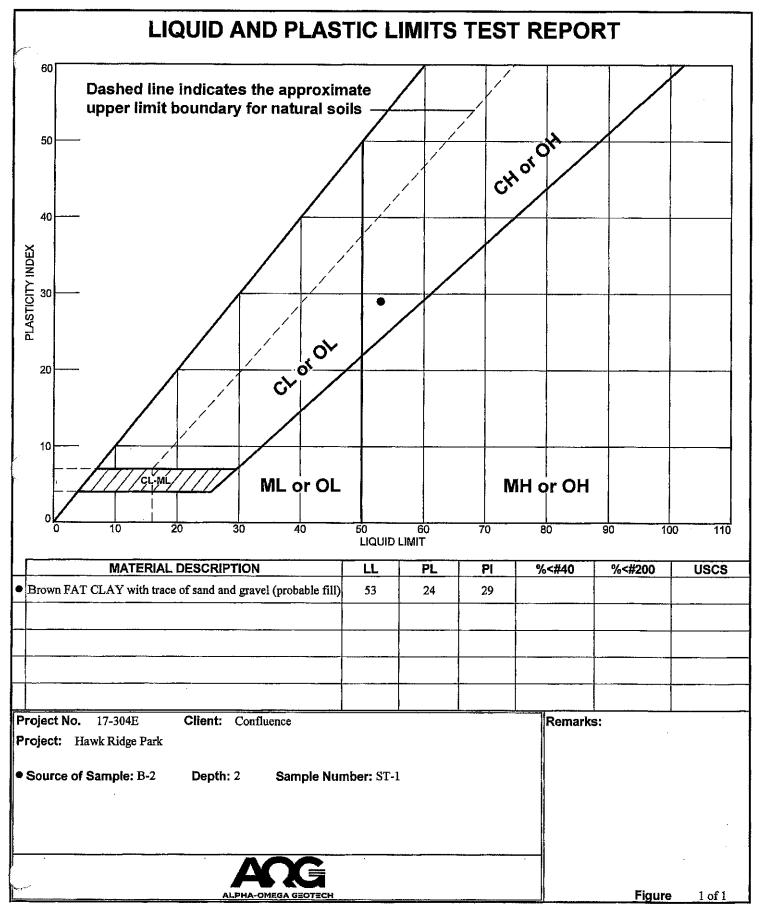
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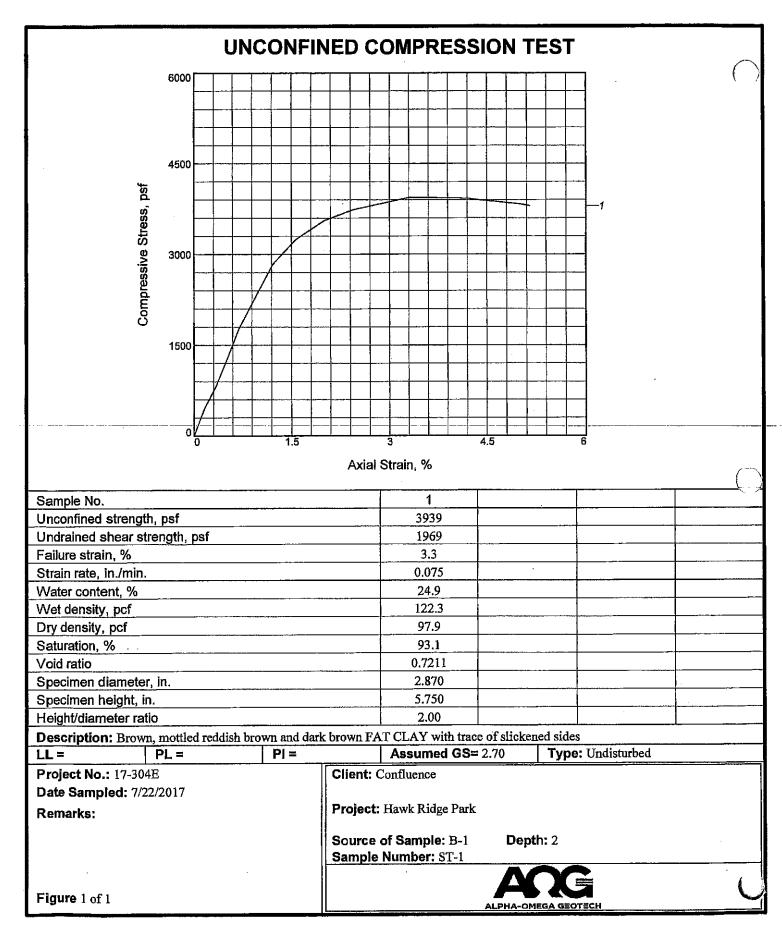
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Alpha-Omega Geotech, Inc. 1701 State Avenue Kansas City, KS 66102 Office: (913) 371-0000 Fax: (913) 371-6710 Website: www.aogeotech.com

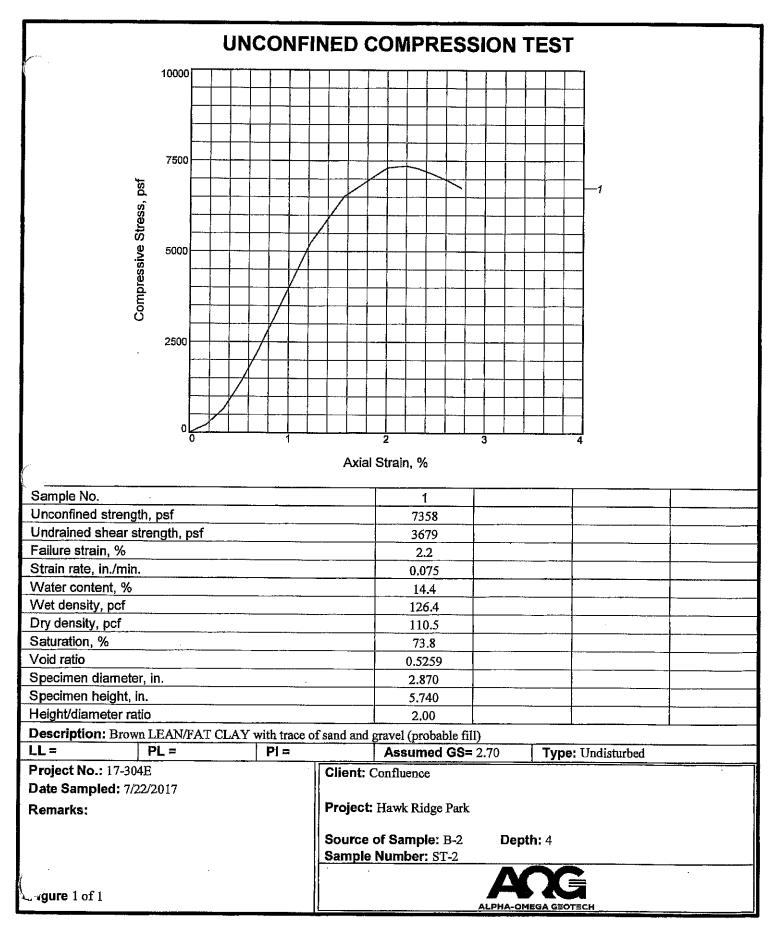
PROJEC PROJEC			Hawk Ridge I Raymore, M			PROJECT NUMBER: DATE:				BER:		304E /2017		
Boring Number	Sample Number	Depth or Elevation	Description	Natural Moisture (%)	Dry Unit Weight (pcf)	LL	Atterberg Limits PL	PI	USCS/ Visual Class.	% Passing No. 200	Unconfined Compression (psf)	%е	% Swell	Remarks
B-6	ST-1	2'-4'	Brown, mottled reddish brown FAT CLAY with trace of organics (finger roots)	20.9	102.6				СН		8307	5.0		PP = 4.0 tsf
B-6	SS-1	5'-6.5'	Brown FAT CLAY with sand and trace of gravel						СН					N = 9
B-6	SS-2	8.5'-10'	Light brown LEAN CLAY with sand	14.1					CL					N = 50/2"
B-7	ST-1	2'-4'	Brown, spotted light reddish brown and gray FAT CLAY with trace of organics (finger roots)	27.6	92.0	-			СН					PP = 1.25 tsf
B-7	SS-1	5'-6.5'	Light brown, mottled gray FAT CLAY	27.0					СН					N = 8
B-7	SS-2	8.5'-10'	Light brown, mottled gray FAT CLAY						СН					N = 12
В-8	ST-1	2'-4'	Dark brown FAT CLAY with trace of gravel and sand	23.7	94.1				СН					PP = 3.5 tsf
B-8	SS-1	5'-6.5'	Brown FAT CLAY						СН					N = 5
B-8	SS-2	8.5'-10'	Brown, speckled reddish brown LEAN/FAT CLAY	32.1					CL/CH					N = 4







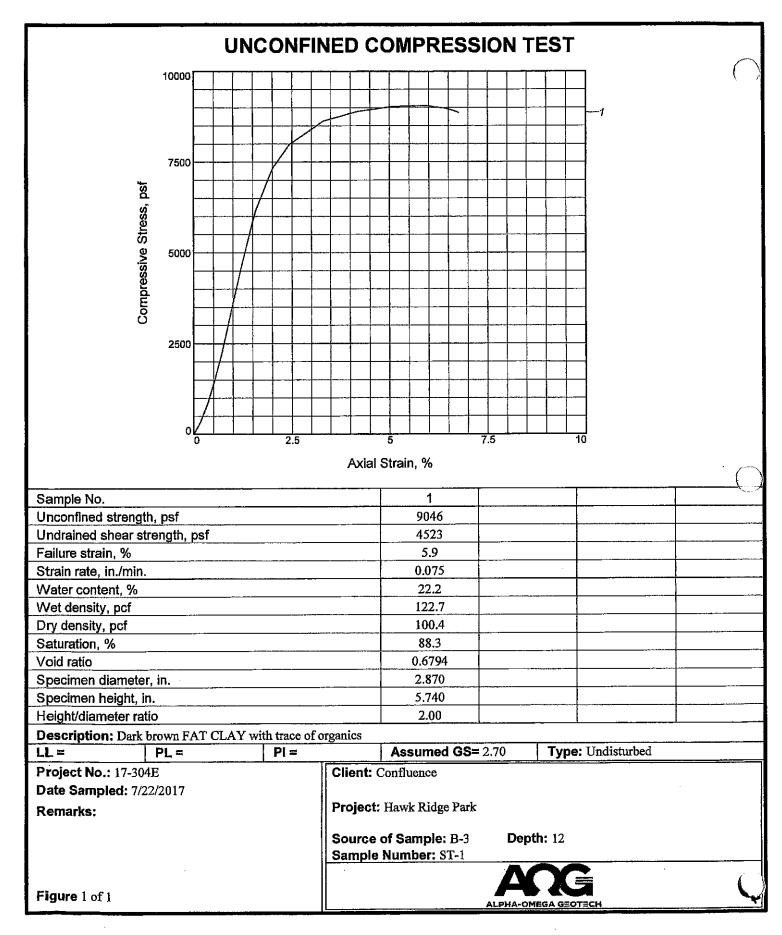
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Tested By: DB

Checked By: GA

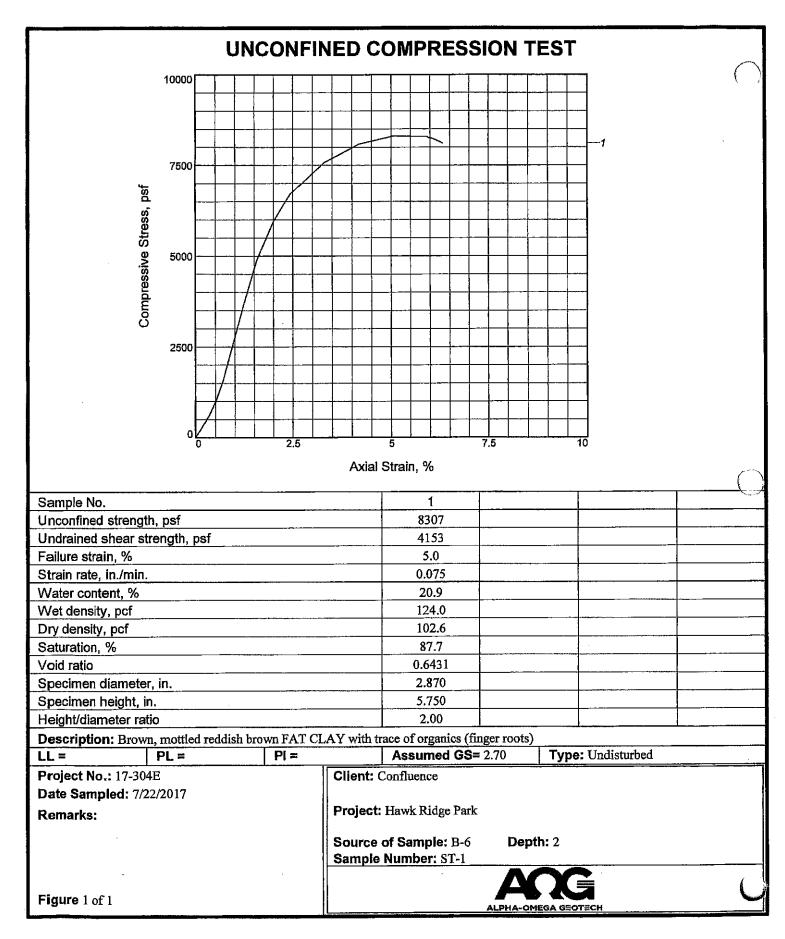
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Checked By: GA

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figure 1 of 1													<u></u>				<u>1A-C</u>	MEG	AG	OTE	CH

Tested By: DB Checked By: GA



Tested By: DB

Checked By: GA

Appendix Section C

BORING LOGS

Note: The logs of subsurface conditions shown in this section apply only at the specific boring location and depths at the date indicated and might not be indicative of all subsurface conditions that may be encountered. This information is not warranted to be representative of subsurface conditions at other locations, depths and times. The passage of time or construction operations at or adjacent to this site may result in changes to the soil conditions at these boring locations and depths. As a result, the character of subsurface materials shall be each bidder's responsibility.

ALPHA-OMEGA GEOTE LOG OF BORING No. B-1	CLIENT: <u>Confluence</u> PROJECT LOCATION: <u>Raymore, MO</u> LOCATION: <u>NW Shelter</u> DRILLER: <u>Chuck Jacobs</u> DRILLING METHOD: <u>AO/SS/ST</u> DEPTH TO - WATER> INITIAL: ₩ 15' AFTER 3									
LOG OF BORING No. B-1	H LOCATION: <u>NW Shelter</u> DRILLER: <u>Chuck Jacobs</u> DRILLING METHOD: <u>AO/SS/ST</u>								1	
LOG OF BORING No. B-1	DRILLER: Chuck Jacobs DRILLING METHOD: AO/SS/ST								{	^
No. B-1	DRILLING METHOD: AO/SS/ST									
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ation Soll Sym Sampler Sy Depth (ft.) and Fletd Te		24 HOU	JRS:	÷	NA			<u>ع <3</u>	1	_
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	Brown, mottled reddish brown and dark brown FA CLAY with trace of slickened sides (probable fill)	T	24.9	97.9				3939	2.0	C
5	Brown, mottled reddish brown and dark brown FA CLAY with trace of slickened sides (probable fill)	T	22.2	98.6					1.75	C
	Brown, mottled brown, spotted reddish brown LEA		20.2		 					
	(finger roots)(probable fill)	6								C
	Light brown LEAN CLAY with sand									
	Reddish brown LEAN/FAT CLAY with sand and gravel	trace of	t 							
-15 - 2	Reddish brown LEAN/FAT CLAY with sand and gravel									
	Gray weathered shale with sand	13/5 16								
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)	ALPHA-OMI	EGA GEOTECH	LOCATION: NW Shelter		FI	EVAT	ION		NI	<u> </u>	
	LOG OF	BORING	DRILLER: Chuck Jacobs						like Bur		3.
i		. B-2	DRILLING METHOD: AO/SS/ST			UUL			<u>11KC Dui</u> 7/1		
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Ē	Elevation	Soil Symbols Sampler Symbols		T	DDen			r	Uncomp.		
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	Ļ	233333 2 3324	Topsoil								
		- H	Brown FAT CLAY with trace of sand and gravel						<i></i>		C
	-		(probable fill)	18.5	95.1	53	29			>4.5	C
	- 5		Brown FAT CLAY with trace of sand and gravel (probable fill)	14.4	110.5				7358	4.5	
	. [2023	Brown LEAN/FAT CLAY with trace of sand and gravel (probable fill)	14.3							C C
	ŀ		Brown FAT CLAY with trace of sand and gravel								CI
	-	5 10 12	(probable fill) Prover FAT CLAN with the fill 7.5								c
	10		Brown FAT CLAY with trace of sand and gravel (probable fill)								c
	-		Light brown, spotted gray sandy LEAN CLAY								
	Ę	507 4"	Light brown, spotted gray sandy LEAN CLAY								c
	- 15		Gray and brown LEAN CLAY with sand (weathered shale)								С
			Gray and brown LEAN CLAY with sand (weathered shale)						:		
	-		Gray shale 18.5								S
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Sampler Symbols and Fletd Test Data Description w% Duent per LL PI 200 % Dition fr. PPen. psr 0 Topsoil 1 0 0 0 0 1 Dark brown FAT CLAY with trace of organics 22.2 100.4 9046 >4.5			PROJECT: Hawk Ridge Park		PR	OJEC	IT NO	»: <u> </u>	17-3	04E	
ALPHA-OMEGA GEOTECM LOCATION: SE Shale: LOCATION: SE Shale: LOGGED BY: Clock Jacoby DRLLING METHOD: ADSSYT DEFINITO: WATER> INITIAL: # None_ AFTER 24 HOURS: # NA_ CAVING> C. None DATE: 71/20217 Depth cl. application Description r/w Definition Description Depth cl. application Description r/w Definition Description r/w Definition Depth cl. application Description r/w Definition Def		XŠ								.	
LOG OF BORING No. B-3 DRLLING METHOD: <u>A058957</u> DRLLING METHOD: <u>A058957</u> DepTH TO - WATER> INITIAL: <u>\$\u03c9</u> None_AFTER 24 HOURS: $\u03c9 NinKOS (Noneation$			PROJECT LOCATION: Raymore, MO			•					~
LOG OF BORING No. B-3 DRILLER: Mike Burdled, Str. LOGGED BY: Cluck Lacks PRILLING METHOD: ADJSYST Depth (IL) Sold Synthed: Depth (IL) DATER: NAT DATER: NAT Depth (IL) Sold Synthed: Depth (IL) Description wk Desc New Deft I I New Depth (IL) Sold Synthed: Depth (IL) End Synthed: End Synthed: Depth (IL) Description wk Poet I <td>ALPHA-OME</td> <td>GA GEOTECH</td> <td>LOCATION: SE Shelter</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	ALPHA-OME	GA GEOTECH	LOCATION: SE Shelter								
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Between the state of the st	No.	B-3	DRILLING METHOD: AO/SS/ST				<u> </u>				
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		CLIENT: Confluence									
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		LOCATION: SE Shelter		EL	EVA1	FION:		<u>N</u>)		
	OF BORING	DRILLER: Mike Burdick, Sr.		LO	GGE	:	Chuck Jacobs				
	No. B-4	DRILLING METHOD: AO/SS/ST			DATE: 7/19/2017						
		DEPTH TO - WATER> INITIAL: ₩ None AFTER 24 HO	URS:	÷ _	NA	c	AVIN	G> _C			
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		Topsoil	1								
		Brown LEAN CLAY with trace of organics (finger \roots)(probable fill)	31.5	83.3					1.25		
-		Brown LEAN CLAY with trace of organics (finger roots)(probable fill)	2						1.20		
- 5	3	Dark brown, speckled reddish brown LEAN/FAT CLAY		90.4				1128	1.0	0	
ĥ		Brown, mottled gray and reddish brown FAT CLAY	24.8								
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	FAT CLAY	<u>Soil Sa</u>	mplers
	LEAN CLAY w/ trace of Sand		Undisturbed thin wall Shelby tube
	Sandy LEAN/FAT CLAY		Standard penetration test
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	SHALE		
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Notes:			
	ngs were drilled on July 18, 2017 boon techiniques.	7 using a	auger only, shelby tube and
2. Groun	nd water was encountered while in	n the dr:	illing process.
3. Borir	ngs were staked by Alpha-Omega Ge	eotech, :	Inc.
4. These in this	e logs are subject to the limitat report.	tions, c	onclusions, and recommendation:
44	ts of tests conducted on samples ations are:	s recove:	red are reported on the logs.
DDen = w% = UComp = -200 = DCP =	<pre>natural moisture content (%) Unconfined compression (psf) percent passing #200 sieve (%)</pre>	PPen :	= Plasticity index = Pocket Penetrometer

SECOND REVISED BID PROPOSAL FORM E – Project No. 18-253-201 Hawk Ridge Park Improvements 4/27/2018

1. MDC REIMBURSED IMPROVEMENTS	Units	Estimated Quantities	\$/Units	Total
8' Width Asphalt Loop Trail / Sub-Base and testing	TON	234		\$
18'x36' ADA Fishing Dock	LS	1		\$
45' ADA Walkway	LS	1		\$
45' Stiff Arm	LS	1		\$
Concrete bulkheads & Material	EA	2		\$
Delivery and Installation	LS	1		\$
Montrose Flush Building	City Provided	0	0.00	\$0.00
Montrose Flush Building Installation	LS	1		\$
8' Width Concrete/Stone Edge Fishing Pier	LS	1		\$
SUBTOTAL MDC REIMBURSED BASEBID	IMPROVEM	IENTS		\$
Alternates 1,2, and 3 are subs City will consider the three tra			item. The	
Alternate 1 8' Concrete Trail/Sub-Base	CY	115		\$
Alternate 2 10' Concrete Trail/Sub Base	CY	144		\$
Alternate 3 10' Asphalt Trail/Sub-Base	TON	292		\$

2. LAKE LOOP TRAIL	Units	Estimated Quantities	\$/Units	Total
8' Width Asphalt Loop Trail / Sub-Base and testing	TON	485		\$
6' x 40' Bridge and Abutment	Lump Sum	1		\$
Trail Water Crossing	LF	40		\$
8' Connection to Boardwalk	TON	10		\$
Superstructure of Boardwalk	LS	1		\$
Substructure of Boardwalk	Linear Feet	900		\$
SUBTOTAL LAKE LOOP TRAIL	BASE BID)	I	\$
Alternates 1,2,and 3 are are so The City will consider the thre			line item.	
Alternate 1 8' Concrete Trail/Sub-Base	CY	305		\$
Alternate 2 10' Concrete Trail/Sub Base	CY	380		\$
Alternate 3 10' Asphalt Trail/Sub-Base	TON	606		\$
Alternate 6 is a stand alone al increase the bridge to 8 feet w bridge				
Alternate 6 8' x 40' Bridge and Abutment in lieu of 6' x 40' Bridge and Abutment	LS	1		\$

3. NORTH PLAYGROUND PARKING	Units	Estimated Quantities	\$/Units	Total
Demolition of Asphalt Pavement	LS	1		\$
Concrete Shelter Surround	CY	37		\$
Asphalt Paving around Parking Lot APWA Type 3, 4 inches thick	TON	99		\$
Parking Lot, and Driveway APWA Type 1, 4 inches Asphalt with testing	TON	441		\$
Parking Lot and Driveway APWA Type 3, 2 inches Asphalt with testing	TON	223		\$
Fly ash Stabilization, 9 inches deep, 15%	SF	16823		\$
Parking Signage	EA	8		\$
Parking Lot Striping	LS	1		\$
Parking Stops	EA	40		\$
North Shelter/Restroom	LS	1		\$
Bioswale Soils	CY	105		\$
Bioswale Drain Pipe	LF	175		\$
Bioswale Plantings (1 Gal.)	EA	468		\$
Turf Type Tall Grass Seeding	SF	48,900		\$
Parking Lot Lighting, Electrical and Base	LS	1		\$
2" Copper Water Service Line	LF	9		\$
2" HDPE Water Service Line	LF	235		\$

Water Meter Pit	LS	1		\$
Connection to Existing Water Main	LS	1		\$
Water Service Stub for Bathroom	LS	1		\$
Flush Mount Hydrant	LS	1		\$
Sanitary Sewer Service Line	LF	80		\$
Connect to Existing Sanitary Sewer Main	LS	1		\$
Sanitary Sewer Service Stub for Bathroom	LS	1		\$
Sanitary Sewer Cleanout	EA	3		\$
Aggregate at North Shelter	SF	112		\$
Heater in one bathroom, and heat tape in mechanical room	LS	1		\$
SUBTOTAL NORTH PLAYGROUN BASE BID	\$			

4. AMPHITHEATER/Montrose Flush Building Rough-in	Units	Estimated Quantities	\$/Units	Total
Asphalt Trail around parking lot	TON	76		\$
Parking Lot, and Driveway APWA Type 1, 4 inches Asphalt with testing	TON	442		\$
Parking Lot and Driveway APWA Type 3, 2 inches Asphalt with testing	TON	224		\$
Fly Ash Stabilization 9 inches deep, 15%	SF	17891		\$
Parking Signage	EA	4		\$
Parking Lot Striping	LS	1		\$

Second Revised Bid Proposal Form E 4/27/2018

Parking Stops	EA	33	\$
	LA		φ
Amphitheater	LS	1	\$
Drain Pipe at Amphitheater	LF	42	\$
Parking Lot Lighting, Electrical and Base	LS	1	\$
Turf Type Tall Grass Seeding	SF	33,000	\$
Bioswale Soils	CY	85	\$
Bioswale Drain Pipe	LF	150	\$
Bioswale Plantings (1 Gal.)	EA	372	\$
Water Meter Pit	LS	1	\$
Connection to Existing Water Main	LS	1	\$
Water Service Stub for Bathroom	LS	1	\$
Flush Mount Hydrant	LS	1	\$
Sanitary Sewer Service Line	LF	290	\$
Connect to Existing Sanitary Sewer Main	LS	1	\$
Sanitary Sewer Service Stub for Bathroom	LS	1	\$
Sanitary Sewer Cleanout	EA	3	\$
Limestone Seating Wall/Retaining Wall Behind Amphitheater	LF	86	\$
Dry Fire Hydrant	LS	1	\$
Over-excavation of Amphitheater foundation	CY	260	\$
Placement of Engineered Fill for Amphitheater foundation	CY	260	\$

South Bathroom Concrete	CY	998		\$
Asphalt Trail Behind Amphitheater	TON	78		\$
Connection to the Front of the Amphitheater	CY	20		\$
2" Copper Water Service Line (Bored)	LF	75		\$
2" HDPE Water Service Line	LF	270		\$
Concrete Sidewalk along Johnston Dr.	CY	9		\$
Concrete Parking Lot Driveway	CY	25		\$
Aggregate at Amphitheater	SF	365		\$
SUBTOTAL AMPHITHEATER/Mo BASE BID	\$			

5. PARK SIGNAGE/SITE FURNISHINGS	Units	Estimated Quantities	\$/Units	Total
Trash Receptacles	EA	6		\$
Bike Racks	EA	3		\$
Entry Signage	EA	2		\$
Limestone Seating Blocks	EA	36		\$
SUBTOTAL PARK SIGNAGE/S BASE BID	\$			

6. Project wide				
Mobilization, Bonds and Insurance	LS	1		\$
Construction Staking	LS	1		\$
Temporary Traffic Control	LS	1		\$

Page 6 of 7

Second Revised Bid Proposal Form E 4/27/2018

Concrete Testing	EA	20		\$
Site Clearing and Grubbing	AC	4		\$
Site Grading	CY	13,246		\$
Erosion Control	LS	1		\$
SUBTOTAL Project Wide	\$			

TOTAL PROJECT BASE BIDS	Total
1. MDC Reimbursed Improvements	\$
2. Lake Loop Trail	\$
3. North Playground/Parking	\$
4. Amphitheater/Montrose Flush Building Rough-in	\$
5. Park Signage / Site Furnishings	\$
6. Project wide	\$
TOTAL	\$

Total Bid for Project Number: 18-253-201

(\$_____)

In blank above write out the sum of the bid total.

BID OF:_____

(Firm Name)

DATE: _____

SECOND REVISED Appendix A 4/27/2018

Methods and Payments

1. MDC Reimbursed Improvements

8' Width Asphalt Loop Trail / Sub-Base: 4" APWA Type 1 Recycled Asphalt with 2" APWA Type 3 Surface Course and 2" AB3 Base Course: 4" APWA Type 1 Recycled Asphalt (30% maximum recycled material) and 2" APWA Type 3 Surface shall be measured and paid per ton or tenth part thereof of. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt over the AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

18'x36' ADA Fishing Dock: Prefabricated ADA Fishing Doc. This item will be a lump sum item.

45' ADA Walkway: Prefabricated ADA Walkway connecting to ADA Fishing Dock. This item will be a lump sum item.

45' Stiff Arm: Prefabricated stiff arm and foundation tie-ins. This item will be a lump sum item.

Concrete Bulkheads & Material: Concrete bulkheads shall be paid for at the unit bid price each. This item shall include all the equipment, labor and materials for connections with fishing dock and stiff arm.

Delivery and Installation: Delivery and installation by fabrication company for ADA fishing Dock, 45' ADA Walkway, and 45' Stiff Arms necessary to deliver complete dock structure. This item will include any restoration necessary from the transportation inside the park or measures to prevent any damage. This item will be a lump sum item.

Montrose Flush Building: City to furnish prefabricated CXT Montrose Multi-User Flush Building with standard simulated cedar shake roof and barn wood wall texture, three 16-gauge galvanized steel doors and frames, vitreous china plumbing fixtures (2-lavatories, 3 water closets, 1 urinal), three 3-roll toilet paper holders, two exhaust fans, three GFI outlets, five floor drains, two S/S mirrors, ADA grab bars, ADA signs, one hose bib in chase area, and motion controlled interior lights and photocell controlled exterior lights. Missouri state engineered sealed drawings. Including freight/delivery to site, crane, off loading and setting of the building on contractor prepared accessible site.

Montrose Flush Building Installation: This line item is to prepare the site for the building delivery. This line item includes the 6" of AB3 to be placed under the building. The AB3 material shall be installed in 2 separate lifts of 3" each when completely compacted. The material should be placed level and compacted to support a minimum of 1500 pounds per square foot. The plumbing stub in is covered under separate line items. Contractor to have electrical and plumber on site during

delivery to make field adjustments as necessary and provide all electrical and plumbing connections to building once set. Contractor to refer to specification section 13 0000 for installation instructions and tie in locations. The building can be delivered in the roadbed of the driveway and parking lot.

8' Width Concrete / Stone Edge Fishing Pier: Concrete / stone edge fishing pier will be paid on a lump sum basis. All leveling material, jointing, and incidental work shall be included in the lump sum contract price.

(Alternate 1) 8' Concrete Trail/Sub-Base: 6" sidewalks will be measured per cubic yard or tenth part thereof. All leveling material, jointing, and incidental work shall be included in the unit price. AB3 is subsidiary to the line item.

(Alternate 2) 10' Concrete Trail/Sub-Base: 6" Sidewalks will be measured per cubic yard or tenth part thereof. All leveling material, jointing, and incidental work shall be included in the unit price. AB3 is subsidiary to the line item.

(Alternate 3) 10' Asphalt Trail/Sub-Base: 4" APWA Type 1 Recycled Asphalt with 2" Type 3 Surface Course and 2" AB3 Base Course: 4" APWA Type 1 Recycled Asphalt (30% maximum recycled material) with 2" APWA Type 3 Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place asphalt a minimum of 6" thick over the AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

2. Lake Loop Trail

8' Width Asphalt Loop Trail / Sub-Base: 4" APWA Type 3 Asphalt Surface Course with 2" AB3 Base Course: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the 2" AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

6' x 40' Pony Truss Pedestrian Bridge and Abutment: Contractor to furnish and install Contech or approved equal Weathering Steel Pony Truss Pedestrian Bridge w/ pine decking – 40' Span x 6' width. Lump Sum price to include freight/delivery, installation of all abutments, materials and bridge components.

Trail Water Crossing: The payment for this item will be per linear foot of 30" pipe installed. The cost shall include all bedding material, labor and equipment. The cost shall also include the four end sections to the pipes.

8' Connection to Boardwalk: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the 2" AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt

delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

Superstructure to Boardwalk: This item will be a lump sum item for all parts to construct the boardwalk above the piers. This line item will include the connection component to the piers. This item shall include all labor, materials and equipment to complete the superstructure of the boardwalk.

Substructure to Boardwalk: This item will be paid for by the linear foot of piling installed. The substructure will be helical piers to be designed for an estimated a dead load of 15PSF, live load of 80PSF and lateral loading of 50PSF. These estimated loads should be used for bidding purposes. Final loads will be provided by the City prior to shop drawing submittal.

Alternate 1. 8' Concrete Trail/Sub-Base: 5" sidewalks with 4" of AB3 will be measured per cubic yard or tenth part thereof. All leveling material, jointing, and incidental work shall be included in the unit price. AB3 is subsidiary to the line item.

Alternate 2. 10' Concrete Trail/Sub-Base: 5" sidewalks with 4" of AB3 will be measured per cubic yard or tenth part thereof. All leveling material, jointing, and incidental work shall be included in the unit price. AB3 is subsidiary to the line item.

Alternate 3. 10' Asphalt Trail/Sub-Base: 4" APWA Type 3 Asphalt Surface Course with 2" AB3 Base Course: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the 2" AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

Alternate 4. Removed

Alternate 5. Removed

Alternate 6. 8' x 40' Pony Truss Pedestrian Bridge and Abutment: Contractor to furnish and install Contech or approved equal Weathering Steel Pony Truss Pedestrian Bridge w/ pine decking – 40' Span x 8' width. Lump Sum price to include freight/delivery, installation of all abutments, materials and bridge components.

3. North Playground/Parking

Demolition of Asphalt Pavement: Payment will be made at lump sum. This item shall include removal from the property subsidiary to the item. If overexcavation occurs, the fill shall be engineered fill or equal. It is a one time payment and will not be paid for additional demolition

Concrete Shelter Surround: Concrete pavement around the north shelter will be measured per cubic yard or tenth part thereof. All AB3 leveling material, grading and incidental work shall be included in the unit price.

Asphalt Paving Around Parking Lot: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the 2" AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

Parking Lot Paving: 4" APWA Type 1 Recycled Asphalt Base Course: 4" APWA Type 1 Recycled Asphalt Surface (30% maximum recycled material) shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over 9" fly ash stabilization. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements.

Parking Lot Paving: 2" APWA Type 3 Asphalt Surface Course: 2" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 2" thick over the asphalt Sub-Base and 9" fly ash stabilization. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements.

Fly Ash Stabilization: Payment will be made square foot at a 9 inch depth. All incidental construction shall be included in the square foot price. This line item will also include all testing to meet geotechnical recommendations.

Parking Signage: The cost per each unit bid price shall include all labor, equipment, signage, connectors, posts and setting materials required to place.

Parking Lot Striping: The lump sum unit bid price shall include all labor, equipment and materials required to place approximately 1030 linear feet of striping.

Parking Stops: The cost per each unit bid price shall include all labor, equipment and materials required to place.

North Shelter/Restroom: The lump sum price shall include all labor, equipment and materials required to place concrete foundations, furnish and install prefabricated Poligon or approved equal structure, restrooms and storage area, fireplace, service counters, grills, signage, picnic tables, and aggregate apron.

Bioswale Soils: The cost per cubic yard bid price shall include all labor, equipment and materials required to place cubic yards or tenth part thereof for bioswale soils meeting drawing specifications.

Bioswale Drain Pipe: Drain line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Bioswale Plantings (1 Gal.): The cost per each unit bid price shall include all labor, equipment and materials required to place bioswale plantings in the prepared parking area bioswales.

Turf Type Tall Grass Seeding: Seeding will be measured per square foot. Seeding will be measured complete, in-place, to the nearest square foot. No measurement will be made in areas that are not grassed, such as street paving, driveways, parking areas, gardens, and sidewalks. Areas that are disturbed which lie outside the Contractor's seeding limits, as defined by the Plans or Contract Documents, will not be measured for payment, but shall be restored to a condition equal to or better than that existing prior to construction.

Parking Lot Lighting, Electrical and Base: The lump sum unit bid price shall include all labor, equipment and materials required to place parking light bases, poles and fixtures and run necessary electrical connections. Subsidiary to this line item, the electrical contractor shall be responsible for electrical connections from utility transformer to power distribution equipment, and from power distribution equipment to all electrical devices including but not limited to parking lot lighting, receptacles, and exterior lights.

2" Copper Water Service Line: 2" Copper service line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

2" HDPE Water Service Line: 2" HDPE water service line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Water Meter Pit: The total lump sum price shall include all labor, equipment and materials required to place.

Connection to Existing Water Main: The total lump sum price shall include all labor, equipment and materials required to place.

Water Service Stub for Bathroom: The total lump sum price shall include all labor, equipment and materials required to place.

Flush Mount Hydrant: The total lump sum price shall include all labor, equipment and materials required to place.

Sanitary Sewer Service Line: Sewer line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Connect to Existing Sanitary Sewer Main: The total lump sum price shall include all labor, equipment and materials required to place.

Sanitary Sewer Service Stub for Bathroom: The lump sum cost shall include all labor, equipment and materials required to place.

Sanitary Sewer Cleanout: The cost per each unit bid price shall include all labor, equipment and materials required to place.

Aggregate at the North Shelter: Aggregate at the base of the north shelter will be measured per square foot or tenth part thereof. All grading and incidental work shall be included in the price per square foot. Aggregate should be a minimum of 6 inches thick.

Heater in one bathroom plus heat tape in mechanical room: This lump sum will pay for a 10,000 BTU heat with a minimum horizontal throw of 9 linear feet. The price will include wiring to electrical panel, thermostat, and all other parts to install. The water lines from the mechanical room to the bathroom must have heat tape installed to prevent freezing. The price will also include an exhaust vent with all necessary parts and wiring to operate as needed.

4. Amphitheater/Montrose Flush Building Rough-in

Asphalt Paving Around Parking Lot: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the 2" AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

Parking Lot Paving: 4" APWA Type 1 Recycled Asphalt Base Course: 4" APWA Type 1 Recycled Asphalt Surface (30% maximum recycled material) shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over 9" fly ash stabilization. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements.

Parking Lot Paving: 2" APWA Type 3 Asphalt Surface Course: 2" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 2" thick over the asphalt Sub-Base and 9" fly ash stabilization. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements.

Fly Ash Stabilization: Payment will be made square foot at a 9 inch depth. All incidental construction shall be included in the square foot price. This line item will also include all testing to meet geotechnical recommendations.

Parking Signage: The cost per each unit bid price shall include all labor, equipment and materials required to place.

Parking Lot Striping: The lump sum unit bid price shall include all labor, equipment and materials required to place.

Parking Stops: The cost per each unit bid price shall include all labor, equipment and materials required to place approximately 765 linear feet of striping.

Amphitheater: The lump sum price shall include all labor, equipment and materials required to place concrete foundations, furnish and install prefabricated Poligon or approved equal structure, movable screens, concrete seat walls, sliding aluminum screens and aggregate apron.

Drain Pipe at Amphitheater: Drain line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Parking Lot Lighting, Electrical and Base: The lump sum unit bid price shall include all labor, equipment and materials required to place parking light bases, poles and fixtures and run necessary electrical connections. Subsidiary to this line item, the electrical contractor shall be responsible for electrical connections from utility transformer to power distribution equipment, and from power distribution equipment to all electrical devices including but not limited to parking lot lighting, receptacles, and exterior lights.

Turf Type Tall Grass Seeding: Seeding will be measured per square foot. Seeding will be measured complete, in-place, to the nearest square foot. No measurement will be made in areas that are not grassed, such as street paving, driveways, parking areas, gardens, and sidewalks. Areas that are disturbed which lie outside the Contractor's seeding limits, as defined by the Plans or Contract Documents, will not be measured for payment, but shall be restored to a condition equal to or better than that existing prior to construction.

Bioswale Soils: The cost per cubic yard bid price shall include all labor, equipment and materials required to place cubic yards or tenth meeting drawing specifications.

Bioswale Drain Pipe: Drain line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Bioswale Plantings (1 Gal.): The cost per each unit bid price shall include all labor, equipment and materials required to place bioswale plantings in the prepared parking area bioswales.

Water Meter Pit: The total lump sum price shall include all labor, equipment and materials required to place.

Connection to Existing Water Main: The total lump sum price shall include all labor, equipment and materials required to place.

Water Service Stub for Bathroom: The total lump sum price shall include all labor, equipment and materials required to place.

Flush Mount Hydrant: The total lump sum price shall include all labor, equipment and materials required to place.

Sanitary Sewer Service Line: Sewer line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Connect to Existing Sanitary Sewer Main: The total lump sum price shall include all labor, equipment and materials required to place.

Sanitary Sewer Service Stub for Bathroom: The total lump sum price shall include all labor, equipment and materials required to place.

Sanitary Sewer Cleanout: The cost per each unit bid price shall include all labor, equipment and materials required to place.

Limestone Wall behind Amphitheater: The cost per linear foot bid price shall include all AB3 leveling material, grading, labor, equipment and materials required to place.

Dry Fire Hydrant: The lump sum cost shall include all part of the hydrant, plumbing and lines to connect the dry hydrant to the lake. It shall also cover the cost of the concrete pad around the hydrant.

Amphitheater Over Excavation: This is to remove all unsuitable materials from under the amphitheater. The City will make a final determination of how much material must be removed from the site. This will be measured by the cubic yard.

Engineered Fill Replacement: This is to replace the over excavated material. This will be measured by the cubic yard. The contractor is responsible for finding appropriate backfill soil and have proper testing by a geotech engineer for soil type and installation testing.

Concrete Restroom Surround: Concrete pavement around the MDC Restroom will be measured per cubic yard or tenth part thereof. All AB3 leveling material, grading and incidental work shall be included in the price per cubic yard.

8' Asphalt Trail behind Amphitheater: 4" APWA Type 3 Asphalt Surface shall be measured and paid per ton or tenth part thereof for the specified depth. The unit price for this item shall include all equipment, labor and materials to place a surface course of asphalt a minimum of 4" thick over the AB3 Sub-Base. The unit price for this item shall include all the equipment, labor and materials to verify that the asphalt delivered to the project conforms to the Marshall properties of the mix design and that the compacted density of the asphalt mat meets the project requirements. AB3 is subsidiary to the line item.

8' Connection to the Front of the Amphitheater: 5" sidewalks with 4" of AB3 will be measured per cubic yard or tenth part thereof. All leveling material, jointing, and incidental work shall be included in the unit price. AB3 is subsidiary to the line item.

2" Copper Water Service Line (Bored): 2" Copper service line will be paid on a linear foot method. Boring shall be included in the cost per linear foot of pipe per each size and type.

2" HDPE Water Service Line: 2" HDPE water service line will be paid on a linear foot method. Excavation, bedding, and backfill shall be included in the cost per linear foot of pipe per each size and type.

Concrete Sidewalk Along Johnston Drive: This is the sidewalk along the street. This will be paid for in cubic yards and built to city sidewalk standards.

Concrete Parking Lot Driveway: 8" non-reinforced concrete pavement into the south parking lot will be measured per cubic yards or tenth part thereof. All AB3 leveling material, grading and incidental work shall be included in the price per square foot.

Aggregate at the Amphitheater: Aggregate at the base of the amphitheater will be measured per square foot or tenth part thereof. All grading and incidental work shall be included in the price per square foot. Aggregate should be a minimum of 6 inches thick.

5. Park Signage/Site Furnishings

Trash Receptacles: The cost per unit bid price shall include all labor, equipment and materials required to place.

Bike Racks: The cost per unit bid price shall include all labor, equipment and materials required to place.

Entry Signage: The cost per unit bid price shall include all labor, equipment and materials required to place.

Limestone Seating Blocks: The cost per unit bid price shall include all labor, equipment and materials required to place.

6. Project wide

Mobilization, Bonds, and Insurance: Mobilization, Bonds and Insurance will be considered a lump sum item for payment. The total lump sum price for this item shall not exceed 5% of the total base bid price. Payment shall be made on the schedule enclosed in the bid documents.

Construction Staking: Construction and Survey Controls shall be paid for as a lump sum item. The unit cost for this item shall include all labor, equipment and materials to develop and establish necessary control, detail dimensions, slope stakes and measurements required for proper layout and performance of the work. The contractor is responsible for all restaking.

Temporary Traffic Control: Traffic Control shall be considered a lump sum for payment. The unit bid cost for this item shall include all materials, labor and equipment required to provide a safe working environment including, but not limited to, all signage to control traffic through the work area as required by the MUTCD.

Concrete Testing: Concrete Testing shall be paid for at the unit bid price per each. The unit bid price for this line item shall include all labor, materials and equipment required to test the concrete for temperature, slump, air content and compressive strength. Each test shall consist of 4 cylinders to be broken at 7, 14, 21, and 28

days. Tests shall be conducted once per day or as directed by the City's representative.

Site Clearing and Grubbing: Clearing and Grubbing shall be considered a lump sum item for payment. The unit cost for this line item shall include all labor, materials and equipment necessary to prepare the site for construction as per plan. This includes any necessary removal and disposal of any above or underground materials, natural or man-made.

Site Grading: Embankment may be listed in the Contract Documents and measured to determine the quantity in cubic yards or tenth part thereof.

Erosion Control: Including silt fencing, rock ditch and stabilized construction entrances will be paid as a lump sum price. The unit bid price shall include all labor, equipment and materials required to install, maintain, replace when necessary and ultimately remove from plan locations or as where required by the Owner.

HAWK RIDGE PARK

SYMBOLS AND ABBREVIATIONS

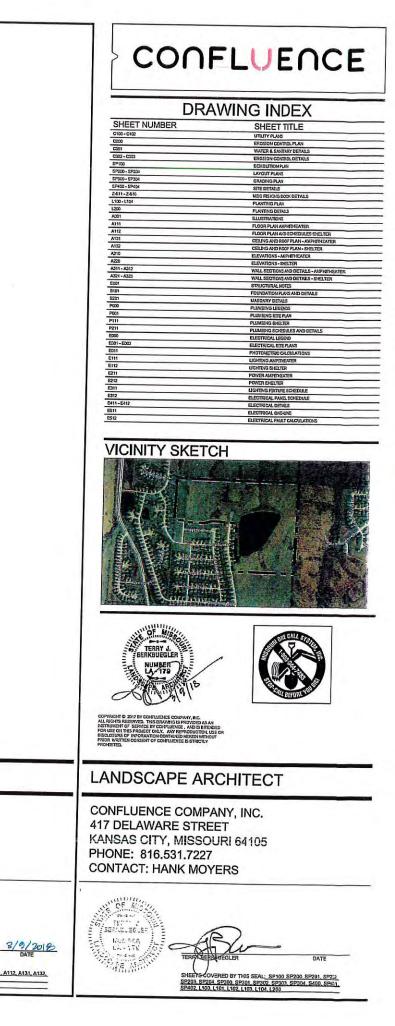
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RUCTURAL ENGINEER	
ROBINSON ST. RLAND PARK, KANSAS 66204 JE: 913.214.2169	

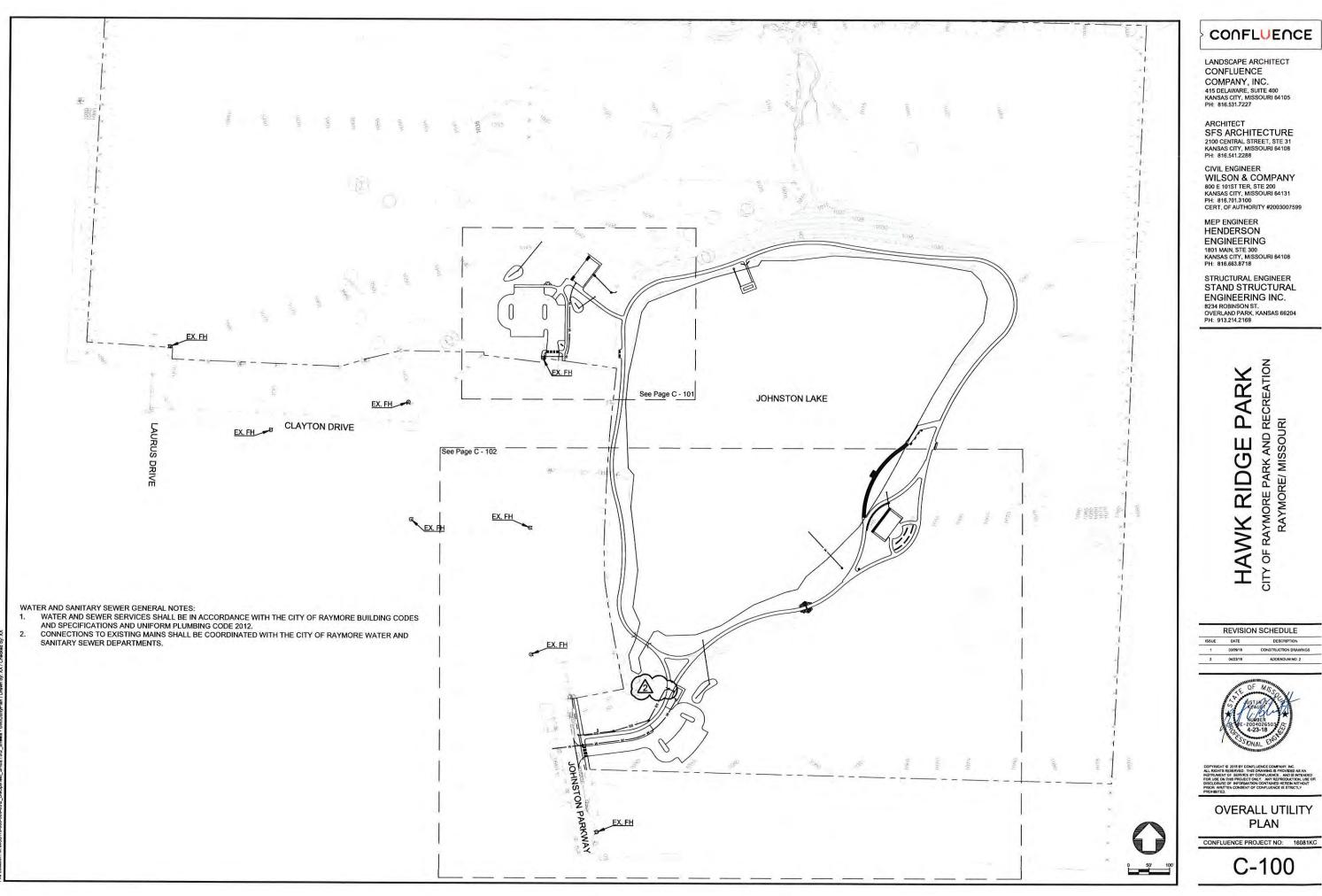
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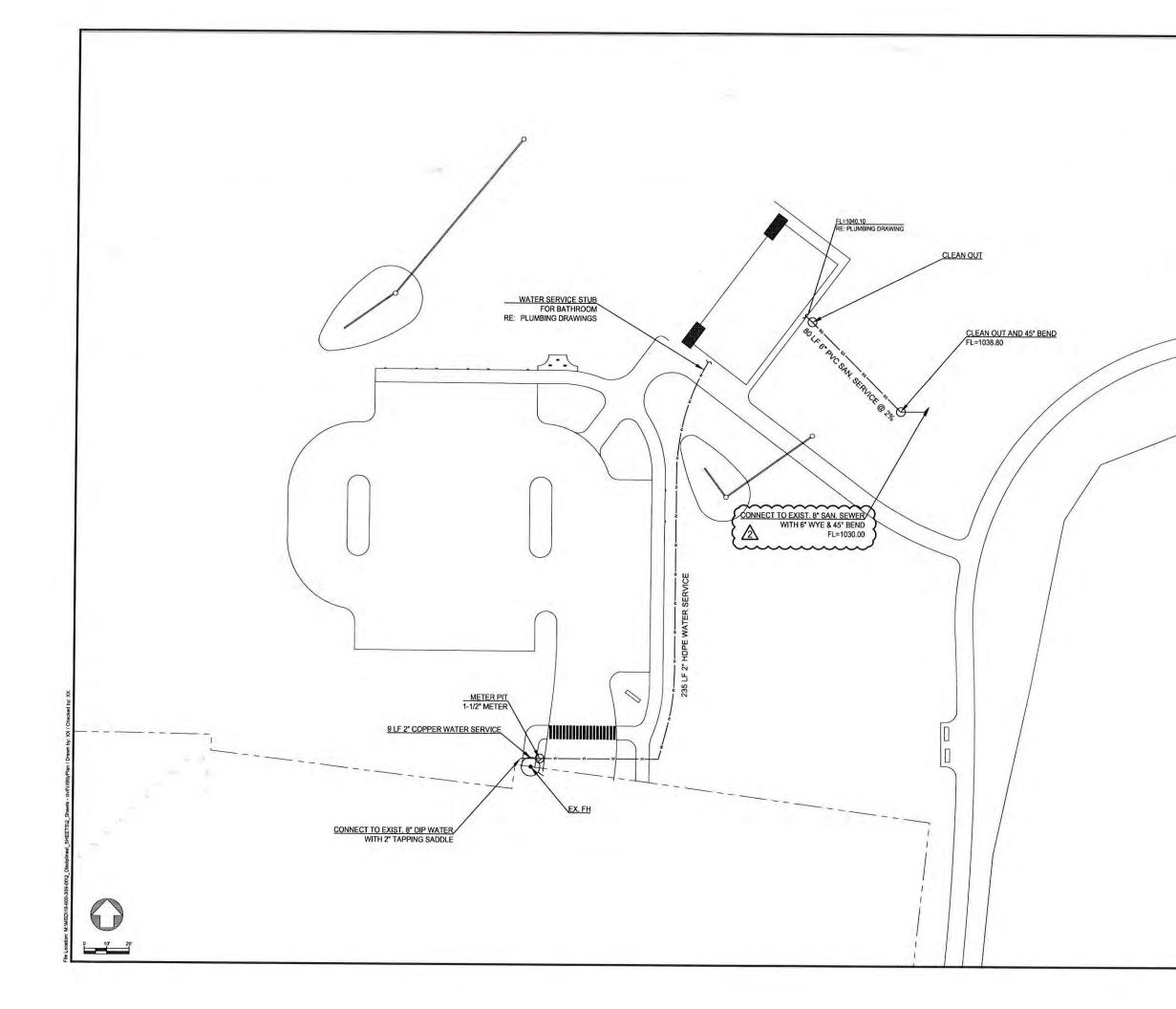
CITY OF RAYMORE RAYMORE / MISSOURI CONFLUENCE PROJECT NO: 16081KC



gn/16081KC	STRUCTURAL ENGINEER MEP ENGINEER		CIVIL ENGINEER	ARCHITECT	
KC - Hawk Ridge Part: Improvements Desk	STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PHONE: 913.214.2169 CONTACT: MATT ENSTROM	HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PHONE: 816.663.8718 CONTACT: MARCUS PERRY	WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PHONE: 816.701.3100 CERT. OF AUTHORITY #2003007599 CONTACT: JUSTIN KLAUDT	SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PHONE: 816.541.2288 CONTACT: KWAME SMITH	
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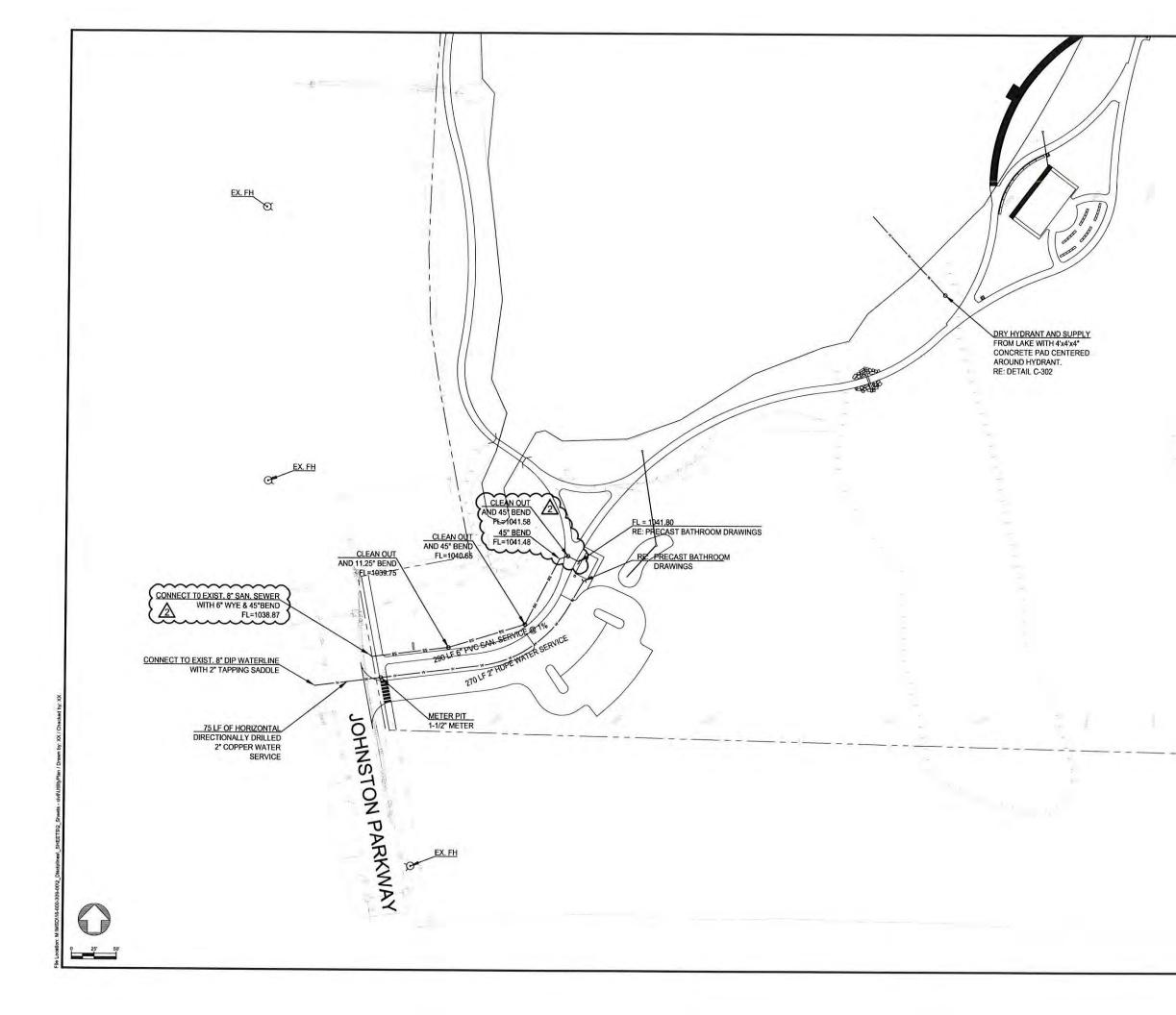
> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI



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UTILITY PLAN NORTH

C-101



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> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI





UTILITY PLAN SOUTH

C-102

C - AFTER BUILDING AND DRIVE CONSTRUCTION SEEDING AND MULCHING

B - MASS GRADING & UTILITY INSTALLATION

PROJECT STAGE

- PRIOR TO LAND DISTURBANCE

EROSION AND SEDIMENT CONTROL NOTES:

199

DRIVE

SEDIMENT FENC

AGING AREA

DITCH CHECK

EROSION AND SEDIMENT CONTROL STAGING CHART

BMP DESCRIPTION

CONSTRUCTION ENTRANCE &

PRIOR TO LAND DISTURBANCE ACTIVITIES, THE CONTRACTOR SHALL: -CONSTRUCT A STABILI ED ENTRANCE/ PARKING/ DELIVERY AREA AND INSTALL ALL PERIMETER SEDIMENT CONTROLS ON THE SITE. -INSTALL AND RE.. UEST THE INSPECTION OF THE PRE CONSTRUCTION EROSION AND SEDIMENT CONTROL MEASURES DESIGNATED ON THE -INSTALL AND RE.. UEST THE INSPECTION OF THE PRE CONSTRUCTION EROSION AND SEDIMENT CONTROL MEASURES DESIGNATED ON THE APPROVED EROSION AND SEDIMENT CONTROL PLAN, LAND DISTURBANCE WORK SHALL NOT PROCEED UNTIL THERE IS A SATISFACTORY

CLAYTON DRIVE

NOTES

SEE NOTE 3 & 4

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REMOVE

AFTER STAGE:

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N/A

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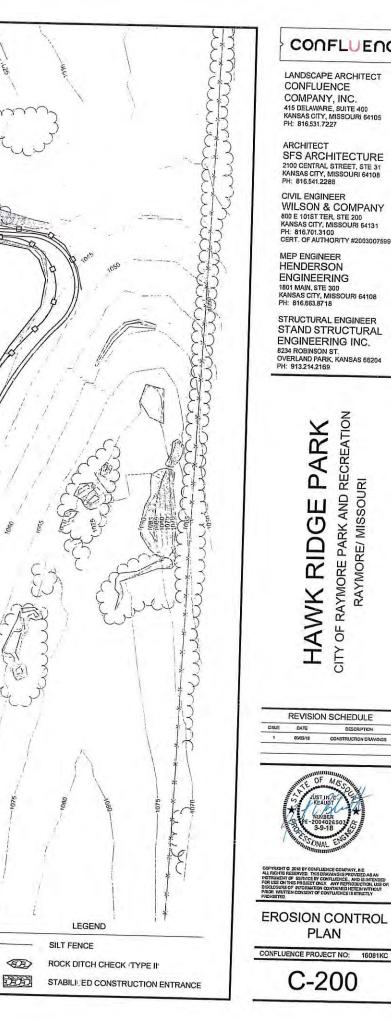
IDENTIFY THE LIMITS OF CONSTRUCTION ON THE GROUND WITH EASILY RECOGNI ABLE INDICATORS SUCH AS CONSTRUCTION STAKING, CONSTRUCTION FENCING, PLACEMENT OF PHYSICAL BARRIERS OR OTHER MEANS ACCEPTABLE TO THE CONTRACTOR AND CITY

EROSION AND SEDIMENT CONTROL DEVICES PROTECTING THE PUBLIC RIGHT-OF-WAY SHALL BE INSTALLED AS SOON AS THE RIGHT-OF-WAY 2.

THE CONTRACTOR SHALL COMPLY WITH ALL RED UIREMENTS OF THE CITY OF RAYMORE AND THE KANSAS CITY METRO CHAPTER OF APWA. THE CONTRACTOR SHALL TEMPORARY SEED, MULCH, OR OTHERWISE STABILIE ANY AREA WHERE THE LAND DISTURBANCE ACTIVITY HAS CEASED FOR MORE THAN 14 DAYS. THE CONTRACTOR MAY USE PERMANENT SEEDING IF SEEDING OCCURS DURING THE CITYS 3.

PERMANENT SEEDING PERIOD. -THE CONTRACTOR SHALL PERFORM INSPECTIONS OF EROSION AND SEDIMENT CONTROL MEASURES AT LEAST ONCE PER MONTH AND WITHIN 24 HOURS FOLLOWING EACH RAINFALL EVENT OF 1/2" OR MORE WITHIN ANY 24-HOUR PERIOD. -THE CONTRACTOR SHALL MAINTAIN AN INSPECTION LOG INCLUDING THE INSPECTOR'S NAME, DATE OF INSPECTION, OBSERVATIONS AS TO THE EFFECTIVENESS OF THE EROSION AND SEDIMENT CONTROL MEASURES, ACTION NECESSARY TO CORRECT DEFICIENCIES WHEN THE DEFICIENCIES WERE CORRECTED, AND THE STATE OF MISSOURI PENEW BY THE CITY OF PAYMORE AND THE STATE OF MISSOURI

- SEE THE LANDSCAPE PLANS FOR PERMANENT SEEDING/SODDING.
- THE CONTRACTOR SHALL MAINTAIN INSTALLED EROSION AND SEDIMENT CONTROL DEVICES IN A MANNER THAT PRESERVES THEIR EFFECTIVENESS FOR PREVENTING SEDIMENT FROM LEAVING THE SITE OR ENTERING A SENSITIVE AREA SUCH AS A NATURAL STREAM CORRIDOR, AREAS OF THE SITE INTENDED TO BE LEFT UNDISTURBED, A STORM SEWER, OR AN ON-SITE DRAINAGE CHANNEL FAILURE TO 5. DO SO IS A VIOLATION OF THE RULES, REGULATIONS, AND STANDARDS ADOPTED AND ESTABLISHED BY THE CITY OF RAYMORE.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING EROSION AND SEDIMENT CONTROL FOR THE DURATION OF A PROJECT. IF THE CITY DETERMINES THAT THE BMP'S IN PLACE DO NOT PROVIDE ADE UATE EROSION AND SEDIMENT CONTROL AT ANY TIME DURING THE PROJECT, THE CONTRACTOR SHALL INSTALL ADDITIONAL OR ALTERNATE MEASURES THAT PROVIDE EFFECTIVE CONTROL. 6.



JOHNSTON LAKE

- SD - 5:

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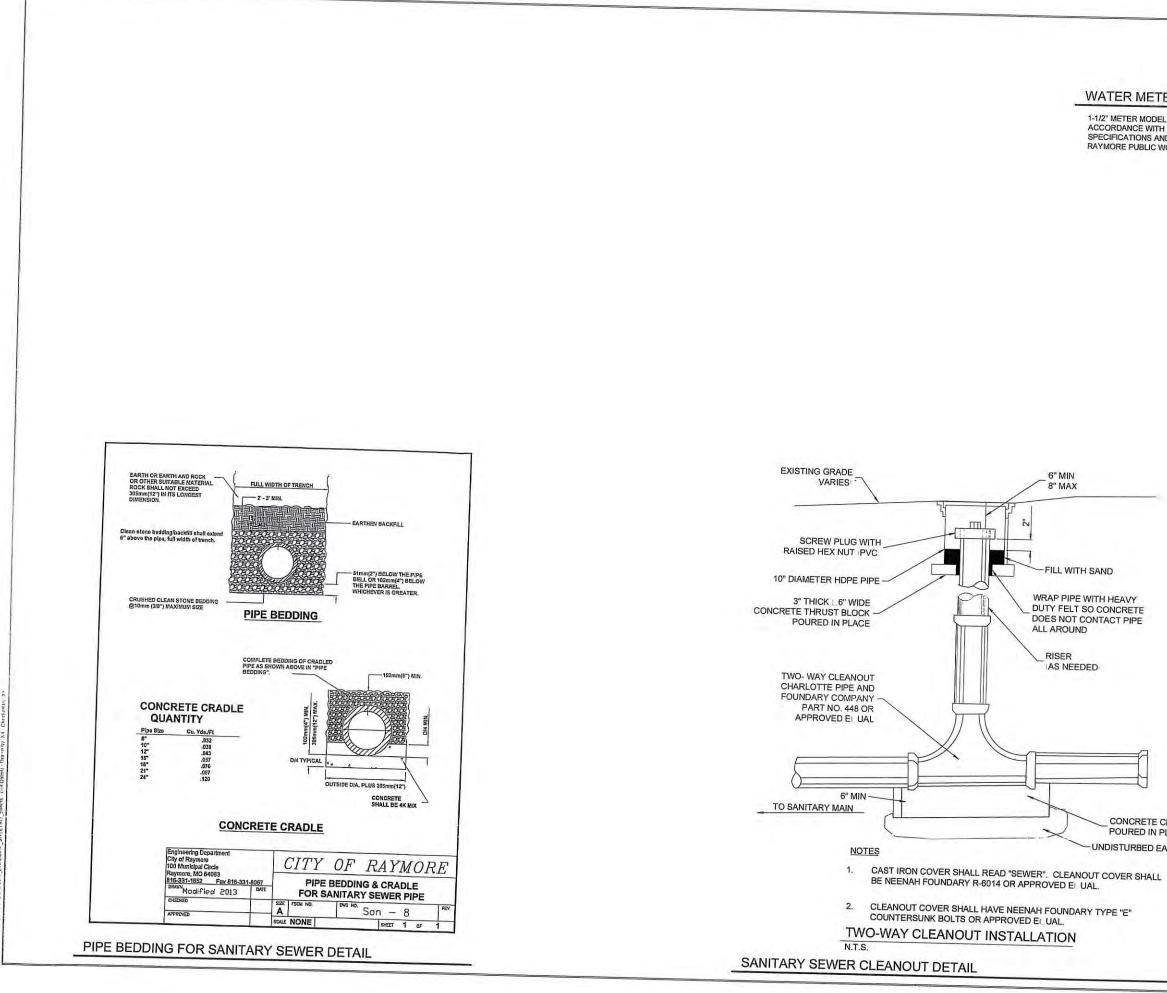
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> RECREATION X A 0 RK AND RE L C ARK RID RAYMORE/ RAYMORE HAWK ЧO CITY



PLAN

C-200



WATER METER SETTING NOTES

1-1/2" METER MODEL AND ASSEMBLY SHALL BE IN ACCORDANCE WITH CITY OF RAYMORE TECHNICAL SPECIFICATIONS AND APPROVED BY THE CITY OF RAYMORE PUBLIC WORKS.

CONFLUENCE

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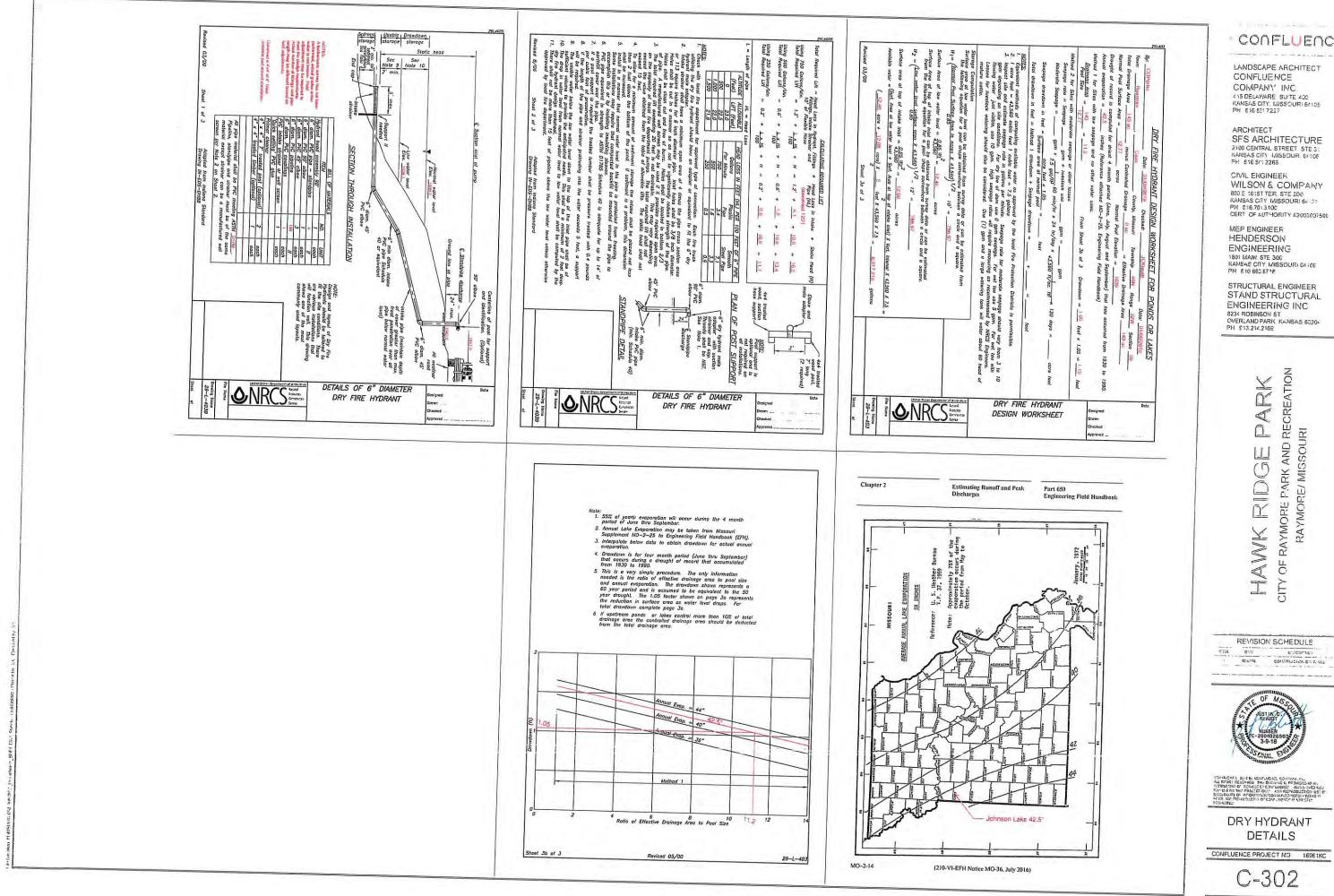
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STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC 8234 ROBINSON ST OVERLAND PARK, KANSAS 65204 PH 913.214 2169





CONCRETE CRADLE POURED IN PLACE -UNDISTURBED EARTH



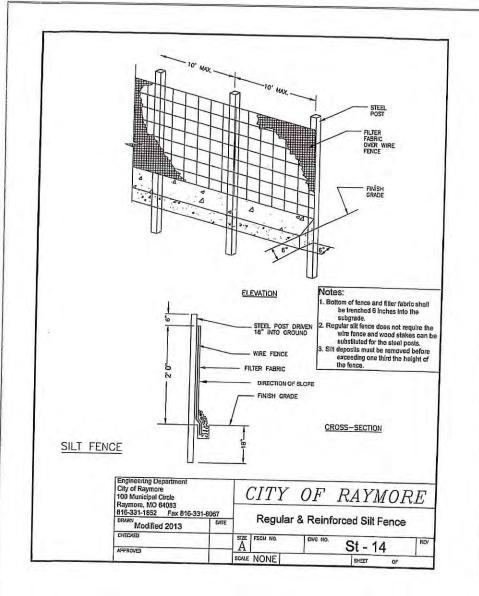
LANDSCAPE ARCHITECT KANSAS CITY, MISSOURI 6/105 PH 816 531 7227

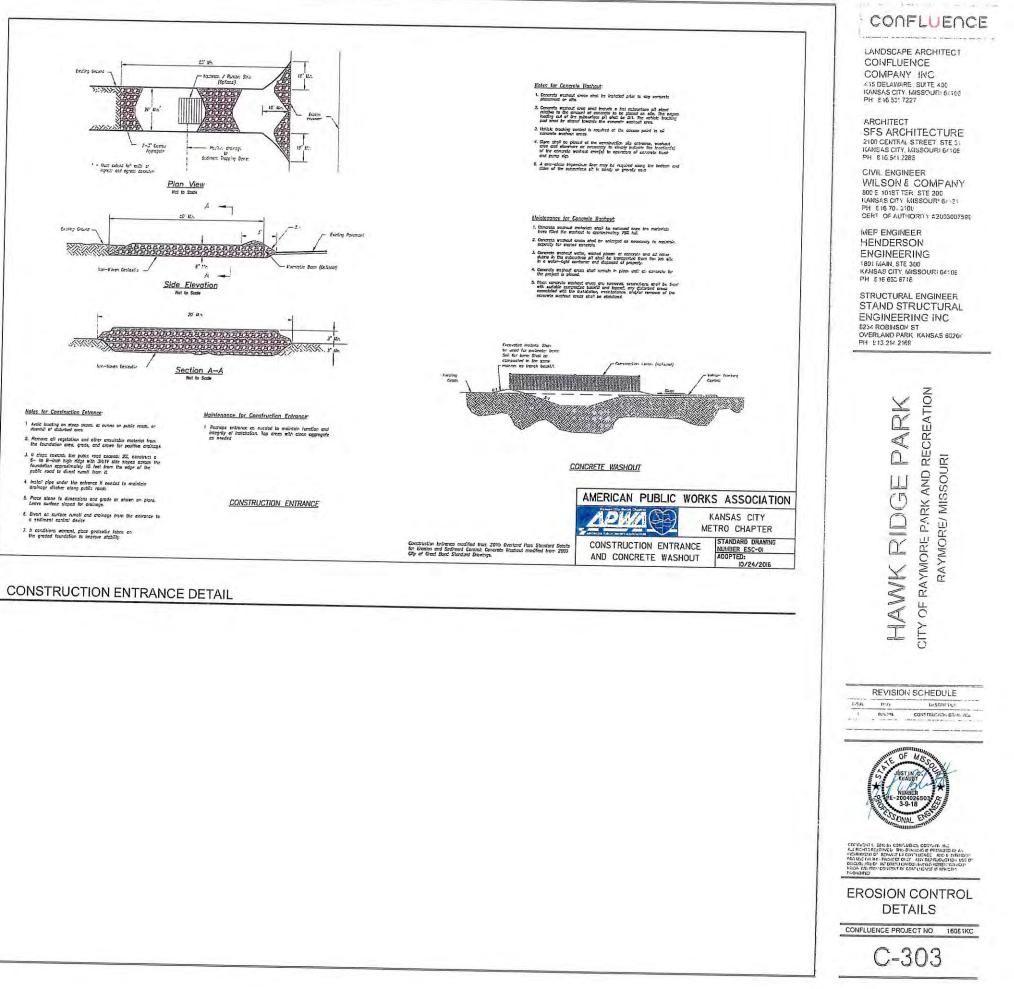
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WILSON & COMPANY 800 E 101ST TER. STE 200 KANSAS CITY MISSOURI 6: 31 CERT OF AUTHORITY #2003007591

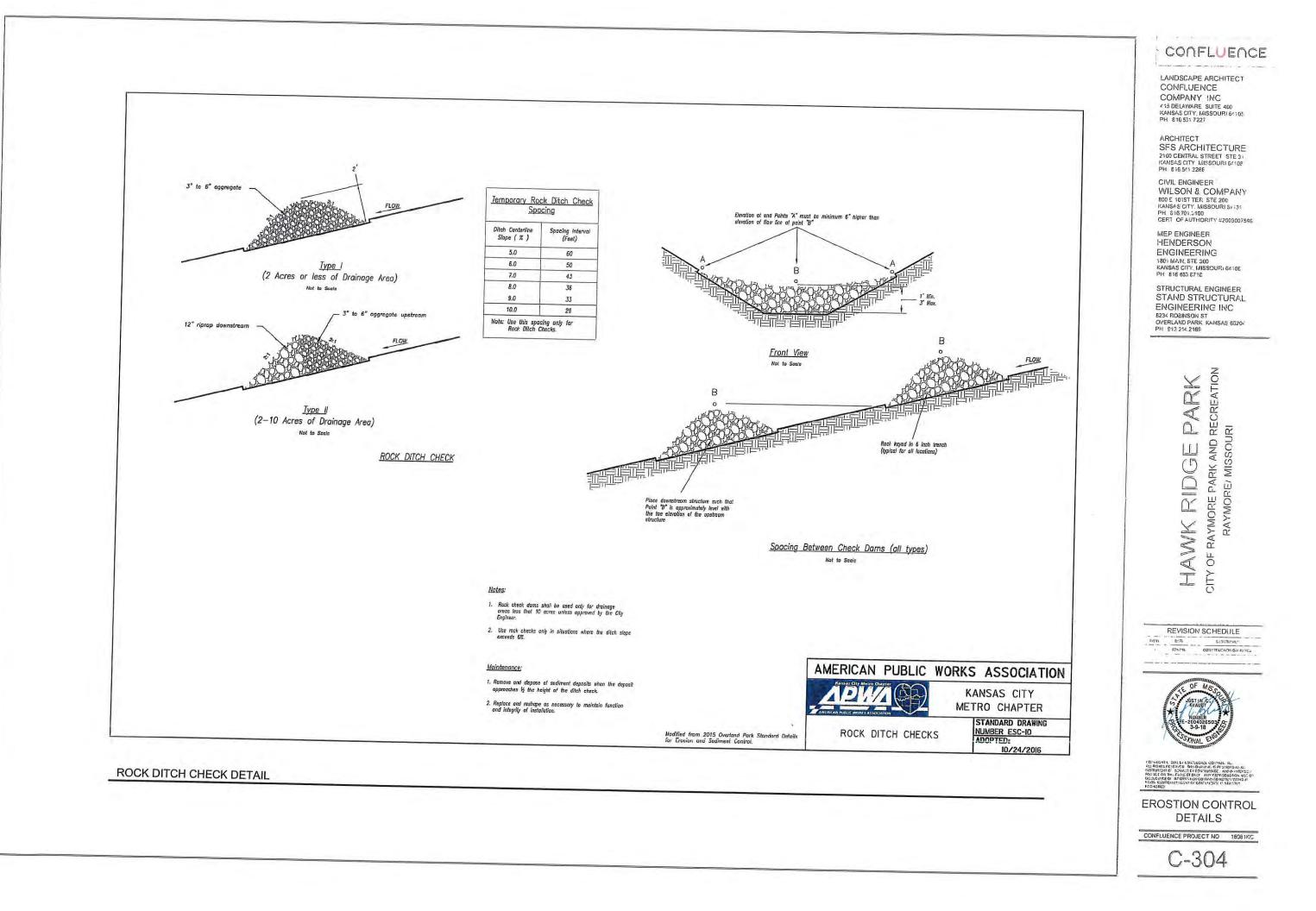
KANSAS CITY, MISSOURI 64108 PH 816 663 8718

STAND STRUCTURAL ENGINEERING INC OVERLAND PARK KANSAS 6020-PH £13.214.2169





SILT FENCE DETAIL



DEMOLITION NOTES:

- 1 CONTRACTOR TO COORDINATE REMOVAL AND/OR STORAGE AND REPLACEMENT OF BARRICADES WITH THE CITY OF RAYMORE
- 2. PRIOR TO ANY EXCAVATION AT THE SITE, CONTRACTOR SHALL EXAMINE ANY APPLICABLE DRAWINGS AVAILABLE FROM THE OWNER AND/OR THE LANDSCAPE ARCHITECT, AND CONSULT WITH OWNER'S PERSONNEL AND UTLITY COMPANIES REPRESENTATIVES TO DETERMINE POSSIBLE UTILITY LOCATIONS AND DEPTHS. NO COMPENSATION WILL BE ALLOWED FOR DAMAGE RESULTING FROM FAILURE TO COMPLY WITH THIS REQUIREMENT.
- 3 CONTRACTORS TO FIELD ADJUST ALL EXISTING SITE UTILITIES TO NEW FINISH GRADES IF NECESSARY EXISTING UTILITIES INCLUDE, BUT ARE NOT LIMITED TO FIRE HYDRANTS, MANHOLE RIMS, INLETS, WATER VALVES AND LIGHT BASES.
- 4. THE CONTRACTOR SHALL VERIFY THE LOCATION AND PROTECT ALL UTILITIES AND STRUCTURES. DAMAGE TO UTILITIES AND STRUCTURES SHALL & REPAIRED BY THE CONTRACTOR TO THE SATISFACTION OF THE OWNER OF THE UTILITIES
- 5 FIELD VERIFY EXISTING GRADES AND LOCATIONS OF EXISTING UTILITIES, CONDUIT, LINES, POLES, TREES, PAVING, BUILDING AND OTHER STRE STRUCTURES PRIOR TO DEMOLITION OR CONSTRUCTION AND IMMEDIATELY INFORM THE CITY OF ANY DISCREPANCIES.
- 6. PRIOR TO REMOVING ANY PLANT MATERIAL NOT INDICATED TO BE PROTECTED OR REMOVED CONTACT THE CITY.
- 7 PROTECT ALL ITEMS WITHIN CONTRACT LIMITS NOT INDICATED TO BE REMOVED NOTIFY THE CITY OF ANY DISCREPANCIES.
- 8. REPORT TO CITY ANY DAMAGE TO EXISTING UTILITIES PRIOR TO REPAIR.
- ALL WORK SHALL BE IN ACCORDANCE WITH OSHA CODES AND STANDARDS. NOTHING INDICATED ON THESE DRAWINGS SHALL RELIEVE THE CONTRACTOR FROM COMPLYING WITH ANY APPROPRIATE SAFETY REGULATIONS.

LEGEND

(A) REMOVE VEGETATION; ANYTHING OVER 2" IN DIAMETER REMOVE TO A DEPTH OF 6"

B REMOVE ASPHALT PAVING TO FULL DEPTH

B

SD -

JUNINS FOR LAST



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ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816,701,3100 CERT, OF AUTHORITY #2003007599

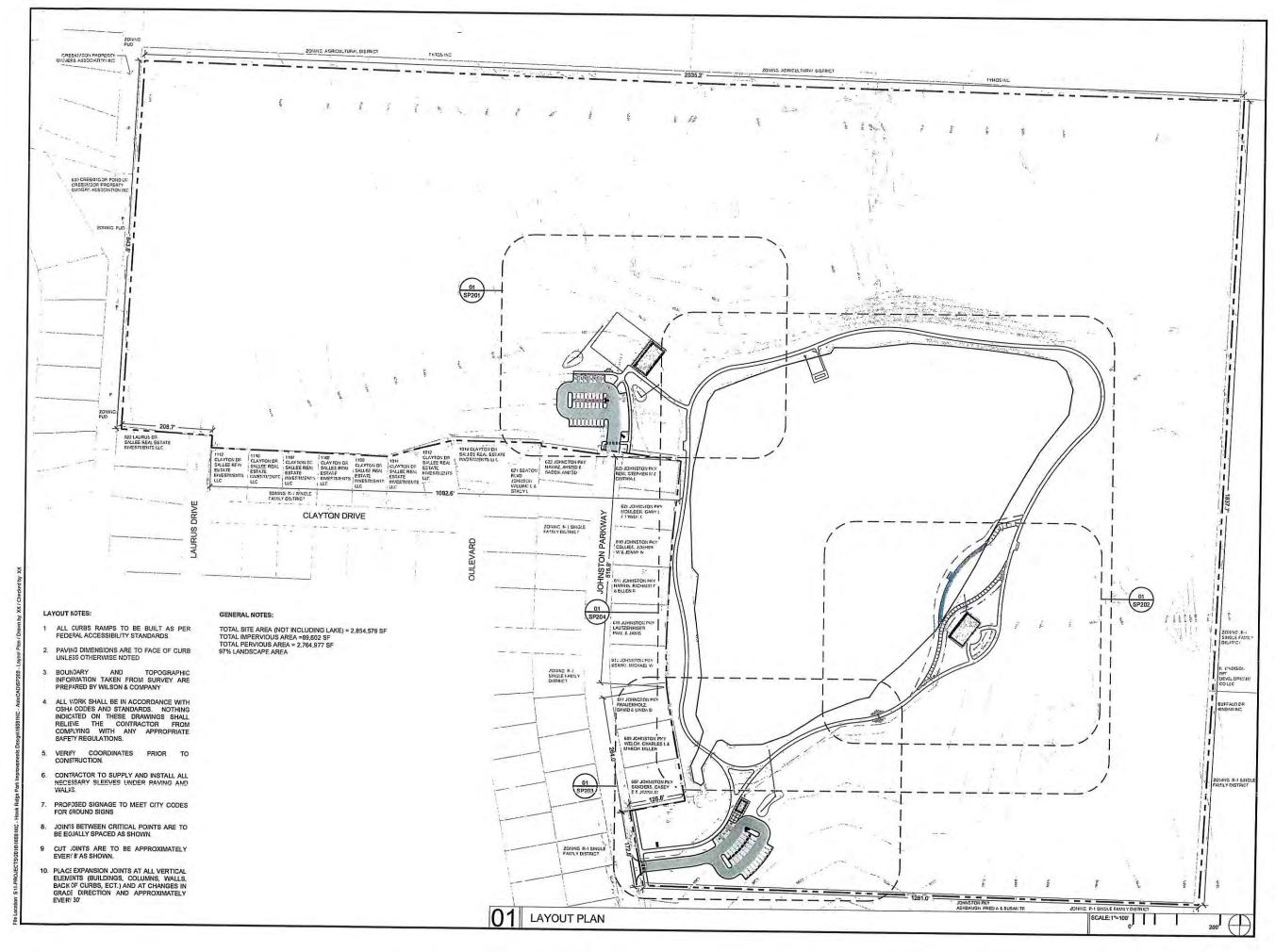
MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI



CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816,531,7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

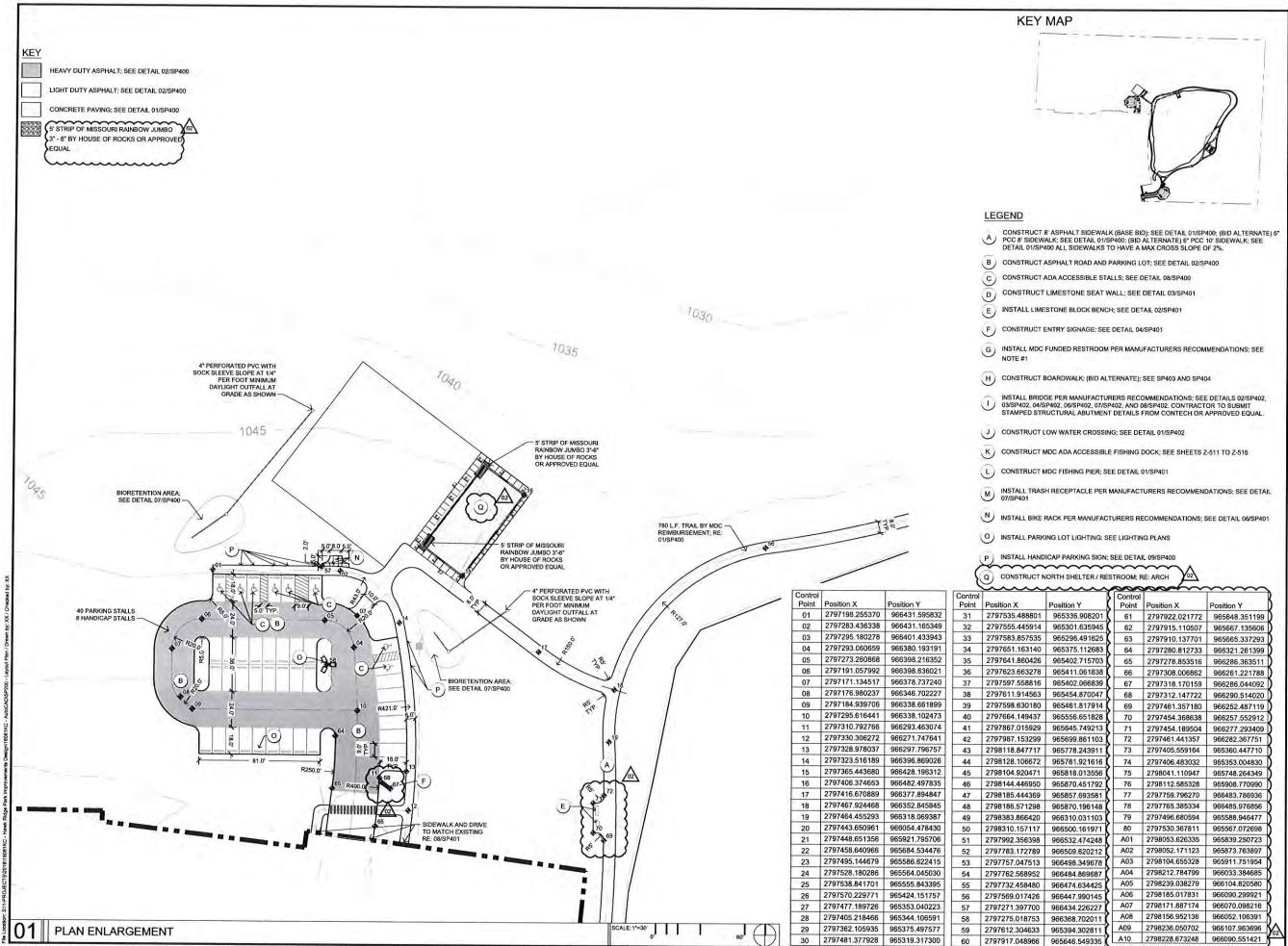






LAYOUT PLAN

CONFLUENCE PROJECT NO: 16081KC



IY	Control Point	Position X	Position Y
6.908201	61	2797922.021772	965648.351199
1.635945	62	2797915.110507	965667.135606
6.491625	63	2797910.137701	965665.337293
5.112683	64	2797280.812733	966321.261399
2.715703	65	2797278.853516	966286.363511
1.061838	66	2797308.006862	966261.221788
2.066839	67	2797318.170159	966286.044092
4.870047	68	2797312.147722	966290.514020
1.817914	69	2797461.357180	966252.487119
6.651828	70	2797454.368638	966257.552912
5.749213	71	2797454.189504	966277.293409
9.861103	72	2797461.441357	966282.367751
3.243911	73	2797405.559164	965360.447710
1.921616	74	2797406.483032	965353.004830
3.013556	75	2798041.110947	965748.264349
0.451792	76	2798112.585328	965908.770990
7.693581	77	2797759.796270	966483.786936
0.196148	78	2797765.385334	966485.976856
0.031103	79	2797496.680594	965588.946477
0.161971	80	2797530.367811	965567.072698
2.474248	A01	2798053.626335	965839.250723
9.620212	A02	2798052.171123	965873.763897
3.349678	A03	2798104.655328	965911.751954
1.869687	A04	2798212.784799	966033.384685
1.634425	A05	2798239.038279	966104.820580
.990145	A06	2798185.017831	966090.299921
226227	A07	2798171.887174	966070.098216
3.702011	A08	2798156.952136	966052.106391
.302811	A09	2798236.050702	966107.963696
5.549335	A10	2798228.673248	966090.551421

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

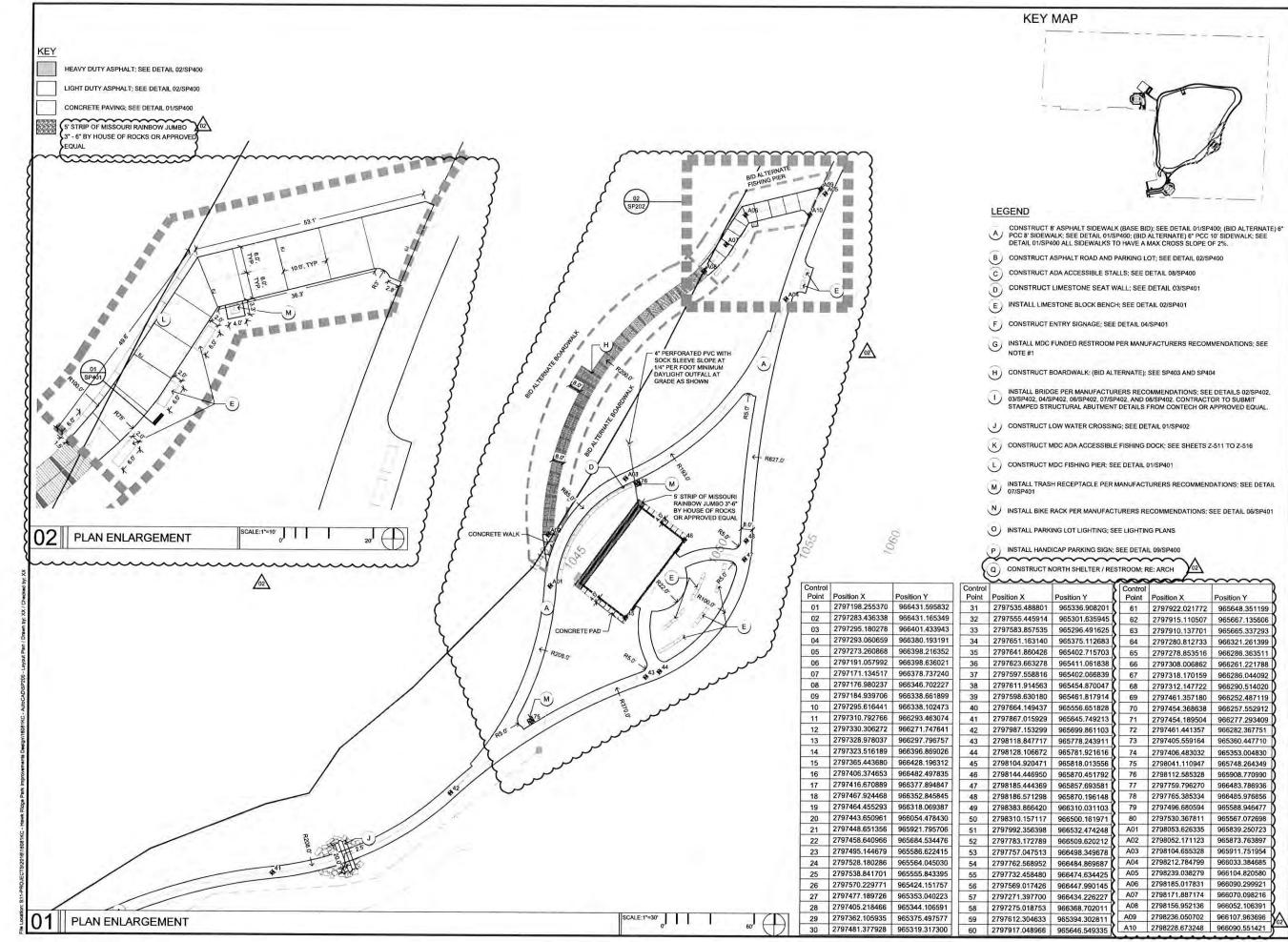
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST OVERLAND PARK, KANSAS 66204 PH: 913.214.2169







~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	
HELTER / RESTROOM; RE: ARCH	02
	1

m	Control	A	
nY S	Point	Position X	Position Y
36.908201	61	2797922.021772	965648.351199
01.635945	62	2797915.110507	965667.135606
6.491625	63	2797910.137701	965665.337293
75.112683	64	2797280.812733	966321.261399
02.715703	65	2797278.853516	966286.363511
11.061838	66	2797308.006862	966261.221788
02.066839	67	2797318.170159	966286.044092
54.870047	68	2797312.147722	966290.514020
61.817914	69	2797461.357180	966252.487119
6.651828	70	2797454.368638	966257.552912
5,749213	71	2797454.189504	966277.293409
9.861103	72	2797461.441357	966282.367751
8.243911	73	2797405.559164	965360.447710
1.921616	74	2797406.483032	965353.004830
8.013556	75	2798041.110947	965748.264349
0.451792	76	2798112.585328	965908.770990
7.693581	77	2797759.796270	966483.786936
0.196148	78	2797765.385334	966485.976856
0.031103	79	2797496.680594	965588.946477
0.161971	80	2797530.367811	965567.072698
2.474248	A01	2798053.626335	965839.250723
9.620212	A02	2798052.171123	965873.763897
8.349678	A03	2798104.655328	965911.751954
4.869687	A04	2798212.784799	966033.384685
4.634425	A05	2798239.038279	966104.820580
7.990145	A06	2798185.017831	966090.299921
4.226227	A07	2798171.887174	966070.098216
8,702011	A08	2798156.952136	966052.106391
4.302811	A09	2798236.050702	966107.963696
6.549335	A10	2798228.673248	966090.551421

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

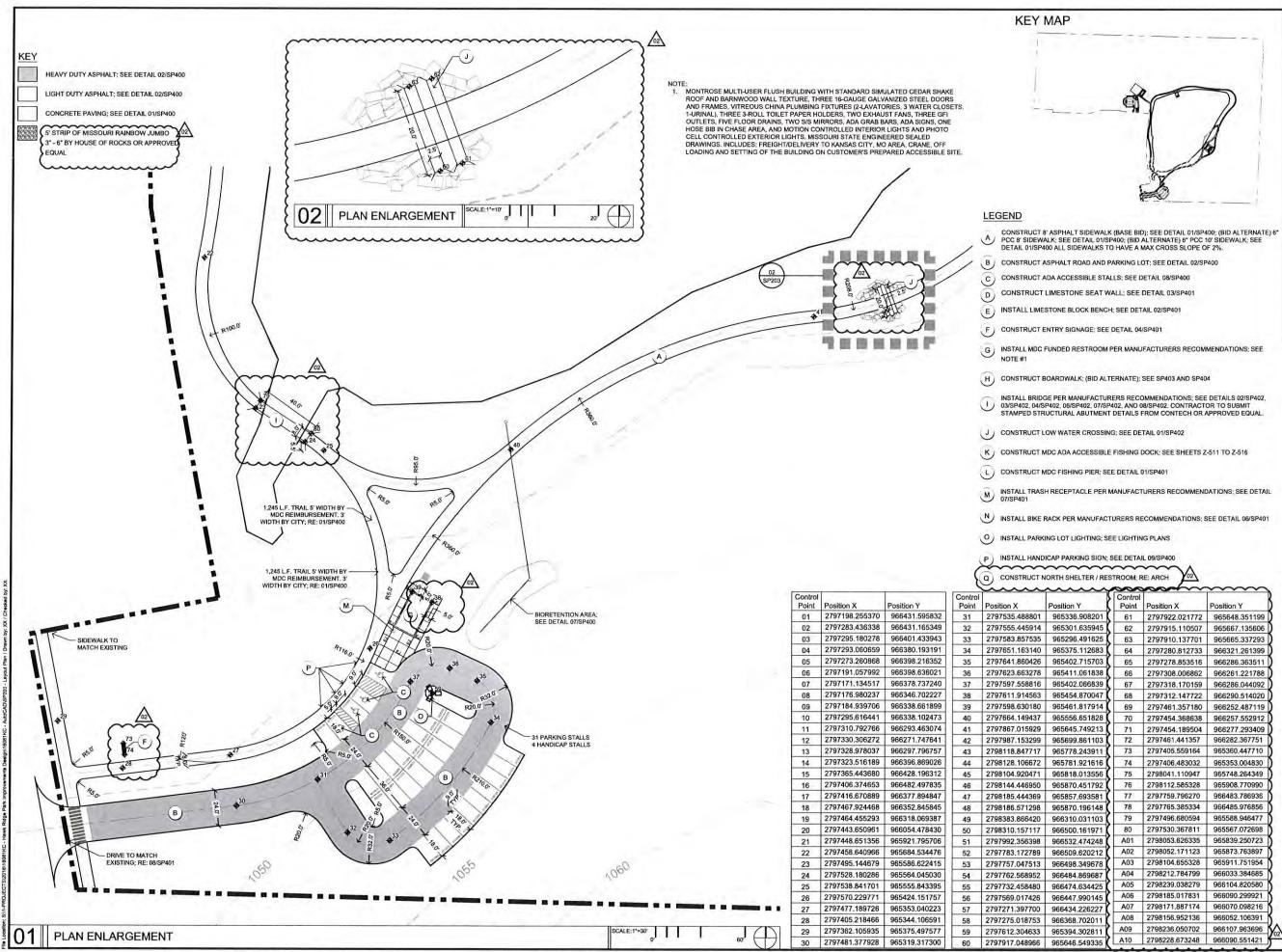
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST OVERLAND PARK, KANSAS 66204 PH: 913.214.2169







•	•	•		•	•	•	*		NA
HELT	ER	/ RE	ST	ROC	M;	RE:	ARC	CH	202

5	Control		
nY S	Point	Position X	Position Y
36.908201	61	2797922.021772	965648.351199
01.635945	62	2797915.110507	965667.135606
96.491625	63	2797910.137701	965665.337293
75.112683	64	2797280.812733	966321.261399
02.715703	65	2797278.853516	966286.363511
11.061838	66	2797308.006862	966261.221788
02.066839	67	2797318.170159	966286.044092
54.870047	68	2797312.147722	966290.514020
61.817914	69	2797461.357180	966252.487119
56.651828	70	2797454.368638	966257.552912
45.749213	71	2797454.189504	966277.293409
99.861103	72	2797461.441357	966282.367751
78.243911	73	2797405.559164	965360.447710
81.921616	74	2797406.483032	965353.004830
18.013556	75	2798041.110947	965748.264349
70.451792	76	2798112.585328	965908.770990
57.693581	77	2797759.796270	966483.786936
70.196148	78	2797765.385334	966485.976856
10.031103	79	2797496.680594	965588.946477
0.161971	80	2797530.367811	965567.072698
32.474248	A01	2798053.626335	965839.250723
09.620212	A02	2798052.171123	965873.763897
8.349678	A03	2798104.655328	965911.751954
34.869687	A04	2798212.784799	966033.384685
4.634425	A05	2798239.038279	966104.820580
7.990145	A06	2798185.017831	966090.299921
4.226227	A07	2798171.887174	966070.098216
8.702011	A08	2798156.952136	966052.106391
4.302811	A09	2798236.050702	966107.963696
6.549335	A10	2798228.673248	966090.551421

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

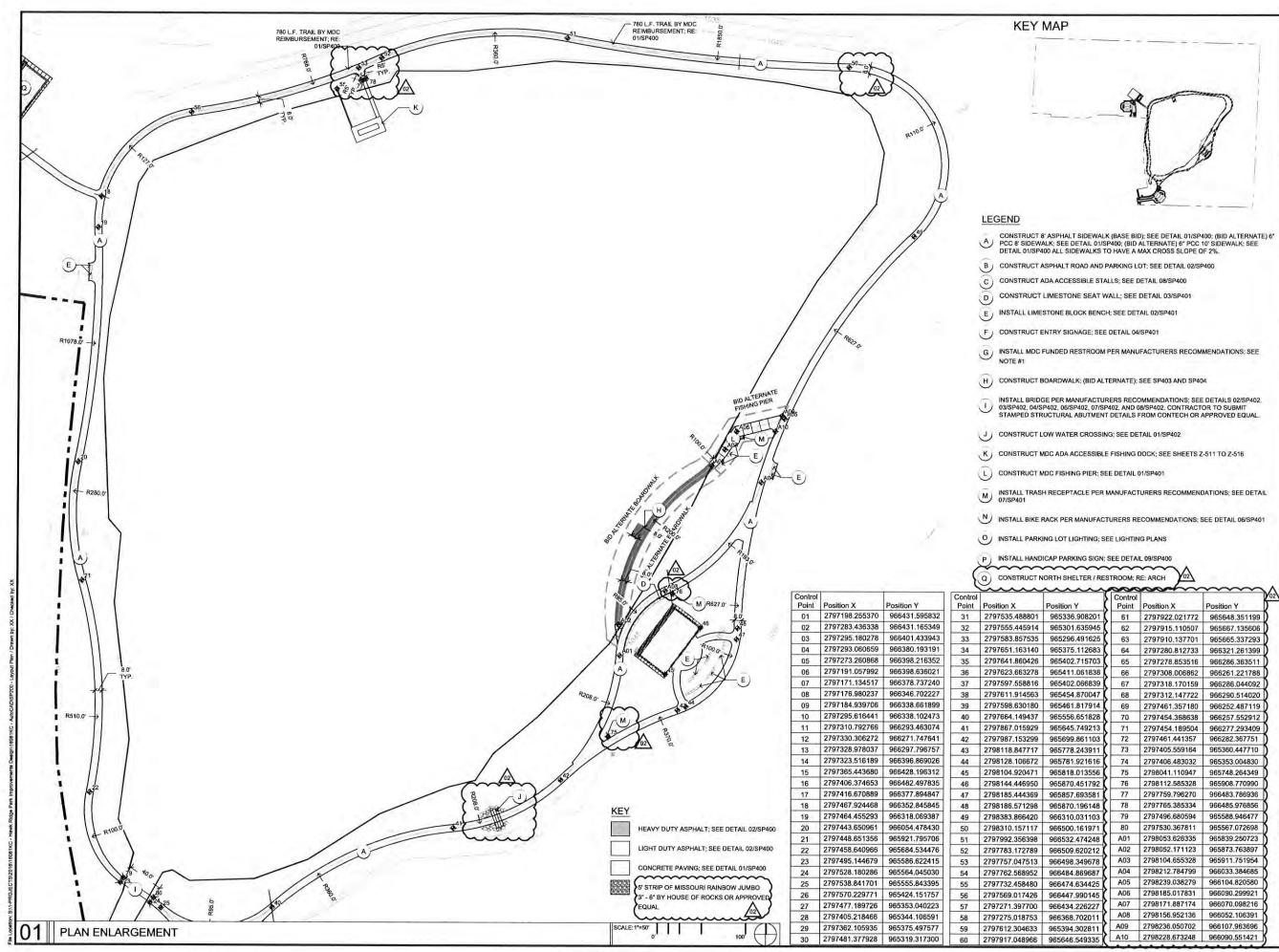
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169







									1/1	ŝ
HELT	ER	/ RE	ST	ROC	M:	RE:	AR	CH	Yoz	

m	Control		
Y	Point	Position X	Position Y
6.908201	61	2797922.021772	965648.351199
1.635945	62	2797915.110507	965667.135606
6.491625	63	2797910.137701	965665.337293
5.112683	64	2797280.812733	966321.261399
2.715703	65	2797278.853516	966286.363511
1.061838	66	2797308.006862	966261.221788
2.066839	67	2797318.170159	966286.044092
1.870047	68	2797312.147722	966290.514020
1.817914	69	2797461.357180	966252.487119
6.651828	70	2797454.368638	966257.552912
5.749213	71	2797454.189504	966277.293409
.861103	72	2797461.441357	966282.367751
.243911	73	2797405.559164	965360.447710
.921616	74	2797406.483032	965353.004830
.013556	75	2798041.110947	965748.264349
451792	76	2798112.585328	965908.770990
.693581	77	2797759.796270	966483.786936
.196148	78	2797765.385334	966485.976856
0.031103	79	2797496.680594	965588.946477
.161971	80	2797530.367811	965567.072698
.474248	A01	2798053.626335	965839.250723
.620212	A02	2798052.171123	965873.763897
.349678	A03	2798104.655328	965911.751954
.869687	A04	2798212.784799	966033.384685
.634425	A05	2798239.038279	966104.820580
990145	A06	2798185.017831	966090.299921
.226227	A07	2798171.887174	966070.098216
702011	A08	2798156.952136	966052,106391
.302811	A09	2798236.050702	966107.963696
549335	A10	2798228.673248	966090.551421

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

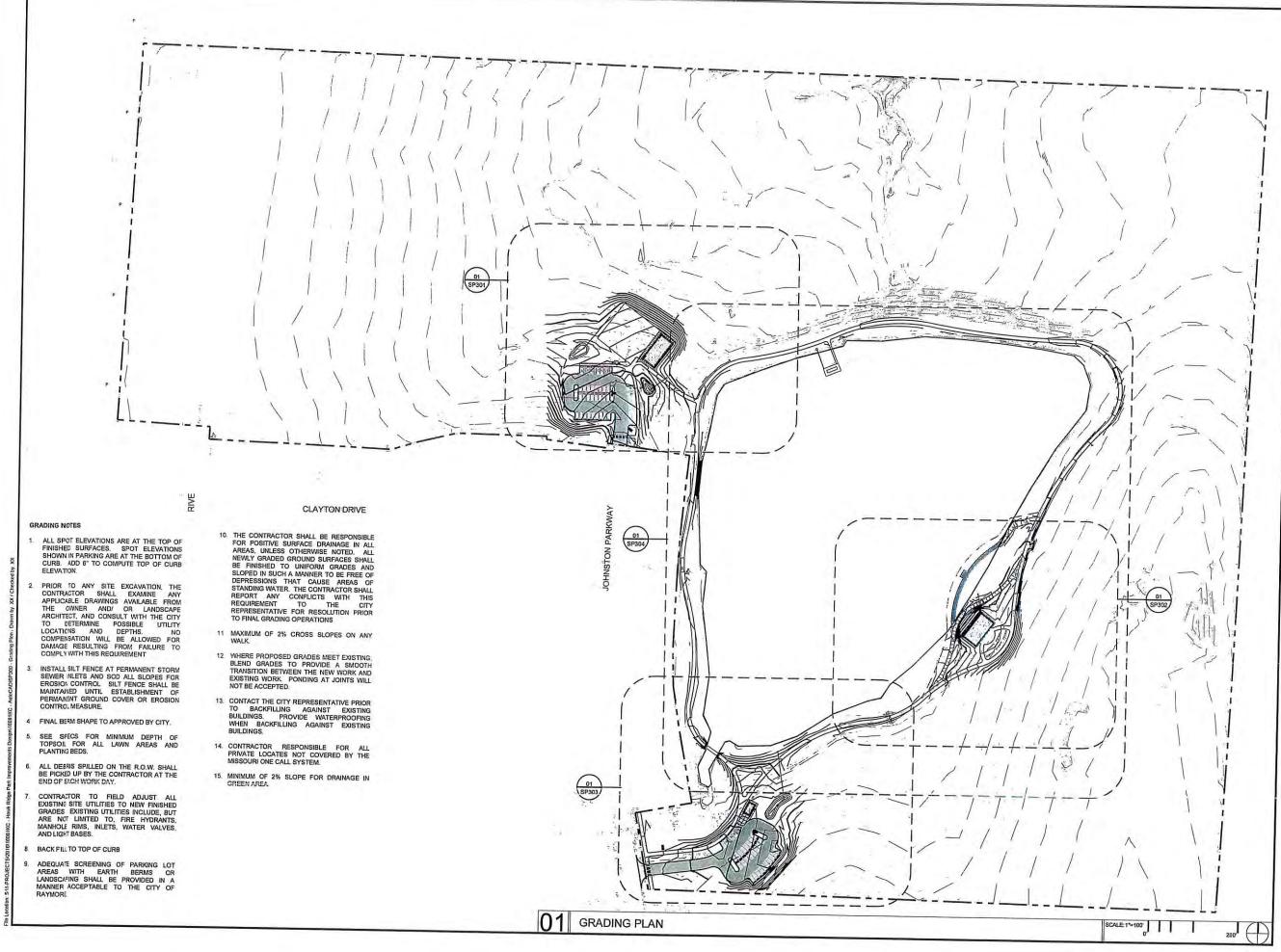
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169







LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816,701,3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

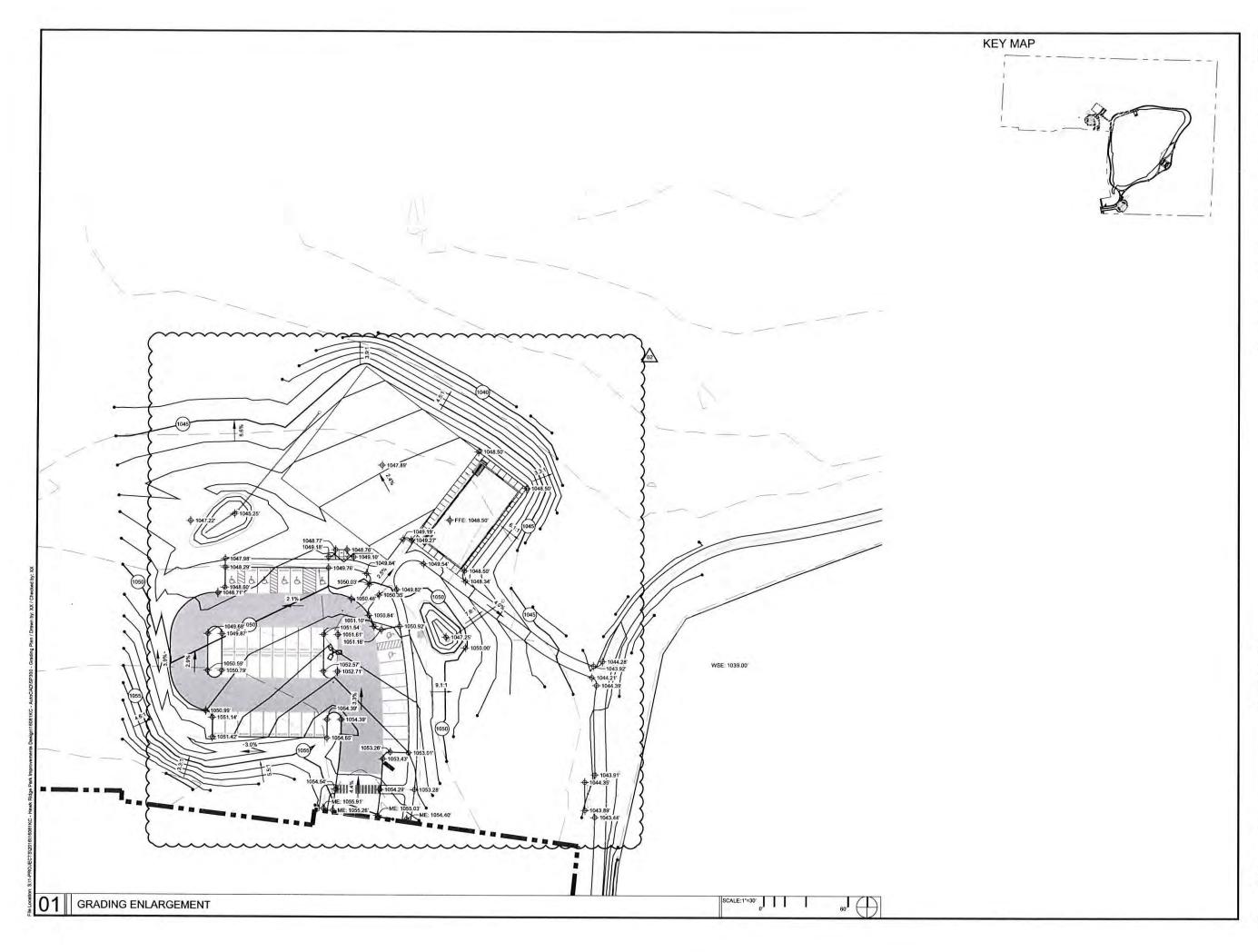




**REVISION SCHEDULE** 

### **GRADING PLAN**

CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CUTY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

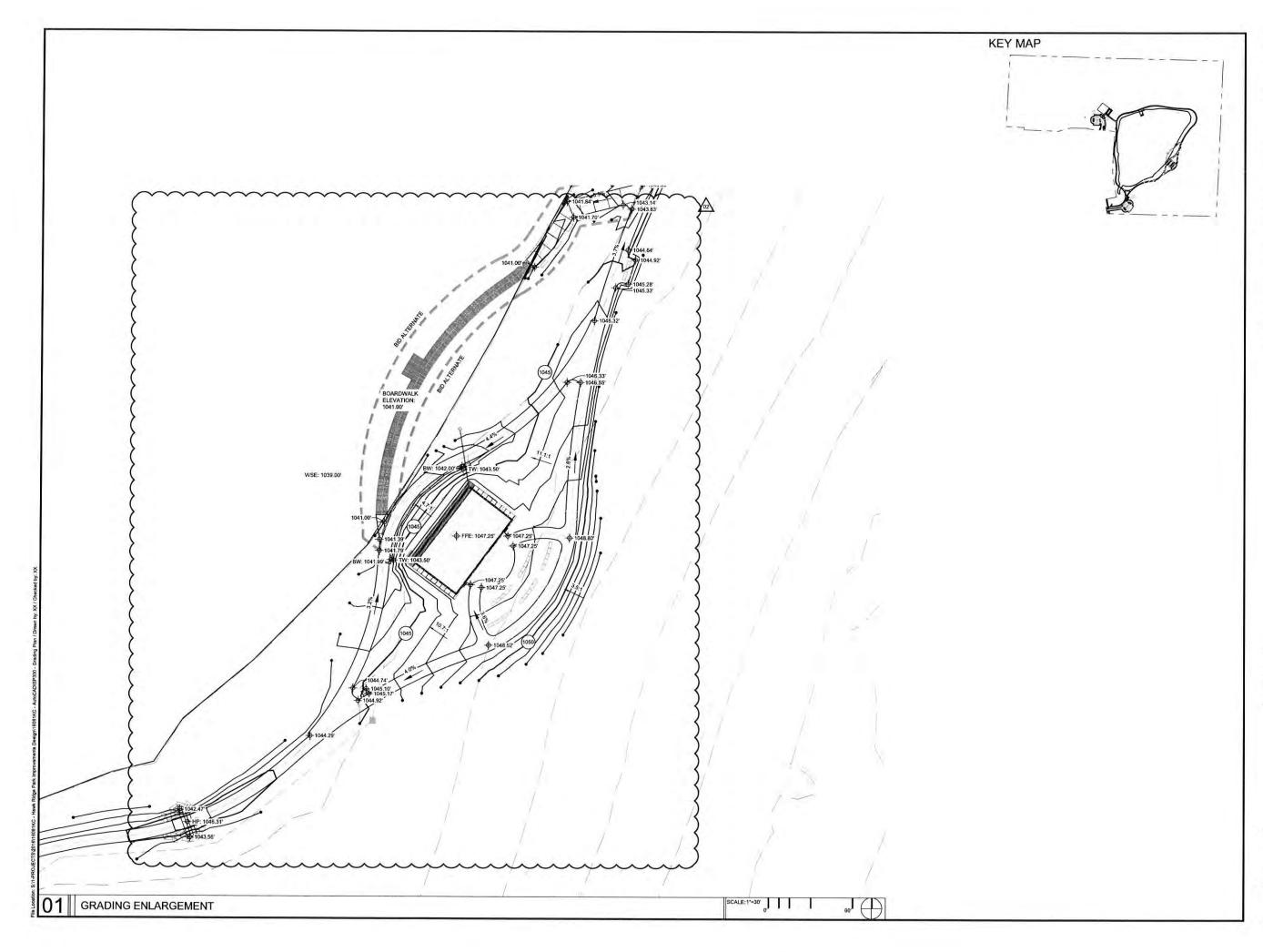
MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI



CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE: SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER. STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

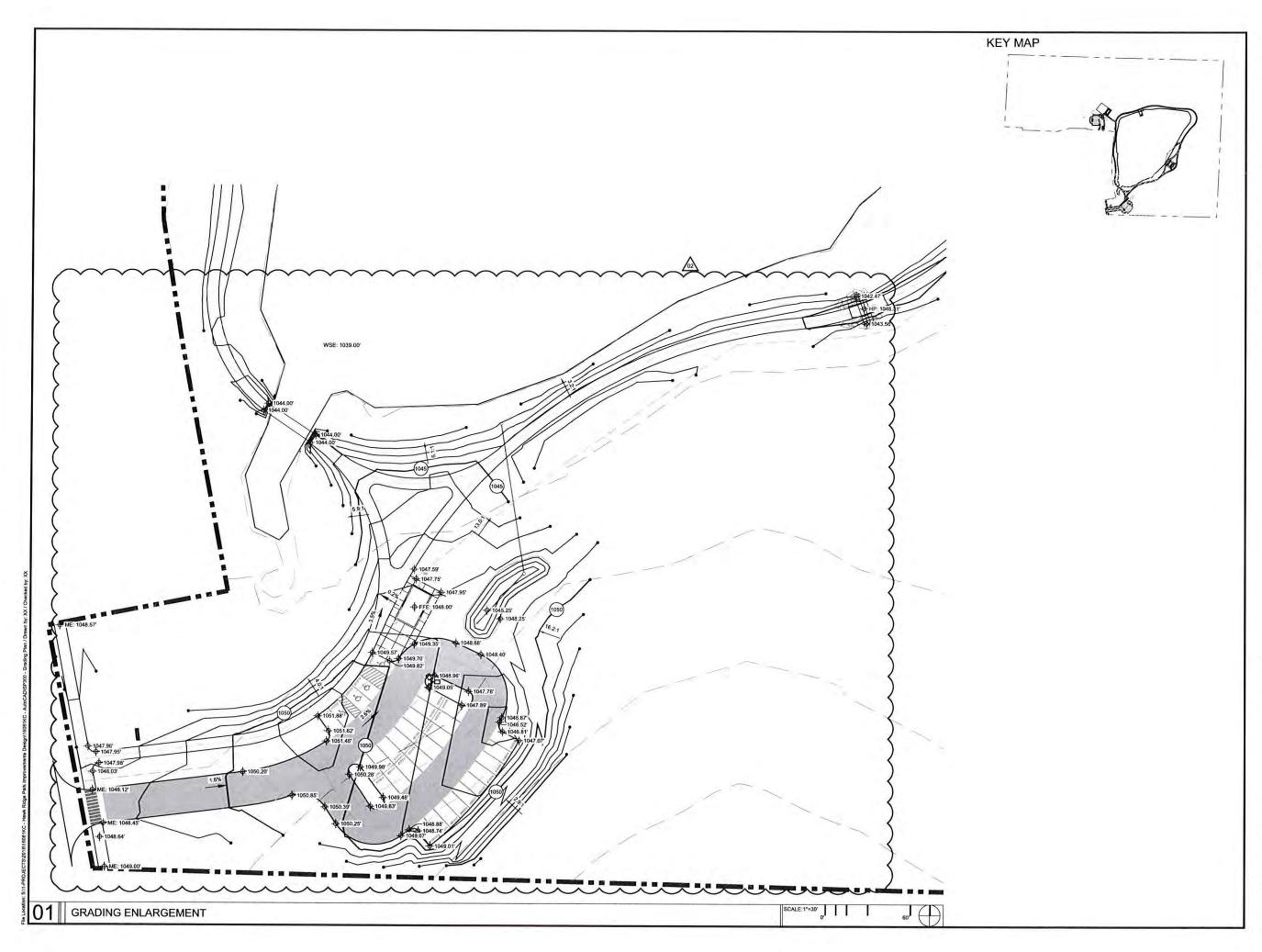
MEP ENGINEER HENDERSON ENGINEERING 1801 Main, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.683.718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI



CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816,541,2288

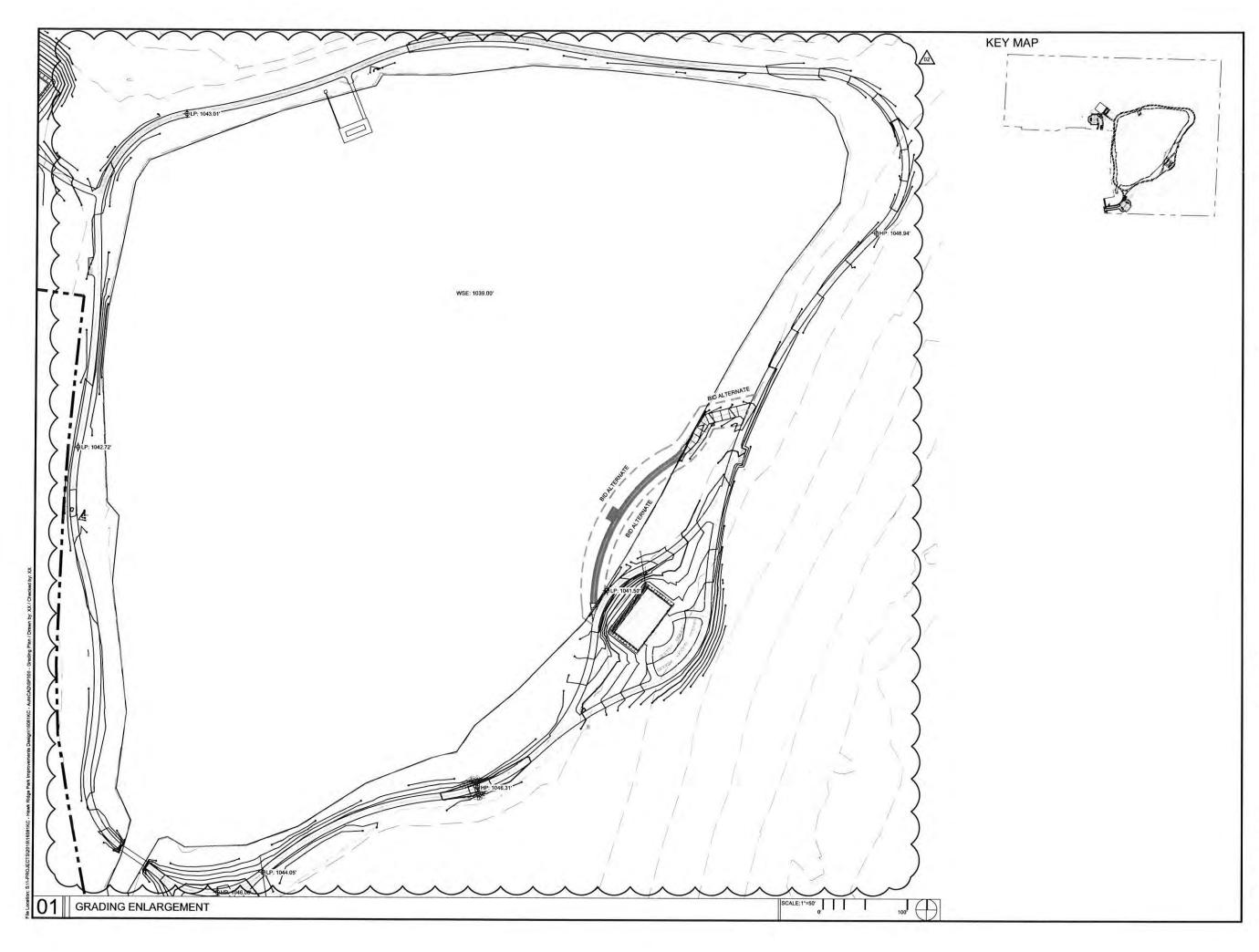
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT, OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI





LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC, 415 DELWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

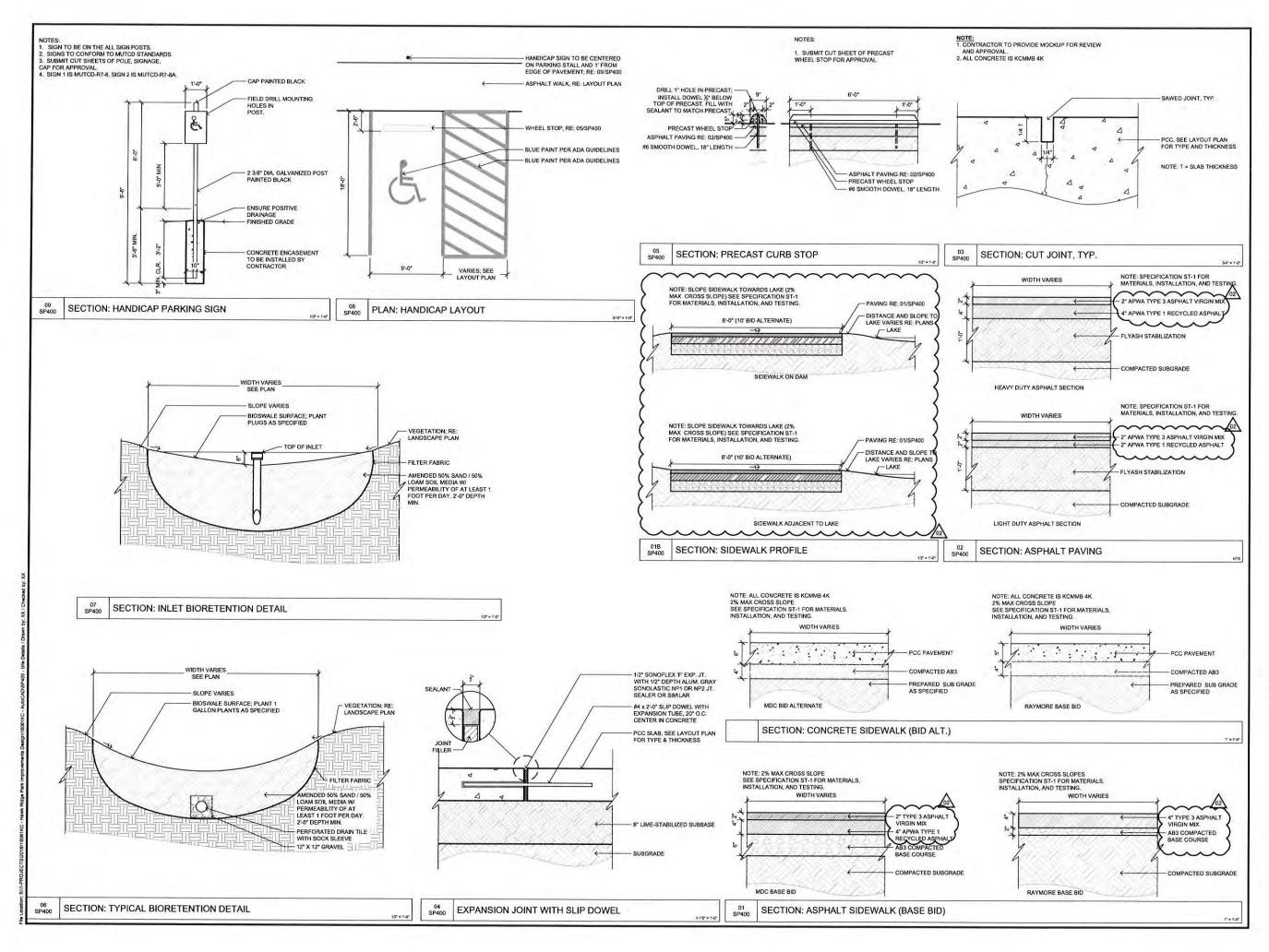
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT, OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CUTY, MISSOURI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI

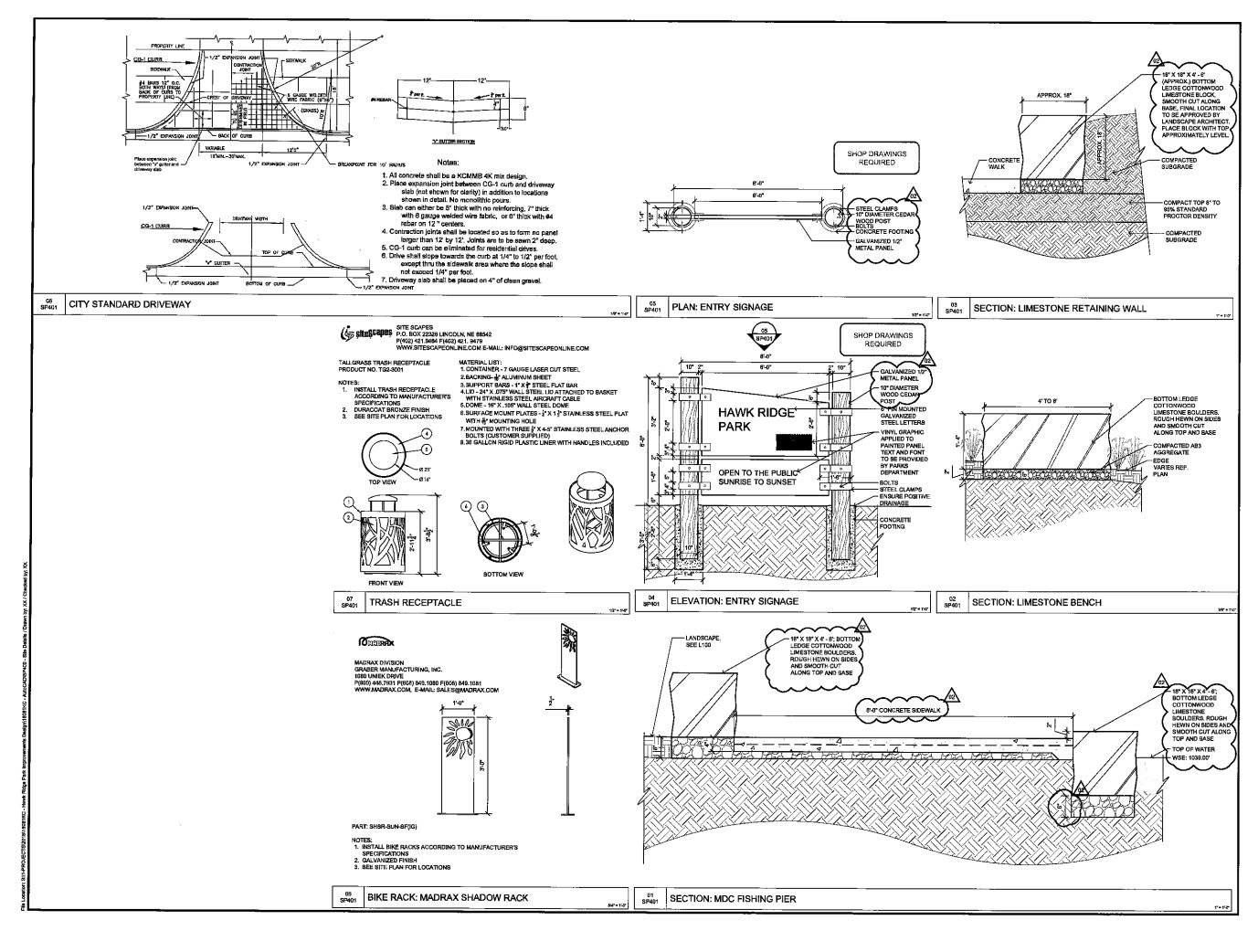




## CONFLUENCE LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227 ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288 CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816,701,3100 CERT. OF AUTHORITY #2003007599 MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718 STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST OVERLAND PARK, KANSAS 66204 PH: 913.214.2169 HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI **REVISION SCHEDULE** ISSUE DESCRIPTION DATE 03/09/18 CONSTRUCTION DR. A 423/18 ADDENDUM 0 OF MISSO TERRY J. BERKBUEGLER NUMBES

SITE DETAILS

CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 84105 PH: 810.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816,541.2288

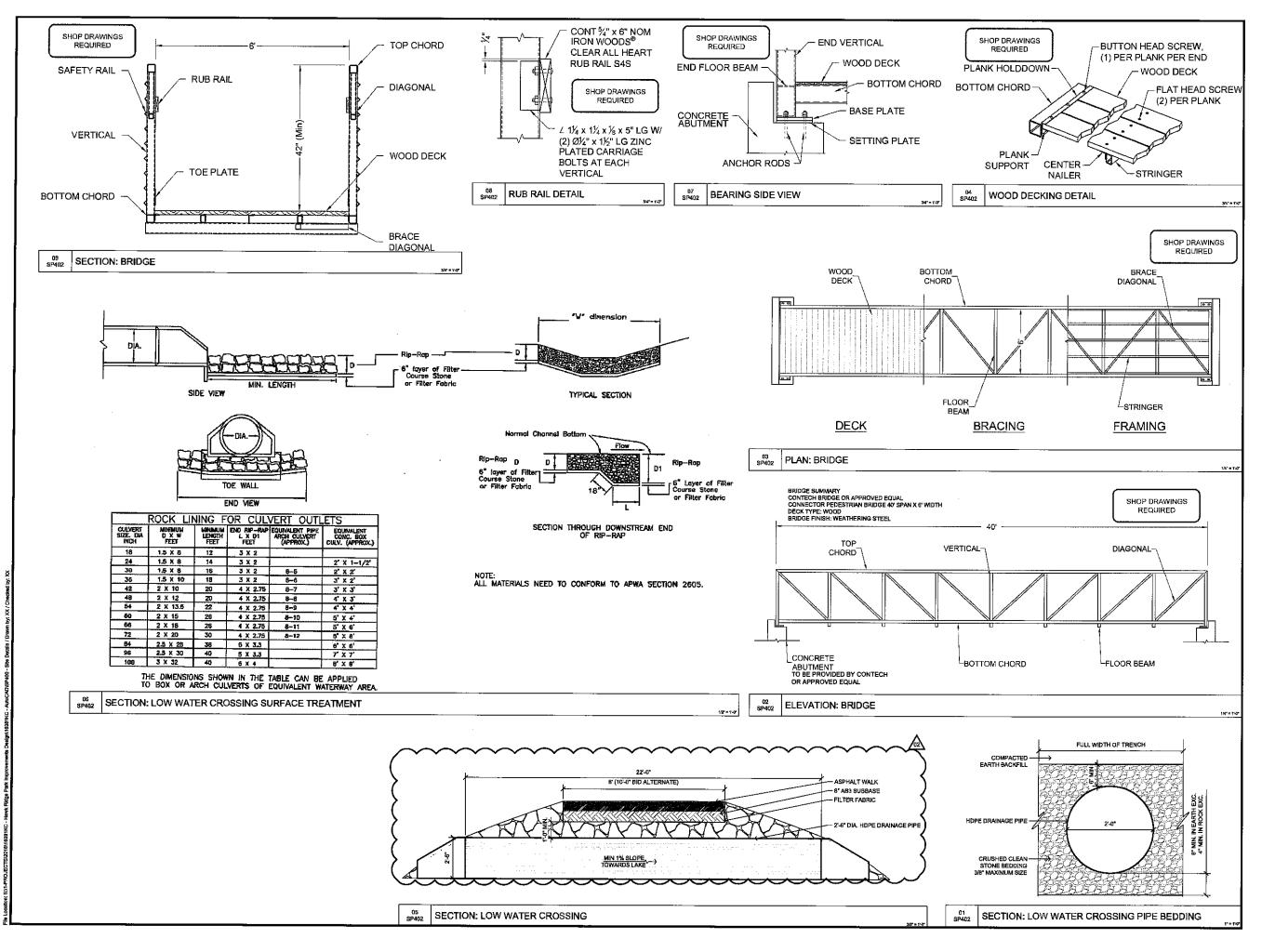
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 818,701,3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CUTY, MISSOURI 64198 PH: 816.683,5718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169





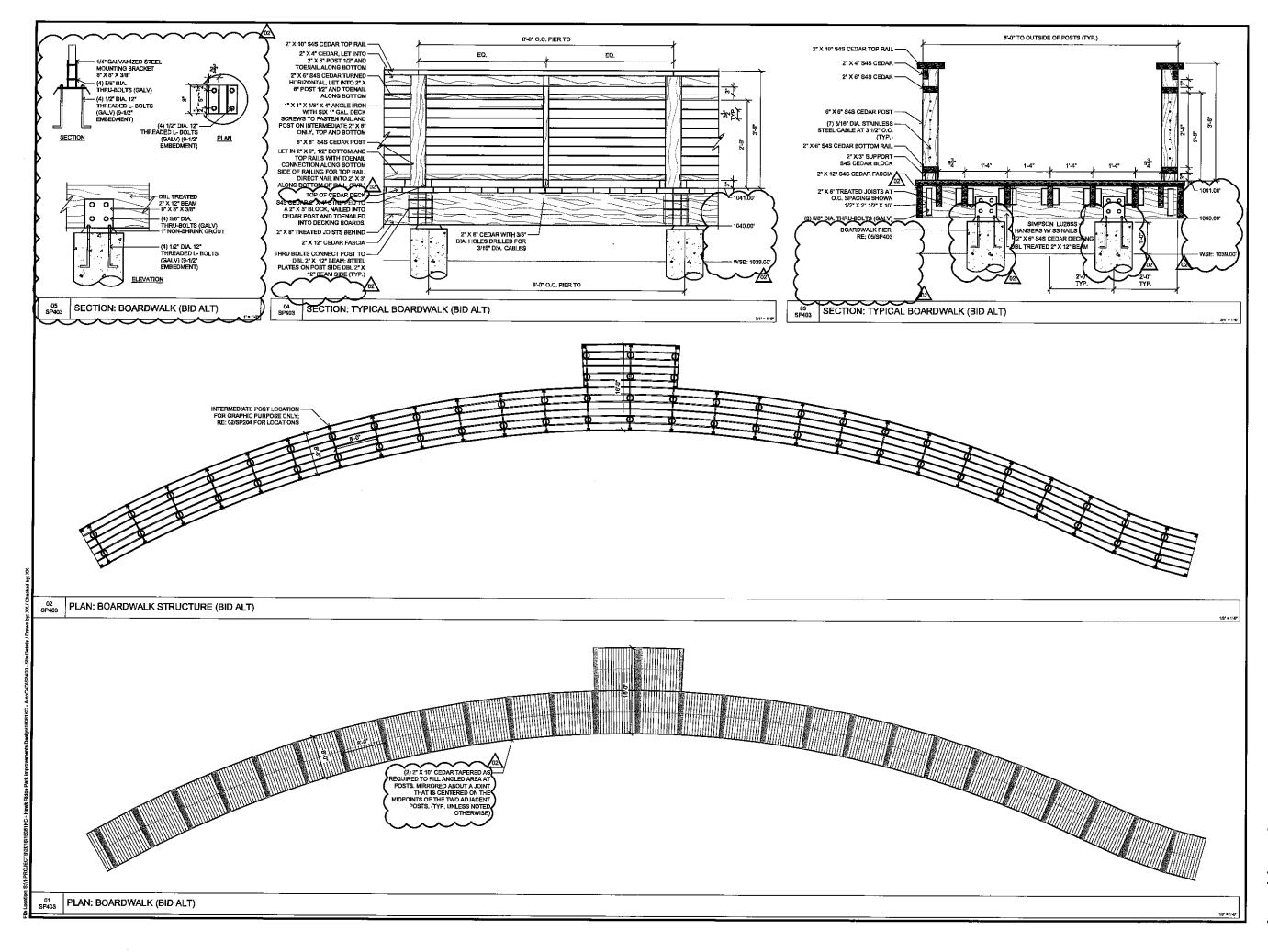


# LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227 ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288 CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER. STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT, OF AUTHORITY #2003007599 MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816,663,8718 STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169 Y OF RAYMORE PARK AND RECREATION RAYMORE/MISSOURI HAWK RIDGE CIT∕ REVISION SCHEDULE ISSUE DATE DESCRIPTIO 03/09/18 CONSTRUCTION DRA A ADDENDUM 02 4/23/18

CONFLUENCE

SITE DETAILS

CONFLUENCE PROJECT NO: 16081KC



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 416 DELAWARE, SUITE 400 KANSAS CUTY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 616.641.2286

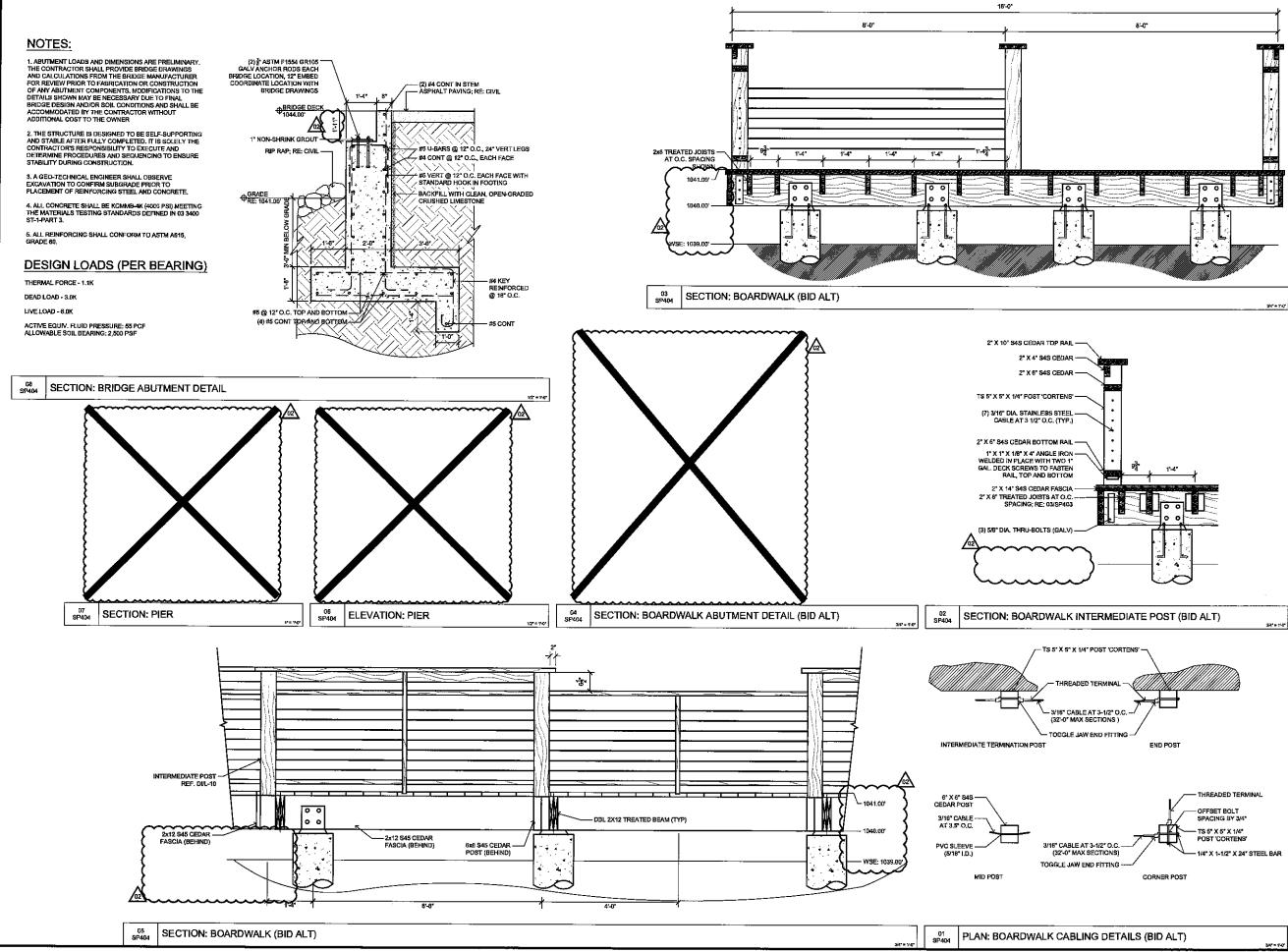
CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 84131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.683.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169







LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 04105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.663.8718

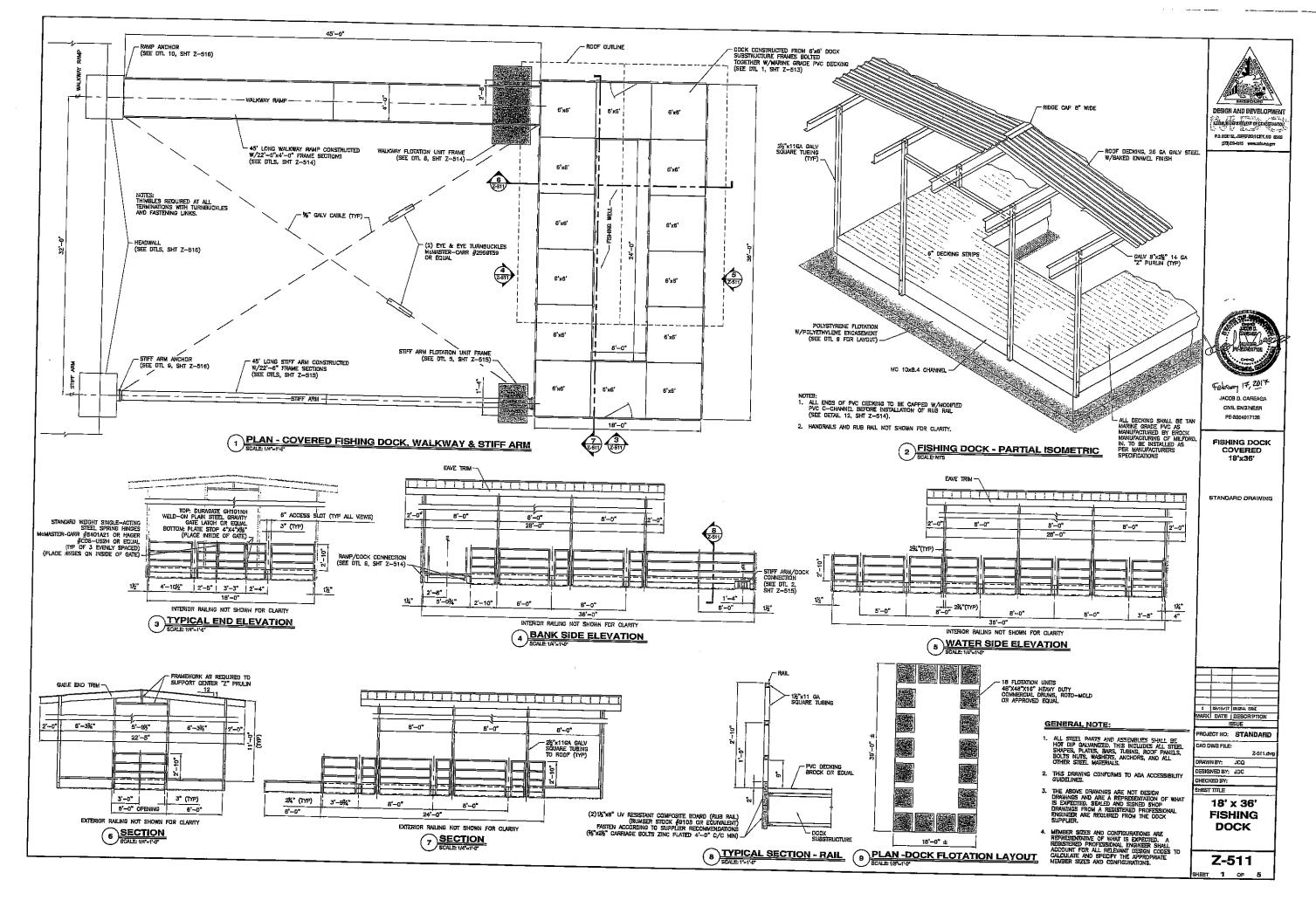
STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169





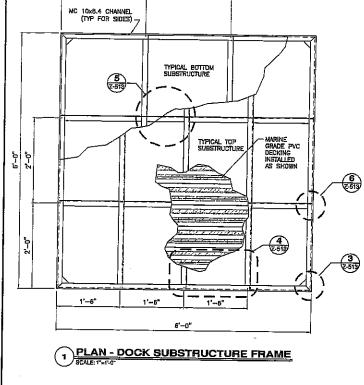
## SITE DETAILS

CONFLUENCE PROJECT NO: 16081KC



ierres: NACADIRhererot.coniemi\Standard Drawings\DeckaZ-511.cMg Philed on 02/17/17 12:00 PM by LeAnn Sea

2'-0"



K PLATE STEEL ALL CORNERS TOP & BOTTOM

<u>PLAN</u>

--<u>ō</u>--

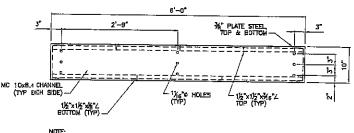
Ο

<u>___</u>___

ELEVATION

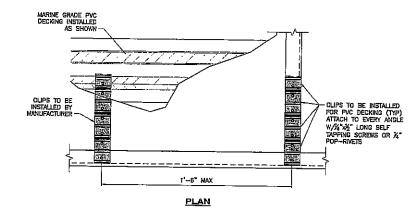
3 DETAIL

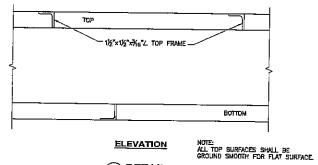
2'-0~



NOTE: 6'x6' SUB-FRAME SECTIONS SHALL BE SECURELY BOLTED TOBETHER AS SHOWN.

2 ELEVATION - DOCK SUBSTRUCTURE FRAME





4 DETAIL SOALE: 3"-1'-0"



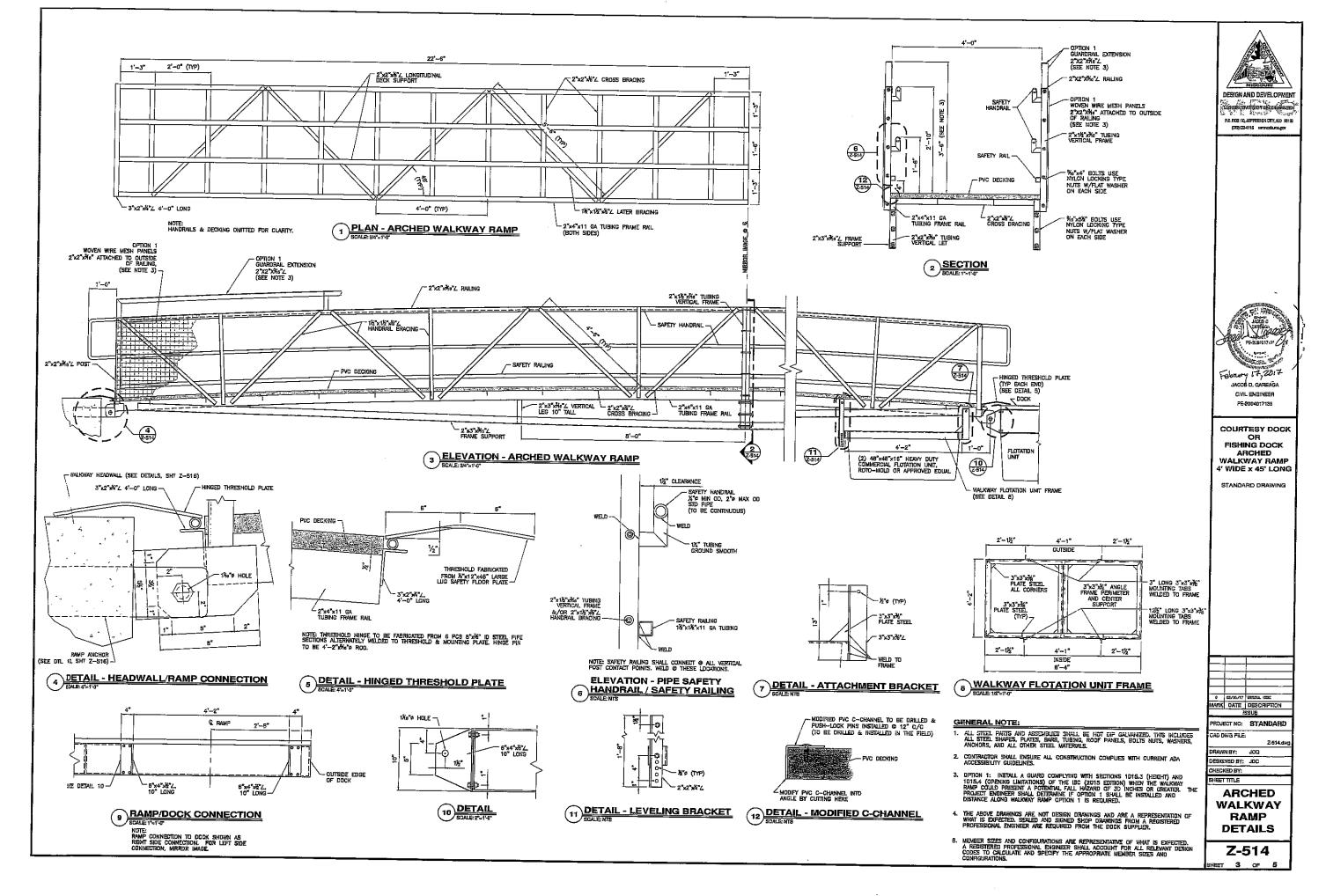
TOP

15"×15"×76"L BRACE -

5 DETAIL SCALE:3"=1"+0"

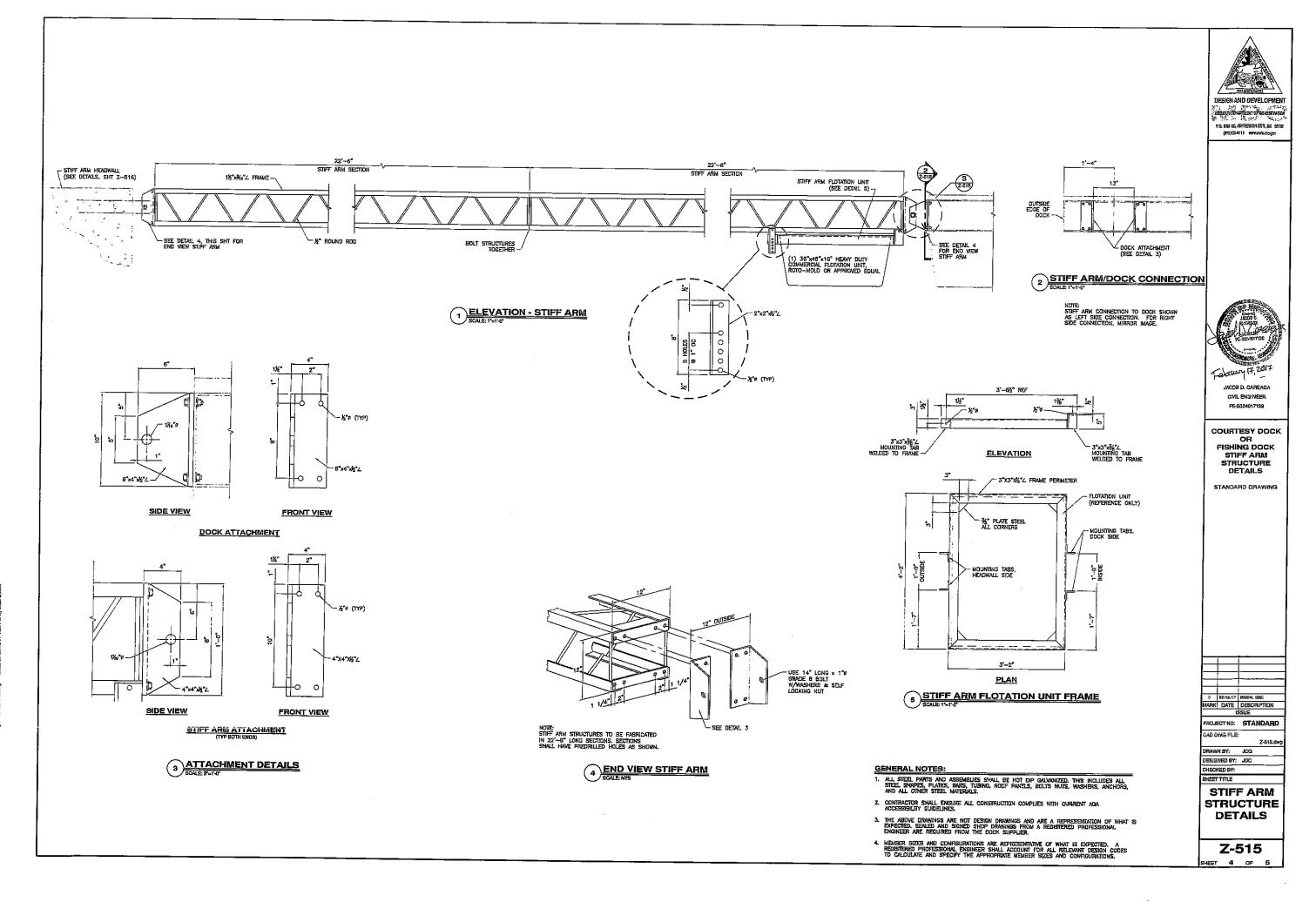
GENERAL NOTE:

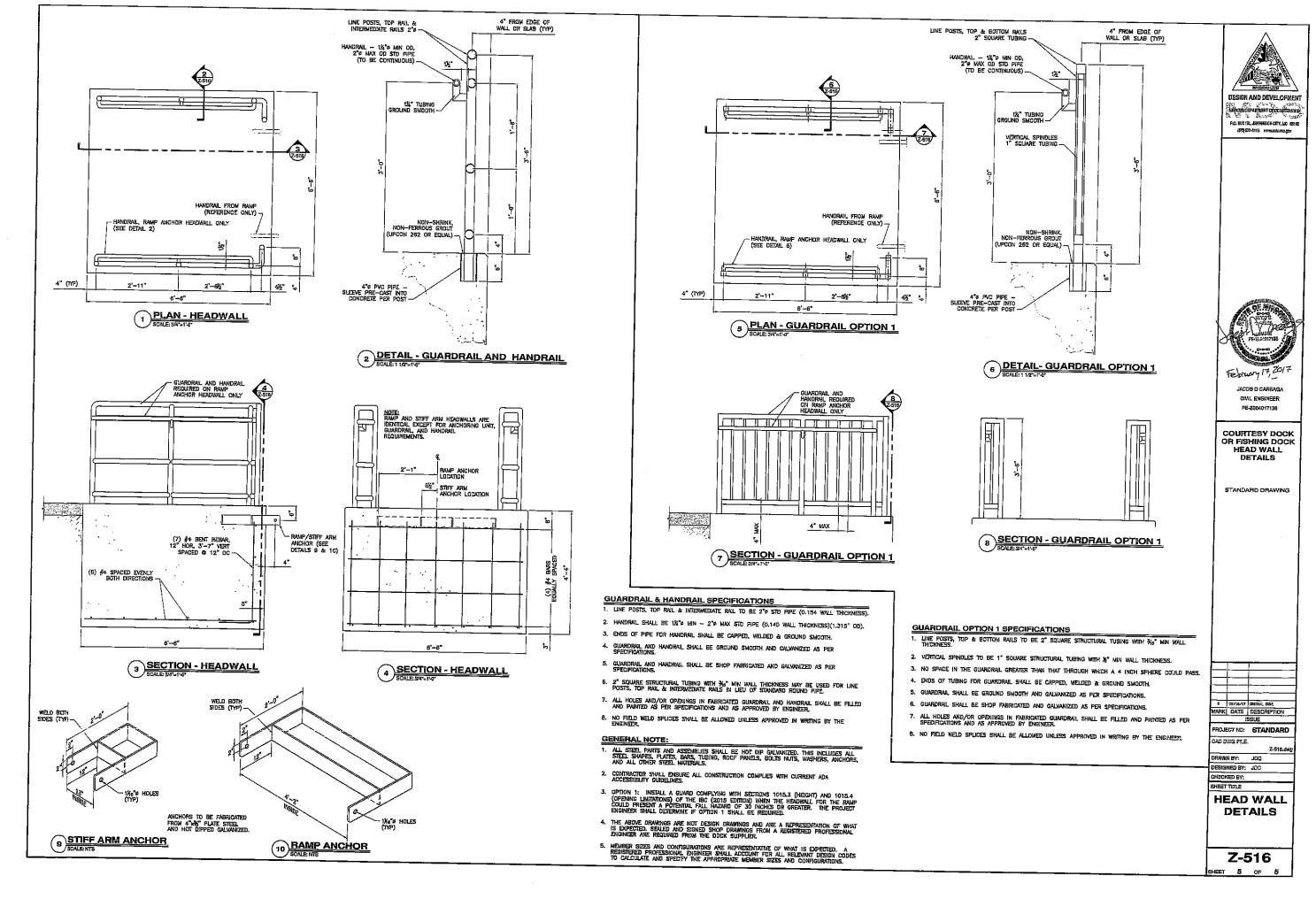
DESIGN AND DEVELOPMEN P.D. BOI IN AND DEVELOPMENT OF COLUMN AND DEPARTMENT OF COLUMN AND DEPARTMENT OF COLUMN AND DE LE COLUMNA AND DE LE COLUMNA AND DE LE COLUMNA - BOTTOM FRAME - BOTTOM BRACE, TOP & BOTTOM FRAME -TOP BRACE ---MC 10XB.4 CHANNEL TOP FRAME February 17,2017 <u>PLAN</u> PLAN JACOB D. CAREAGA CIVIL ENGINEER PE-2004017188 COURTESY DOCK 1½"x1½"x兆8"乙 FRAME OR FISHING DOCK STRUCTURE DETAILS MC 10X8.4 CHANNEL "1½"×1½"x½"上 FRAME 15 x15 x6 Z BOTTOM FRAME /-- 1½"x1½"x½"2 BRACE воттом STANDARD DRAWING **ELEVATION** ELEVATION 6 DETAIL SCALE: 3'=1'-0' 0 02/16/17 00284 1527 ARK DATE DESCRIPTION ISSUE PROJECT NO: STANDARD CAD DWG FILE: Z-513.dv DRAWN BY: JCQ ESIGNED BY: JDC IECKED BY: All Steel parts and assemblies shall be not dip galvanized. This includes all steel shapes, parts, barrs, tubing, roof panels, balts nuts, washers, anchors, and all other steel materials. IGET TITLE DOCK CONTRACTOR SHALL ENSURE ALL CONSTRUCTION COMPLIES WITH CURRENT ADA ACCESSIBILITY GUIDELINES. STRUCTURE DETAILS 3. THE ABOVE DRAWINGS ARE NOT DESIGN DRAWINGS AND ARE A REPRESENTATION OF WHAT IS EXPECTED. SEALED AND SIGNED SHOP DRAWINGS FROM A REDISTERED PROFESSIONAL ENGINEER ARE REQUIRED FROM THE DOCK SUPPLIER. 4. MEMBER SIZES AND CONFIGURATIONS ARE REPRESENTATIVE OF WHAT IS EXPECTED. A REGISTERED PROFESSIONAL ENGINEER SHALL ACCOUNT FOR ALL RELEVANT DESIGN CODES TO CALCULATE AND SPECIFY THE APPROPRIATE MEMBER SIZES AND CONFIGURATIONS. Z-513 HEET 2 OF 5



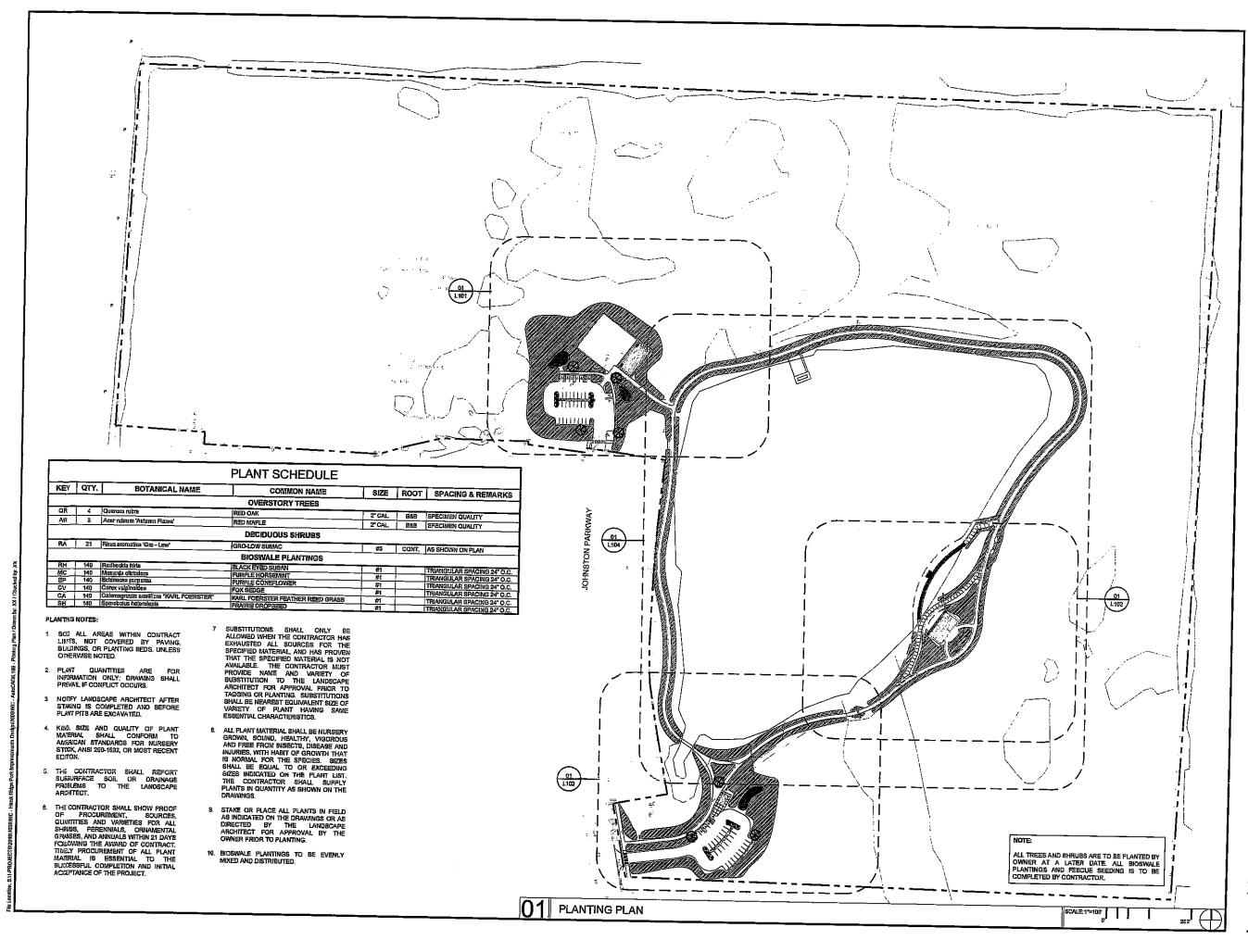
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daled on 02/10/17 1:34 PM by Jack Quedo



LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS GTY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 84108 PH: 816.541,2288

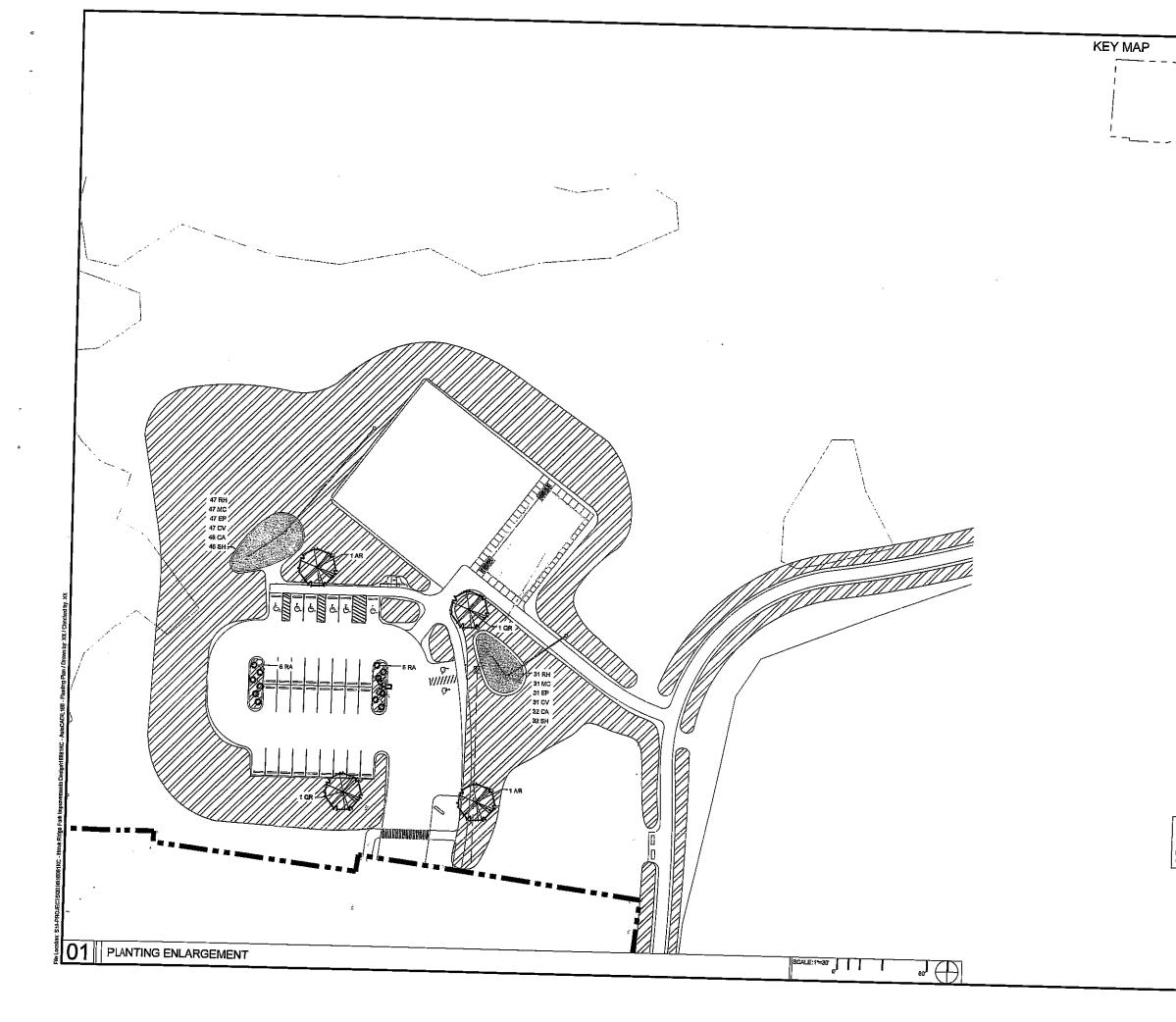
CIVIL ENGINEER WILSON & COMPANY 800 E 1018TTER, 8TE 200 KANSAS CITY, MISSOURI 64131 PH: 815.701.3100 CERT. OF AUTHORITY #2003007598

MEP ENGINEER HENDERSON ENGINEERING 1801 Main, ste 300 Kansas city, Missouri 64108 PH: 616.653.0718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINGON ST. OVERLAND PARK, KANSAS 66204 PH: 913244/2169









BIOSWALE PLANTINGS



FESCUE GRASS SEEDING

### KEY

ALL TREES AND SHRUBS ARE TO BE PLANTED BY OWNER AT A LATER DATE. ALL BIOSWALE PLANTINGS AND FESCUE SEEDING IS TO BE COMPLETED BY CONTRACTOR.

NOTE:



CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOLIRI 84105 PH: 816,531,7227

SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816,541,2288

CIVIL ENGINEER WILSON & COMPANY

800 E 1015T TER, STE 200 KANSAS CITY, MISSOLIRI 64131 PH: 816.701.3100 CERT. OF AUTHOR(TY #2003007599

MEP ENGINEER HENDERSON ENGINEERING

ARCHITECT









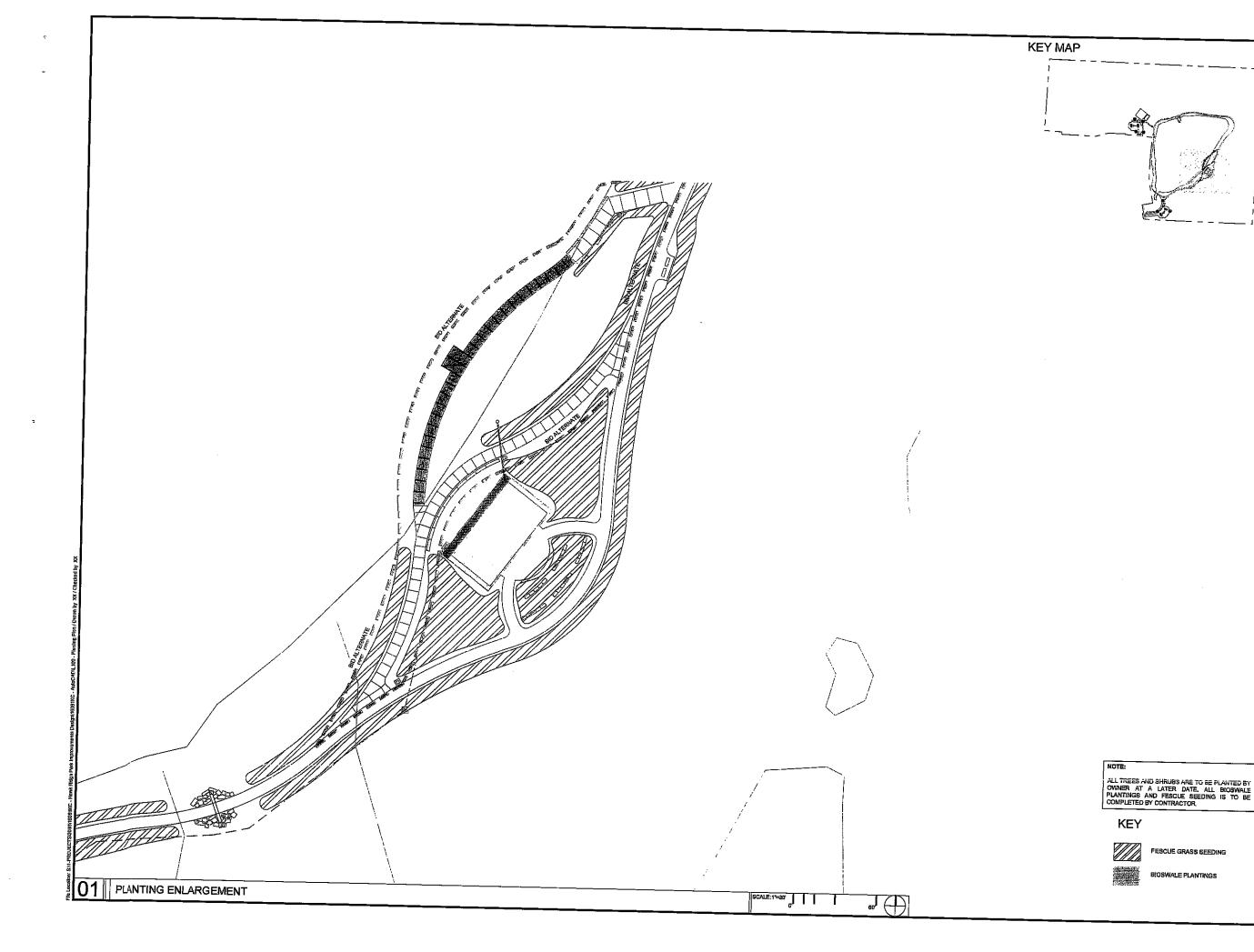


PLANTING

ENLARGEMENT

CONFLUENCE PROJECT NO: 16081KC







BIOSWALE PLANTINGS

FESCUE GRASS SEEDING





### KEY

# CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64705 PH: 816.531.7227

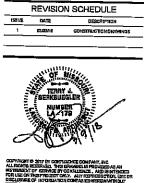
ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816,541,2288

CIVIL ENGINEER WILSON & COMPANY 800 E 1015T TER, STE 200 KANSAS CITY, MIBBOURI 64131 PH: 816.71.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.653.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

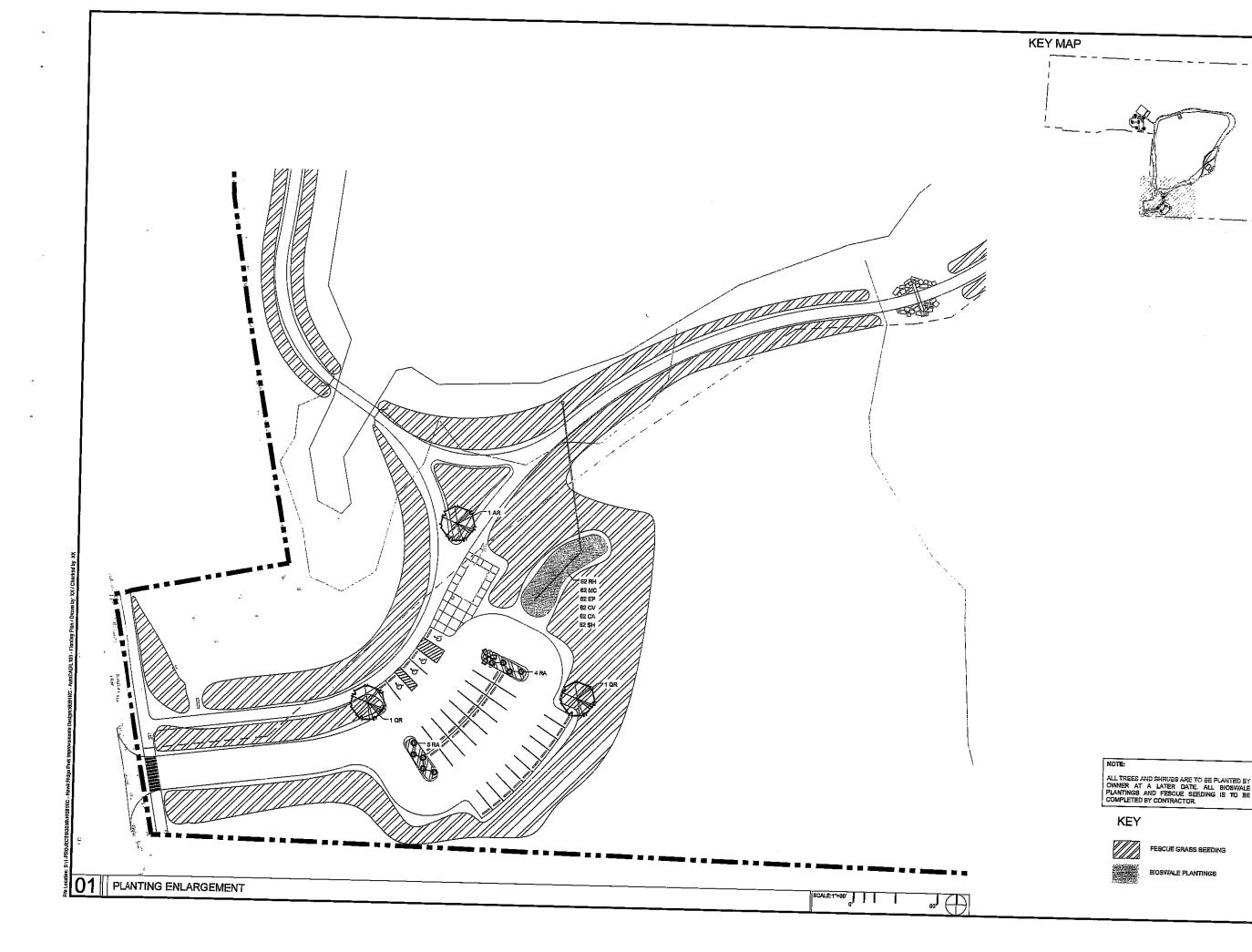




PLANTING

ENLARGEMENT

CONFLUENCE PROJECT NO: 18081KC





BIOSWALE PLANTINGS

Â

FESCUE GRASS SEEDING

### KEY

NOTE:

## CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 84105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MIBSOURI 64108 PH: 816,541,2288

CIVIL ENGINEER WILSON & COMPANY 800 E 1015T TER, STE 200 KANAAS CITY, MISSOURI 64131 PH: 816.71.3100 CERT, OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 816.693.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214,2169



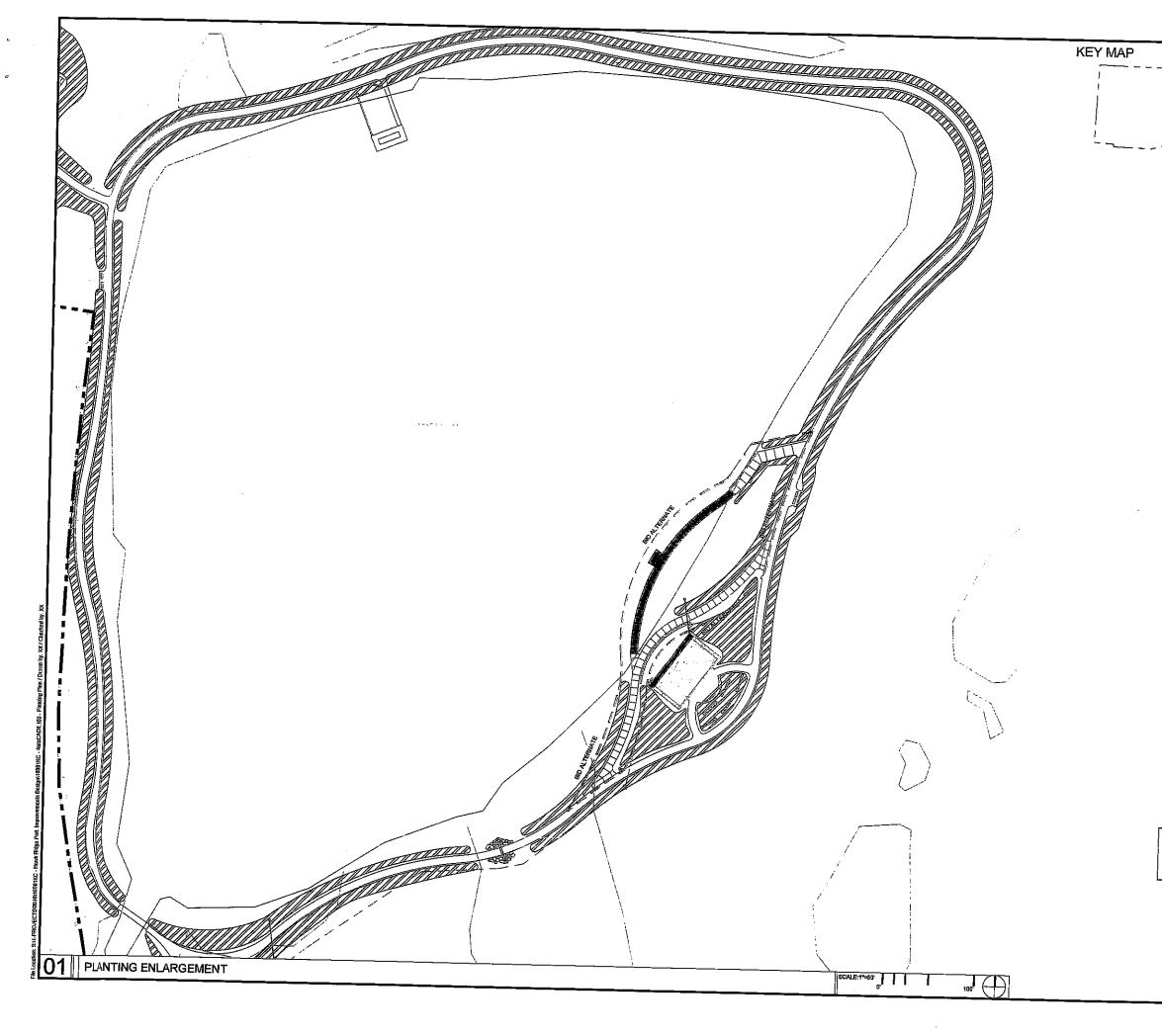


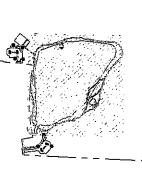




ENLARGEMENT

CONFLUENCE PROJECT NO: 16081KC





### NOTE:

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### KEY



FESCUE GRASS SEEDING



## CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 416 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 FH: 616.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.641.2288

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, STE 200 KANSAS CITY, MISSOURI 64131 PH: B16.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOURI 64108 PH: 818,653,8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANBAS 66204 PH: 913,214,2169

> HAWK RIDGE PARK CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI

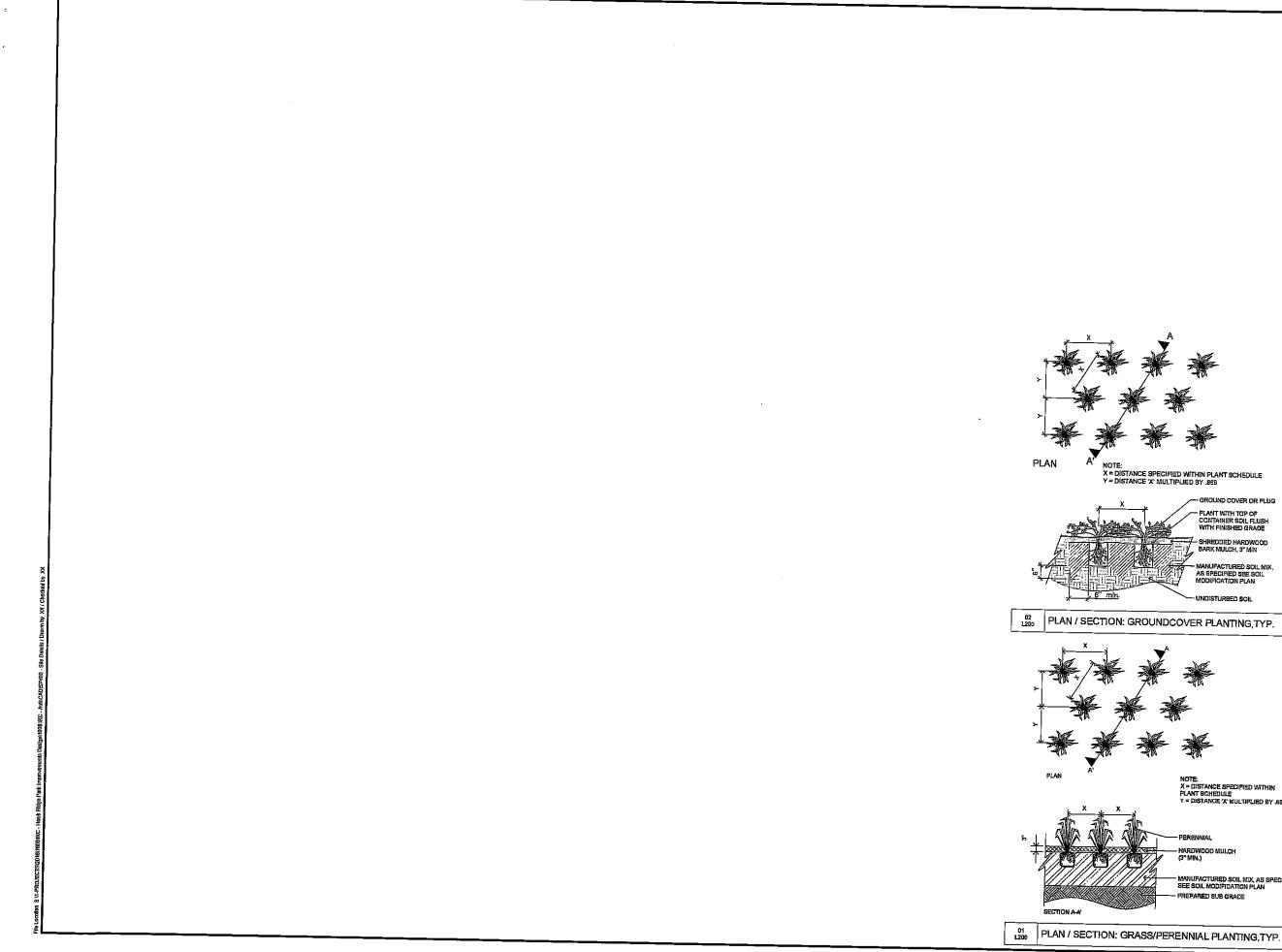




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CONFLUENCE PROJECT NO: 16081KC



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- PLANT WITH TOP OF CONTAINER SOIL FLUSH WITH FINISHED GRADE

- SHREDDED HARDWOOD BARK MULCH, 3" MIN

-- MANUFACTURED SOIL MIX, AS SPECIFIED SEE SOIL MODIFICATION PLAN.

1/2" = 1"-0"

Note: X = Distance specified within Flant Schedule Y = Distance: X: Multiplied By 1858

- MANUFACTURED SOIL MIX, AS SPECIFIED SEE SOIL MODIFICATION PLAN FREPARED SUB GRADE

12 = 1-0

## CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816,541,2288

CIVIL ENGINEER WILSON & COMPANY WILSON & COMPANY 800 E 1018TTER, STE 200 KANSAS CITY, MISCOURI 64131 PH: 816.701.3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MISSOLIRI 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169



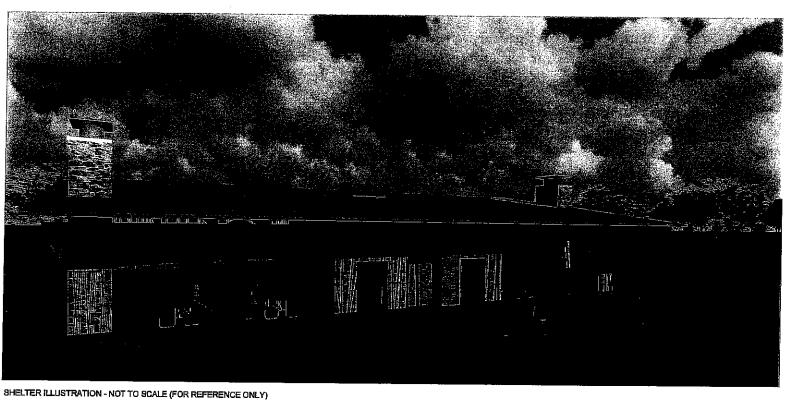


CONFLUENCE PROJECT NO: 16081KC L200





AMPHITHEATRE ILLUSTRATION - NOT TO SCALE (FOR REFERENCE ONLY)



## CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANBAS CITY, MISCOURI 64105 PH: 816.531.7227 FAX: 816.531.7229

ARCHITECT ARCHITECTURE 2100 CENTRAL STREET, BUITE 31 KANSAS CITY, MIBBOURI 64108 PH: 816.474.1397

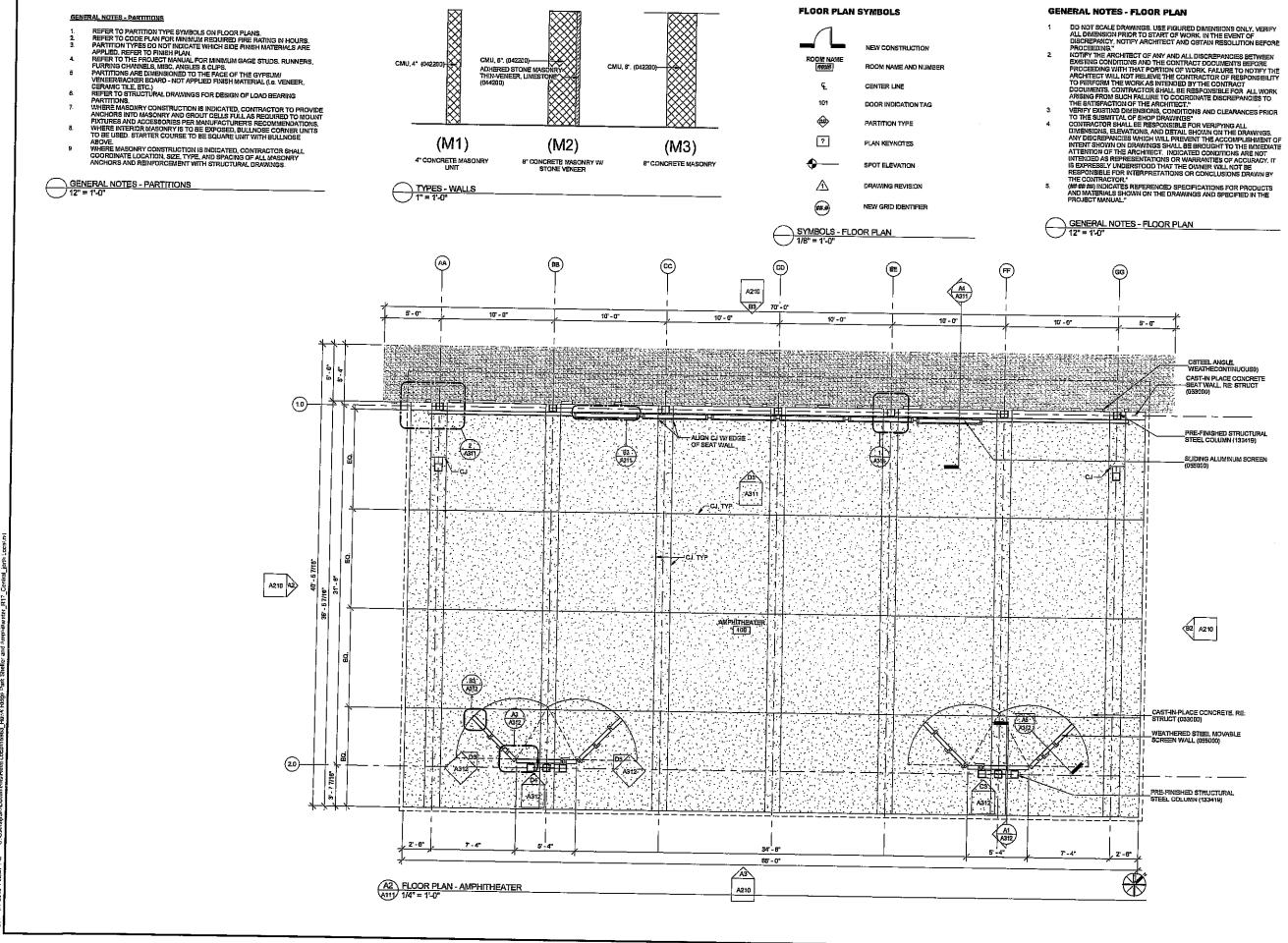
CIVIL ENGINEER WILSON & COMPANY 800 F 10151 TER, SUITE 200 KANSAS CITY, MO 64131 PH: 816,701.3100 CERT. OF AUTHORITY #2003007559

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STREET, SUITE 300 KANBAS CITY, MO 64108 PH: 816.653.8718

# STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

CITY OF RAYMORE PARKS AND RECREATION RAYMORE, MISSOURI HAWK RIDGE PARK





LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 31 KANSAB CITY, MISSOURI 64108

PH: 816.474.1397

**CIVIL ENGINEER** WILSON & COMPANY 800 E 1018T TER, SU/TE 200 KANGAS CITY, MO 64131 PH: 816 701 3100 CERT. OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MO 64108 PH: 816663.8718

### STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC.

6234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

> RAYMORE PARKS AND RECREATION ARK מ RAYMORE, MISSOUR Ш RIDGE HAWK CITY OF

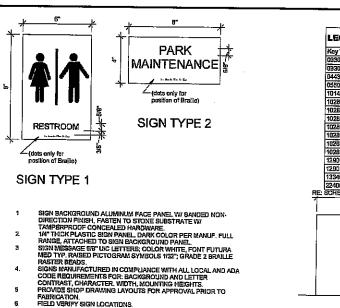


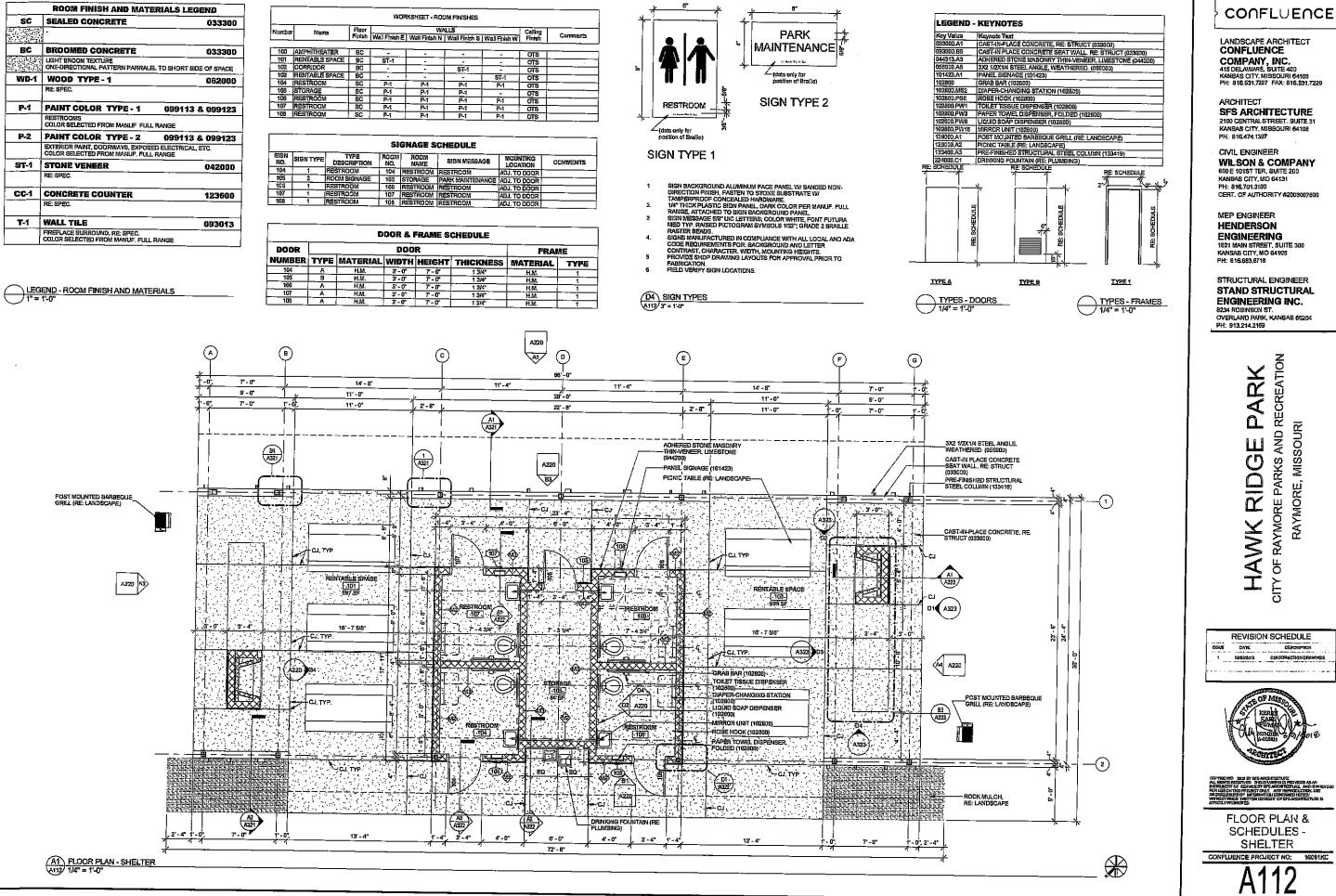
	ROOM FINISH AND MATE	RIALS LEGEND
SC	SEALED CONCRETE	033300
	-	<u>.</u>
BC	BROOMED CONCRETE	033300
63 S	UGHT BROOM TEXTURE ONE-DIRECTIONAL PATTERN PARRALE	TO SHORT SIDE OF SPACE
WD-1	WOOD TYPE-1	062000
	RE: SPEC.	
P-1	PAINT COLOR TYPE - 1	099113 & 099123
	RESTROOMS COLOR SELECTED FROM MANUF FULL	RANGE
P-2	PAINT COLOR TYPE - 2	099113 & 099123
	EXTERIOR PAINT, DOORWAYS, EXPOSE COLOR SELECTED FROM MANUF, FULL	D ELECTRICAL, ETC. RANGE
ST-1	STONE VENEER	042000
	RE: SPEC.	···
CC-1	CONCRETE COUNTER	123600
	RE SPEC.	
<b>T</b> -1	WALL TILE	093013
	FIREPLACE SURROUND, RE: SPEC. COLOR SELECTED FROM MANUF, FULL	

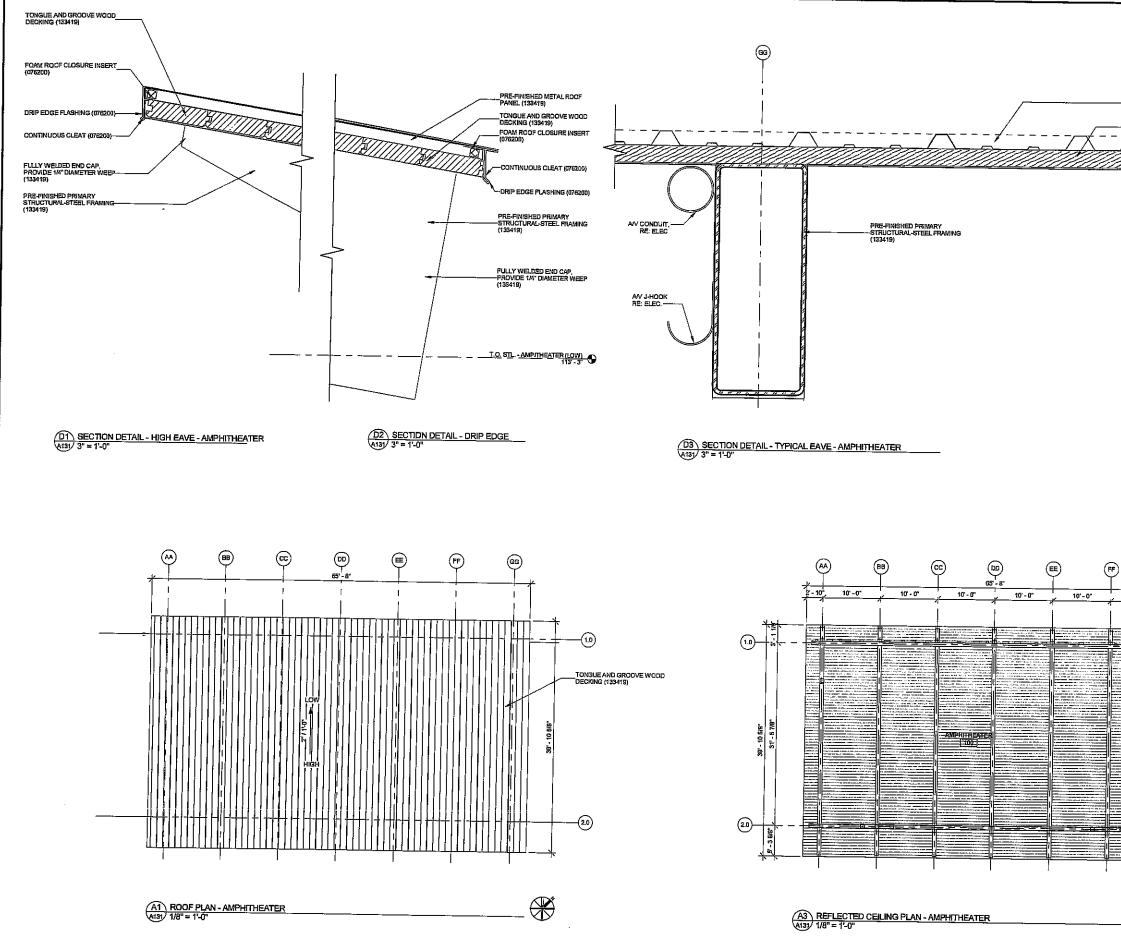
			WOR	KSHEET - ROO	DM FINISHES			
Number	Name	Flaor	L		LLS		Geiling	
		Finish	Wall Finish E	Wall Finish N	Wall Finish S	Wall Finish W	Finish	Comments
100	AMPHITHEATER	BC						
				-	•	-	OTS	
101	RENTABLE SPACE	BC	<u>S</u> T-1	-	-		OTS	
	CORRIDOR	BC		-	ST-1		OTS	
	RENTABLE SPACE	6C		-	-	ST-1	ors	
104	RESTROOM	SC	P-1	P-1	P-1	 P-1	OTS	
105	STORAGE	SC	P-1	P-1	P-1		OTS	
106	RESTROOM	SC	P-1	P-1	P-1	P-1	015	
107	RESTROOM	SC	P-1	P-1	P-1	P-1	ors	
108	RESTROOM	SC	P-1	P-1	P-1		OTS	

·				· · · · · · · · · · · · · · · · · · ·							
SIGNAGE SCHEDULE											
	DESCRIPTION	RÓOM NO,	ROOM	SIGN MESSAGE	MOUNTING	COMMENTS					
1	RESTROOM	104	RESTROOM	RESTROOM	SOOD OT LOA	·······					
2	ROOM SIGNAGE	105	STORAGE			· · · · ·					
1	RESTROOM	106									
	RESTROOM	107	RESTROOM								
1	RESTROOM	106	RESTROOM								
	1 2 1 1		SIGN TYPE TYPE ROOM DESCRIPTION NO. 1 RESTROOM 104 2 ROOM SIGNAGE 105 1 RESTROOM 106 1 RESTROOM 106	SIGN TYPE         TYPE         ROOM         ROOM           1         RESTROOM         104         RESTROOM         14         RESTROOM           2         ROOM SIGNAGE         105         STORAGE         1         RESTROOM         14         RESTROOM         16         RESTROOM         16         RESTROOM         1         RESTROOM         106         RESTROOM         107         RESTROOM         107	SIGN TYPE TYPE ROOM ROOM SIGN MESSAGE DESCRIPTION ND, NAME SIGN MESSAGE 1 RESTROOM 104 RESTROOM RESTROOM 2 ROOM SIGNAGE 105 STORAGE PARK MAINTENANCE 1 RESTROOM 108 RESTROOM RESTROOM 1 RESTROOM 107 RESTROOM RESTROOM	SIGN TYPE         TYPE         ROOM         ROOM         SIGN MESSAGE         MOUNTING           1         RESTROOM         NO.         NAME         SIGN MESSAGE         LOCATION           1         RESTROOM         104         RESTROOM         ADJ. TO DOOR           2         ROOM SIGNAGE         105         STORAGE         PARK MAINTENANCE         ADJ. TO DOOR           1         RESTROOM         106         RESTROOM         RESTROOM         ADJ. TO DOOR           1         RESTROOM         107         RESTROOM         ADJ. TO DOOR					

DOOR & FRAME SCHEDULE											
DOOR			FRAME								
	TYPE	MATERIAL	WIDTH	HEIGHT	THICKNESS	MATERIAL	ТҮРЕ				
104	Α.	H.M.	3'-0"	7-0	1 3/4"	H.M.	1				
105		H.M.	3'-0'	7.0	1 3/4"	H.M.	1				
106	A	H.M.	3'-0"	7-0	1 3/4"	H.M.	<u>;</u>				
107	A	H.M.	3'-0"	7-0	1 3/4"	H.M.	<u>i</u>				
108	A	H.M.	3'-0"	7-0	1 3/4"	H.M.					







PRE-FINISHED METAL ROOF PANEL (133419)

TONGUE AND GROOVE WOOD DECKING (133419)

---CONTINUOUS CLEAT (076200) --DRIP EDGE FLASHING (076200)

## CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANBAS CTY, MIBGOURI 64405 PH: 816,531,7227 FAX: 816,531,7229

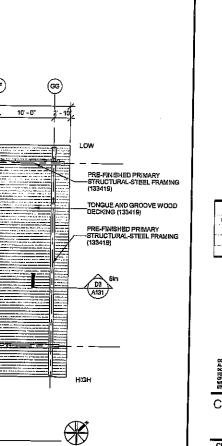
ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 37 KANSAS CITY, MISSOURI 54108 PH: 816,474,1397

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, SUITE 200 KANSAS CITY, MO 64131 PH: 81670.13100 CERT. OF AUTHORITY #2003007699

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MO 64108 PH: 814653.4716

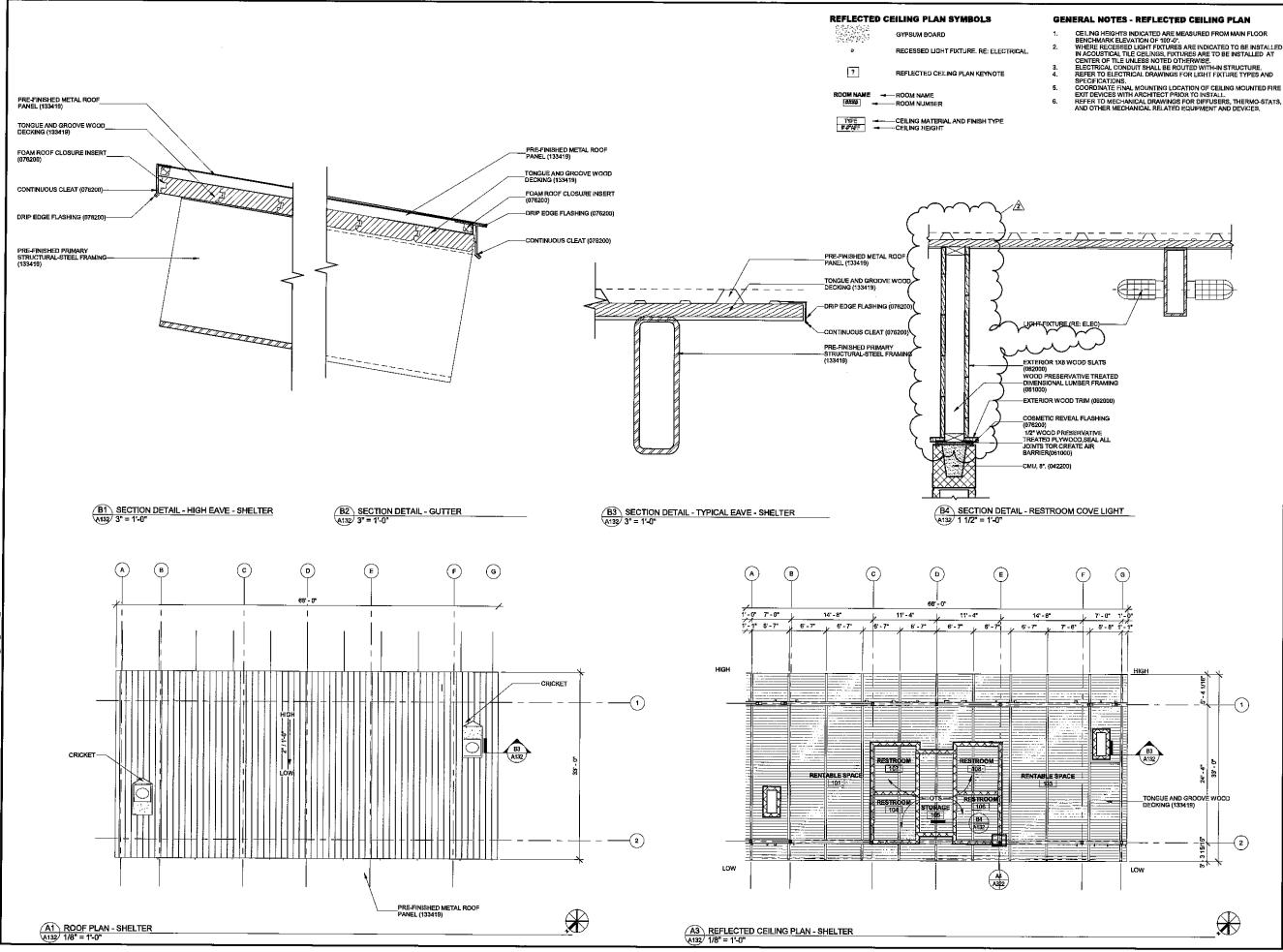
### STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST.

OVERLAND PARK, KANSAS 66204 PH: 913.214.2169



HAWK RIDGE PARK CITY OF RAYMORE PARKS AND RECREATION RAYMORE, MISSOURI





LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816,531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 31 KANSAS CITY, MISSOURI 64108 PH: 816.474.1397

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, SUITE 200 KANSAS CITY, MO 64131 PH: 816,701,3100 CERT. OF AUTHORITY #2003007599

### MEP ENGINEER HENDERSON

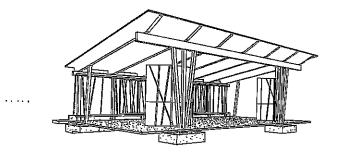
ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MO 64108 PH: 816.663.8718

### STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST.

OVERLAND PARK, KANSAS 66204 PH: 913.214.2169

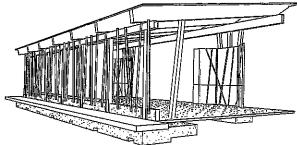


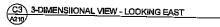


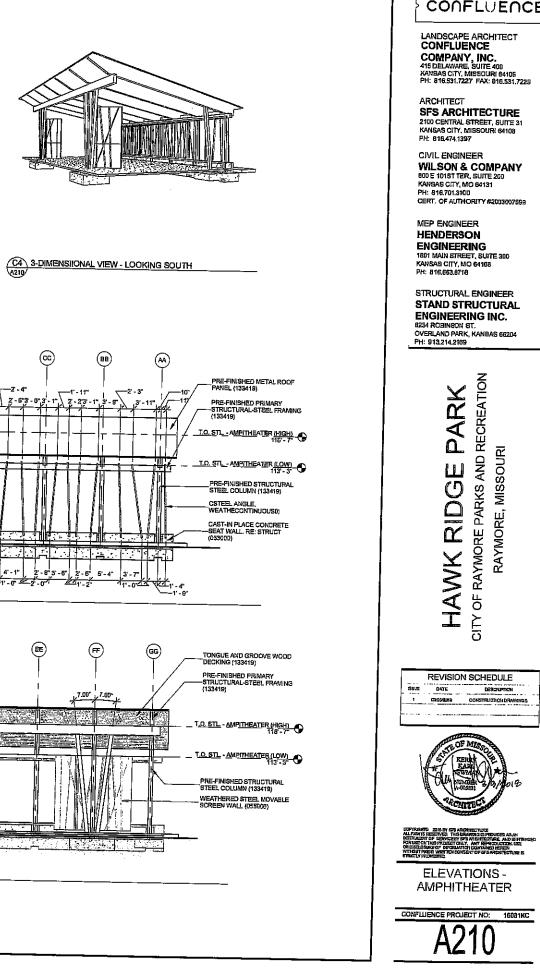


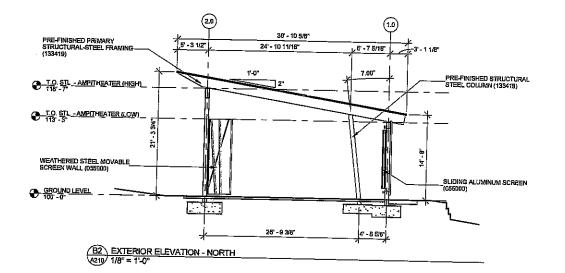


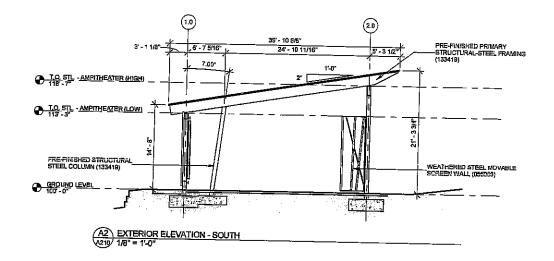
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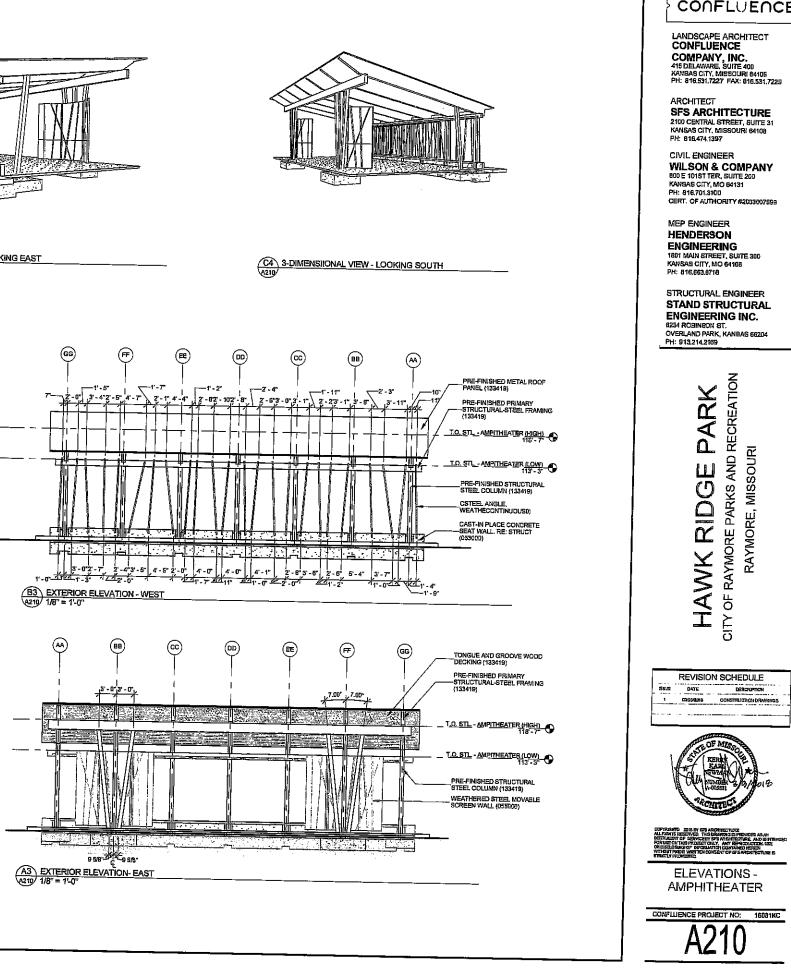


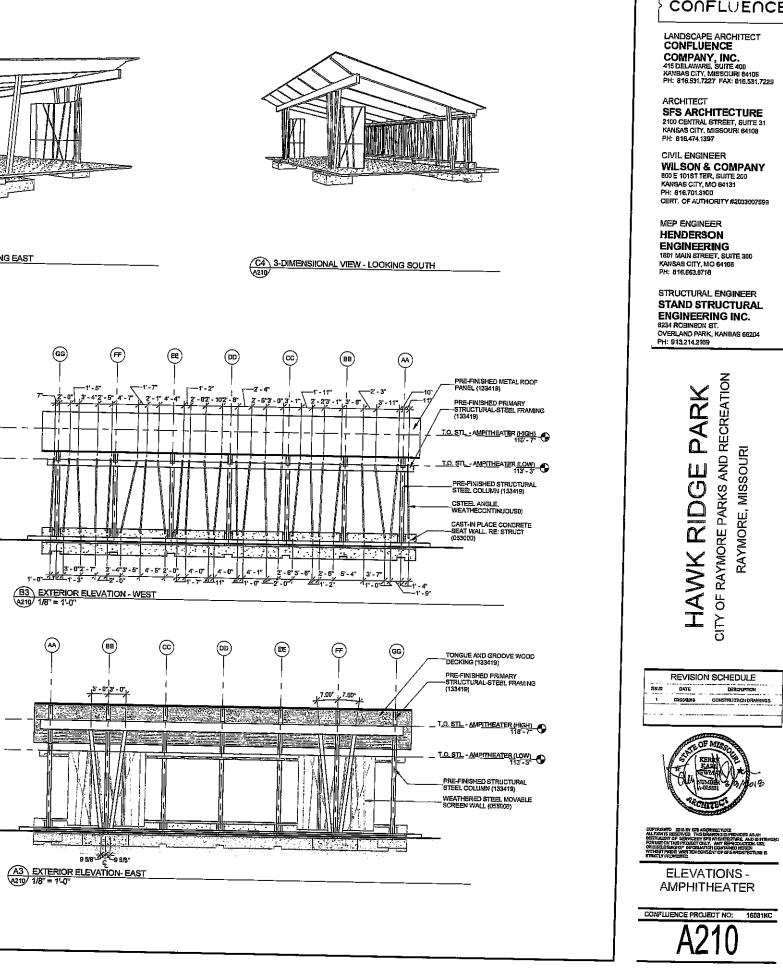




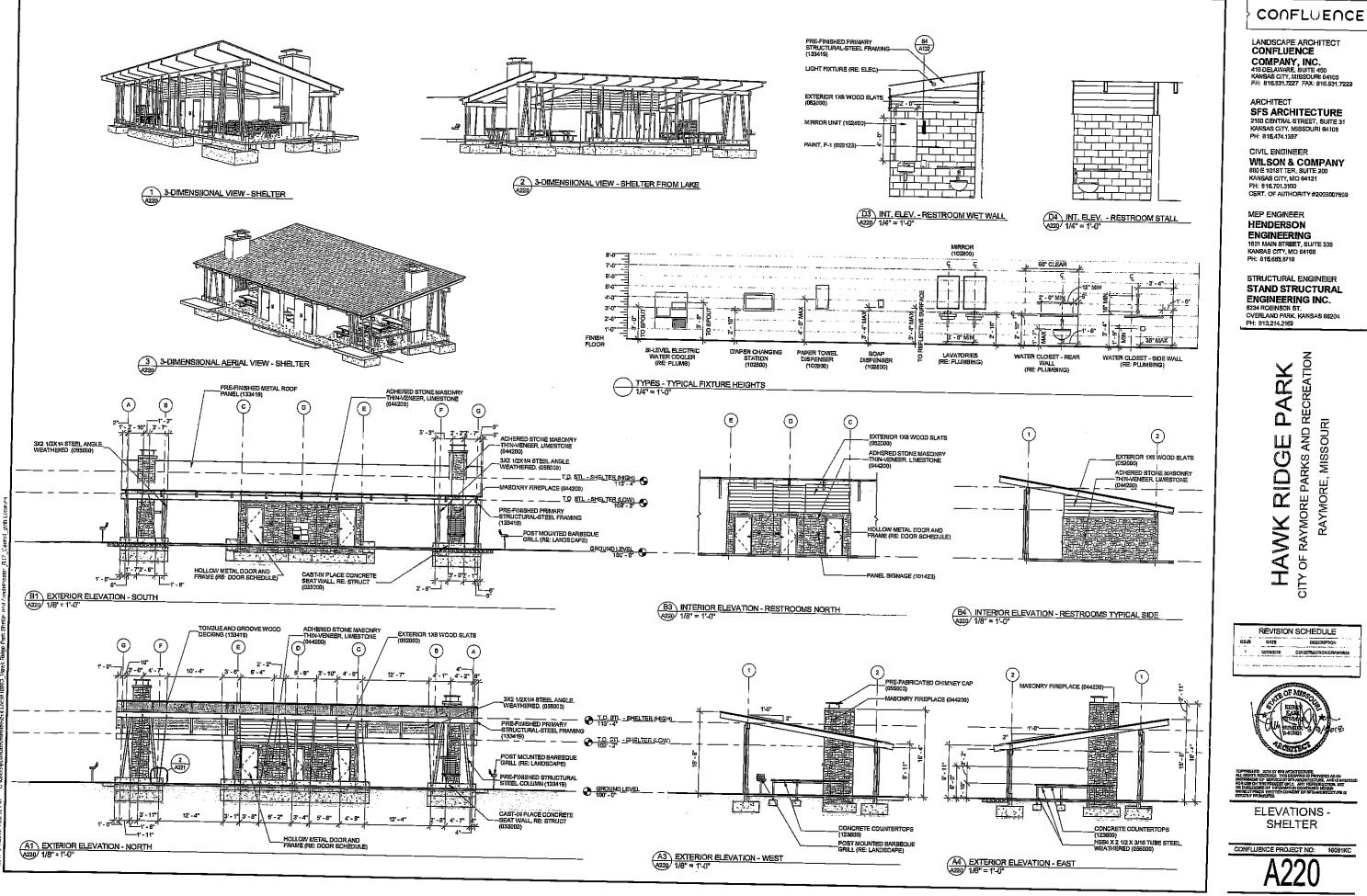




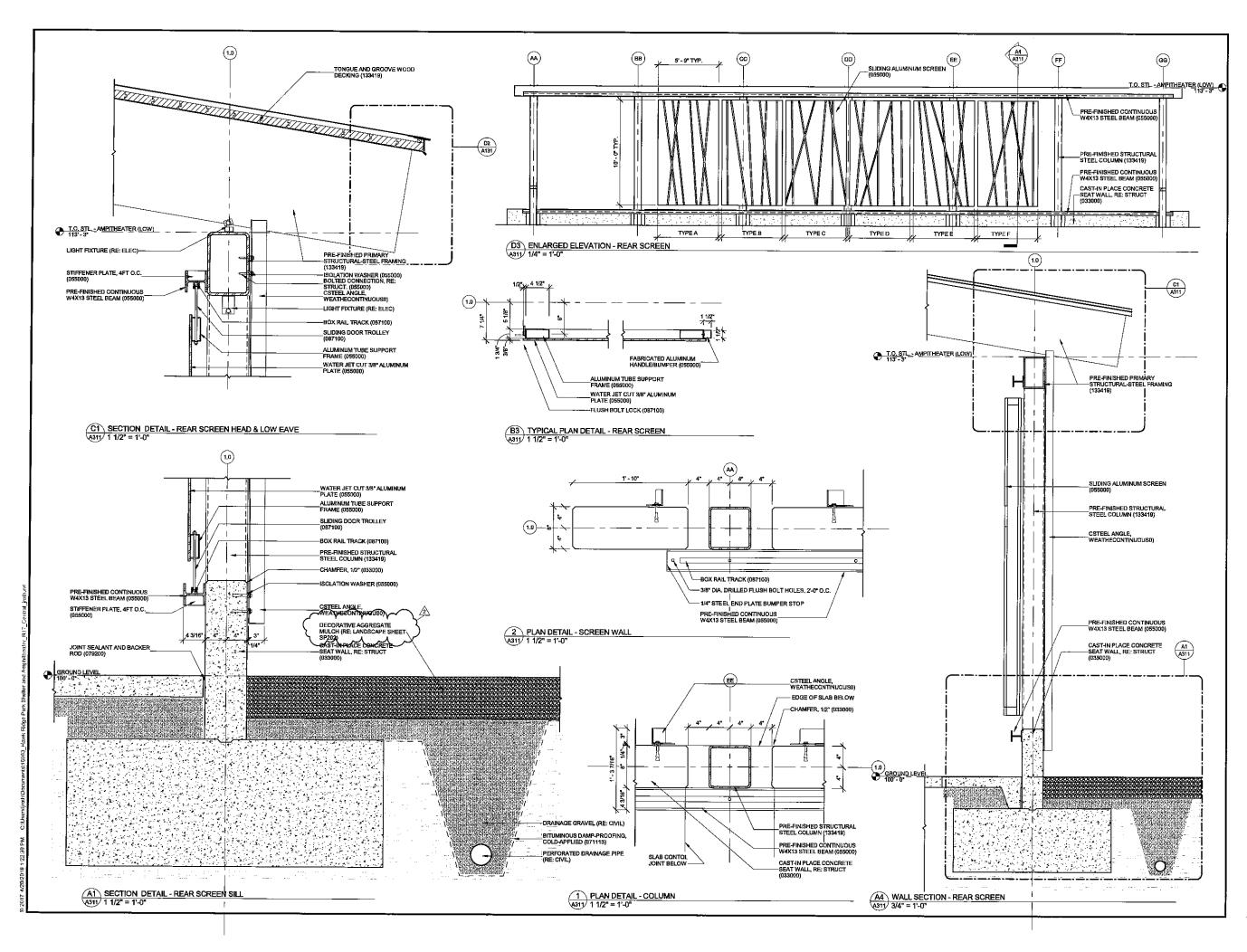




## CONFLUENCE



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LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 31 KANSAS CITY, MISSOURI 64108 PH: 816,474,1397

CIVIL ENGINEER **WILSON & COMPANY** 800 E 101ST TER, SUITE 200 KANSAS CITY, MO 84131 PH: 816,701,3100 CERT. OF AUTHORITY #2003007599

# MEP ENGINEER

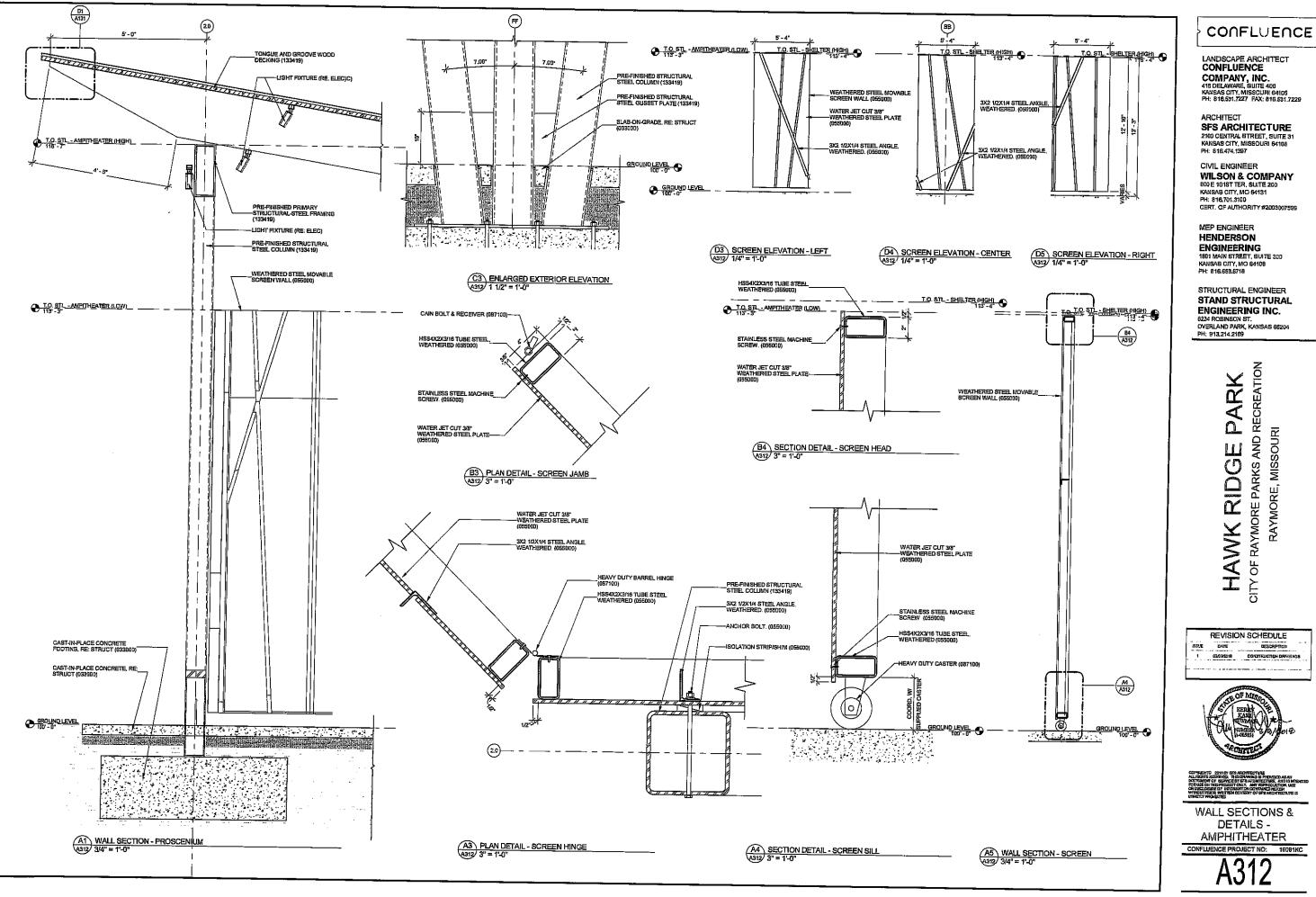
ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MD 64108 PH: 816,663.8718

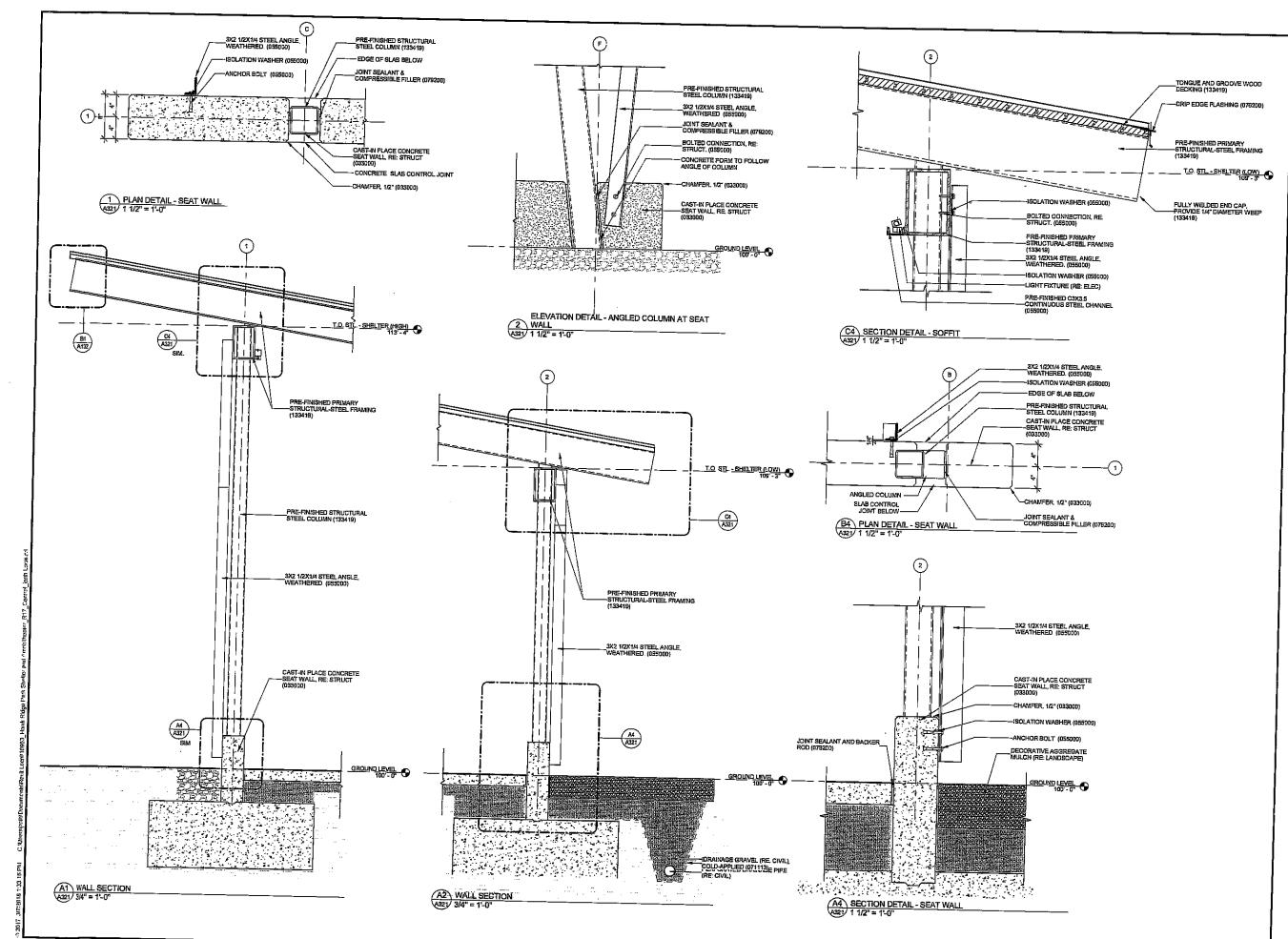
STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 66204

PH: 913.214.2169

HAWK RIDGE PARK CITY OF RAYMORE PARKS AND RECREATION RAYMORE, MISSOURI







LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CUTY, MISSOURI 64105 PH: 816.531.7227 FAX: 616.531.7229

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 31 KANSAS CITY, MISSOURI 64100 FH: 618474.1397

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, SUITE 200 KANSAS CITY, MO 64131 PH: B16.701.3100 CERT. OF AUTHORITY #2003007599

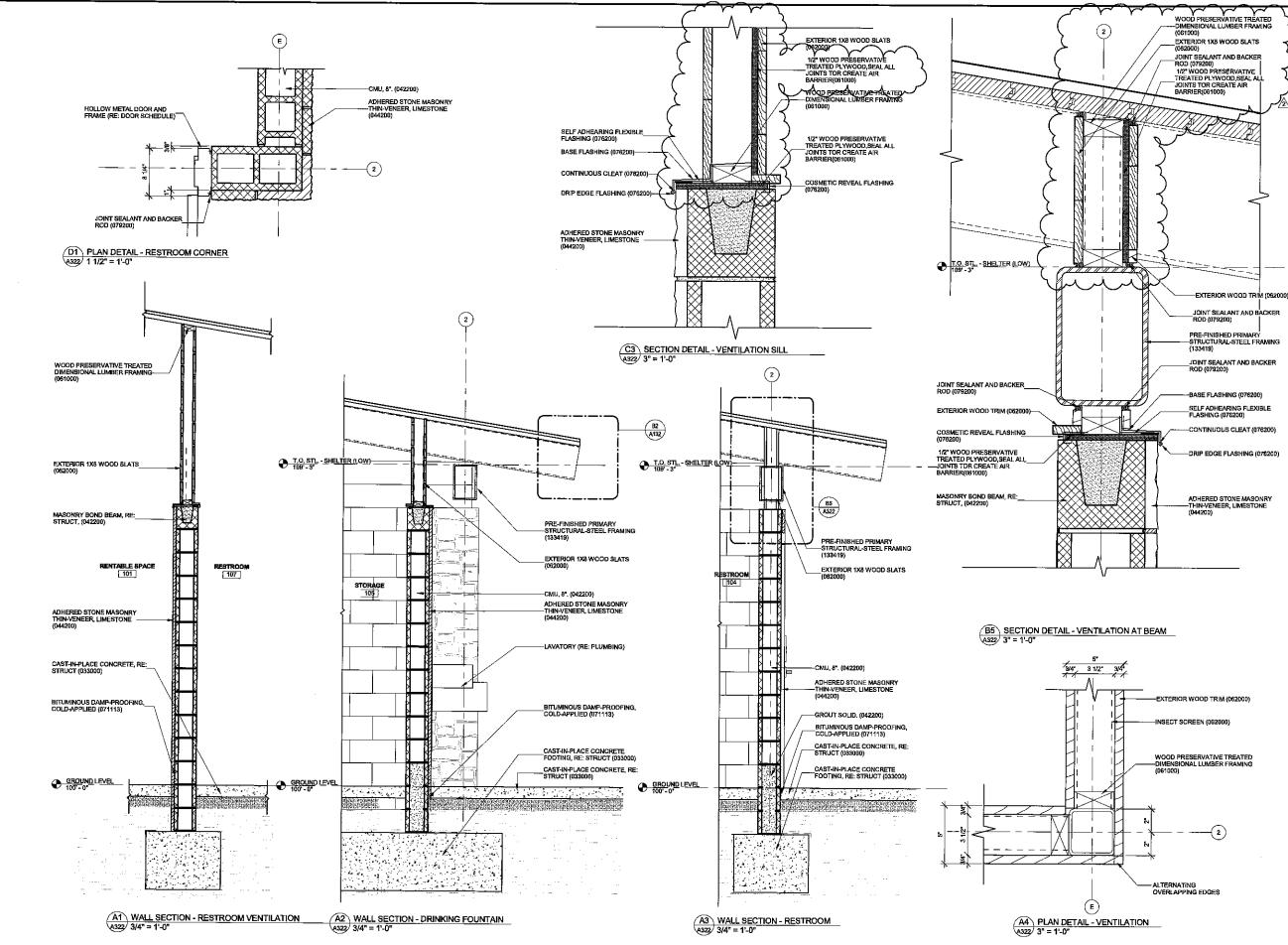
MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MO 64108 PH: 816.683,8718

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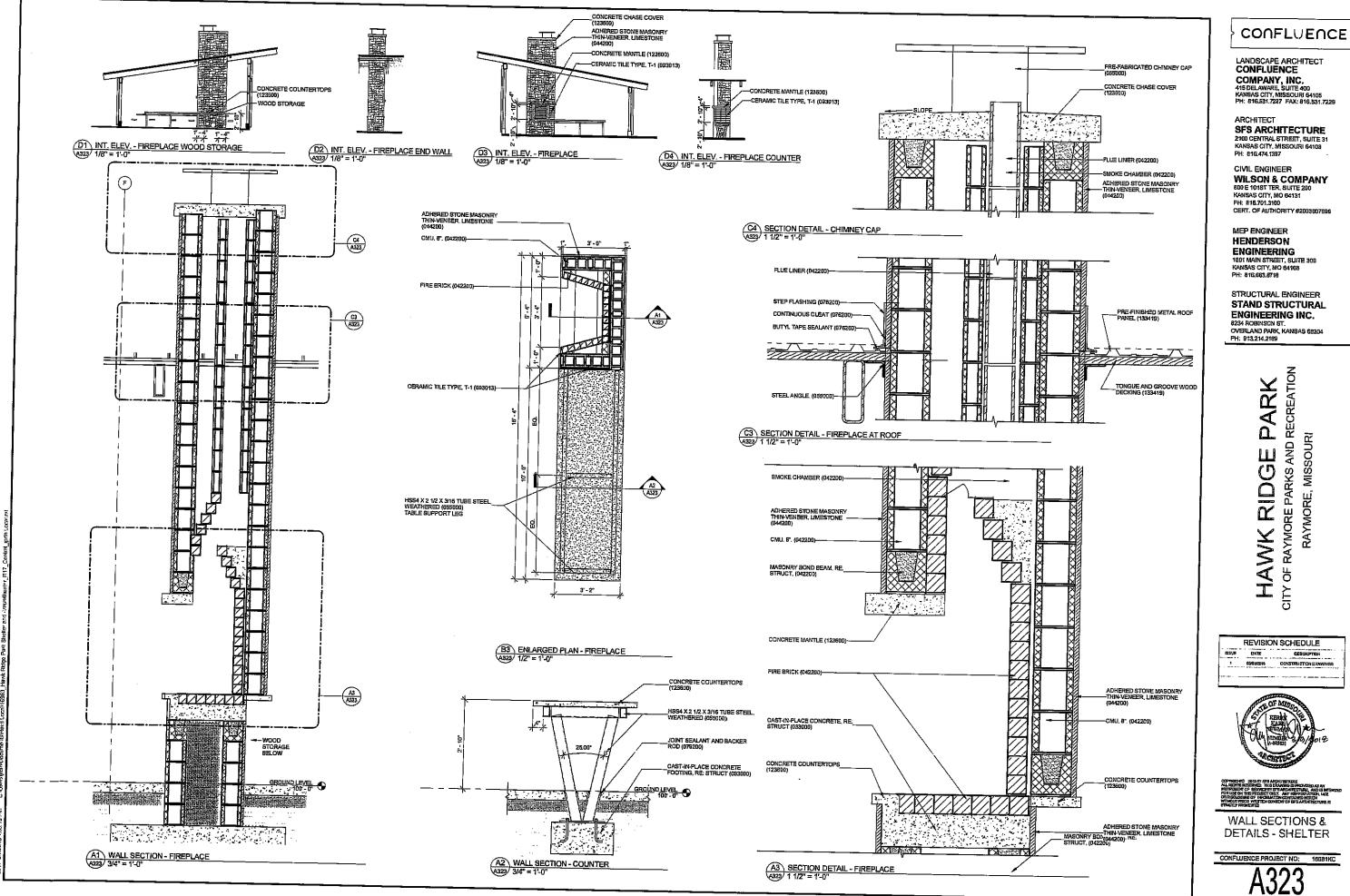
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CONFLUENCE PROJECT NO: 16981KC A322



	<u>GP</u> GP1	GENERAL PARAMETERS STRUCTURAL DRAWNIGS ARE A PORTION OF THE COM DOCUMENTS AND ARE MEMORED TO BE USED WITH ARCHTECTURAL, MERNANCAL, AND DELECTRICAL DRAW COMTRACTOR IS RESPONDED FOR COMPARISON COMTACTOR IS RESPONDED FOR COMPARISON OF THE RESUMANCE AND WORK.	Tract Wings. The He Shop
	GP2	VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO FABRU ANY ITEMS.	
	GP3	THE THERE DRAWINGS SHALL BE COORDNATED WITH ARCHITECT MECHANICAL ELECTRICAL, AND PLUMBING DRAWINGS ADDITIONAL, OFENHAGE, LEUTINAL, DOMENITH, FL ADDITIONAL, OFENHAGE, LEUTINAL, DAMENISTI, A LOGATIKIN, SIZE AND REMFORTSHEATH OF ALL OPENIN RESPECTIVE TRADES BEFORE FARILLENT REPORT DIGGEPANCES ANDOR WITHFFERENCE FROILENST ARCHITECT NO STRUCTARY. ENGOLESS	
	GP4	THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS, REFER TO THE FROLECT SPECIFICAT ADDITIONAL REQUIPEMENTS IN OTHE AND DETAILS ON STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OF GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DE PROVIDED, CONSTRUCTION SHALL BE AS SHOWN FOR WORK	IONS FOR THE VER THE ETALS ARE SIMLAR
	GP5		
	GP8	CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SE SHALL RECOGNIZE AND CONSDER THE EFFECTS OF TH MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.	
	GP7	RETAINING WALLS AND BASEMENT WALLS WHICH THE T SLADS SHALL NOT BE BACKPILED UNTEL THE UPPER SI PULL DEBIGN STRENGTH, UNLESS ADEQUATE BRACING PROVIDED AT THE TOP OF THE WALL	o upper AB3 reach IS
	GPð	AS USED IN GENERAL NOTES AND THROUGHOUT STRUC THE TERM "CONTRACTOR IS DEFINED TO INCLUEE ANY FOLLOWING SOFIERAL CONTRACTOR AND THEIR SUBCC CONSTRUCTION MANAGER AND THEIR SUBCCONTRACTO INSTRUCTION MANAGER AND THEIR SUBCCONTRACTO INSTRUCTION MANAGER AND THEIR SUBCCONTRACTO	CTURAL DRAWINGS: OR ALL OF THE INTRACTORS, RS, FABRICATORS,
	<u>DP</u> DP1	DESIGN PARAMETERS THE GTRUCTURE & DESIGNED IN ACCORDANCE WITH T INTERNATIONAL BUILDING CODE, 2012 EDITION.	THE ICC
	DP2	UNFORM LIVE LOADS: TYPXCAL ROOF (GOVERNED BY SNOW) DRIFT LOAD AS SHOWN ON PLAN	20 PSF
	DP3	CONCENTRATED LIVE LOADS: N/A	
	DP4	SNOW DESKAN PARAMETERS: GRUONID SNOW LOAD (Pg) FLAT ROOF SNOW LOAD (Pf) SNOW EXPOSIVE FACTOR (CF) SNOW EXPOSIVE FACTOR (I) SNOW LOAD THERMAL FACTOR (C)	20 PSF 20 PSF 1.0 1.0 1.2
	DP5	EARTHQUAKE DESIGN PARAMETERS: SEISMIC IMPORTANCE FACTOR (I/E)	1.0
I		S5 81 SEISMIC BITE CLASS	0.1140 0.067g D
I		Sds 8d1 SHORT-PERIOD SITE COEFFICIENT (Fa)	0.122g 0.107g
ĺ		LONG-PERIOD SITE COEFFICIENT (Fy) SEISMIC DESIGN CATEGORY SEISMIC-FORCE-RESISTING SYSTEM:	1,6 2,4 B
I		STEEL SYSTEM NOT SPECIFICALLY DETAILED F SEISMIC BASE SHEAR (V) N-S SEISMIC BASE SHEAR (V) N-S	OR SEISMIC RESISTANCI 1.5 k 1.5 k
I		SEISMIC RESPONSE COEFFICIENT (Ca) N-S SEISMIC RESPONSE COEFFICIENT (Ca) E-W DESPONSE MONSE COEFFICIENT (Ca) E-W	0.04
ļ		STELL SYSTEM NOT SPECIFICALLY DETAILED F SEISMIC BASE SHEAR (V) NEW SEISMIC RUSPONSE COEPIFICIET (C) N.S SEISMIC RUSPONSE COEPIFICIET (C) N.S BESISMIC RUSPONSE MODEFICIENT (C) N.S HERSPONSE MODEFICIENT ACTOR (R) N-S HERSPONSE MODEFICIENT ACTOR (R) N-S HERSPONSE MODEFICIENT ACTOR (R) N-S HERSPONSE MODEFICIENT ACTOR (R) N-S HERSPONSE MODEFICIENT (C) N-W SEISMIC ANALYSIS PROCEDURE: EQUIVIDANCY FACTOR (R) F-W SEISMIC ANALYSIS PROCEDURE:	3.0 3.0 1.0 1.0
	DP6	WIND DEBIGN PARAMETERS: BASIC WIND SPEED (V) (8 SECOND GU8T)	116 MPH
		WIND DESIGN PARAMETERS: LARD WARD SPEED (N. (8 SECOND GUST) RISK CATEGORY WIND EXPOSURE CATEGORY MIRTINUA, PRESSURE COEFFCIENT BARIO DESIGN PRESSURE & RODE NET WIND UPUET & RODE	l C 0.00 (OPEN) PER BLDG DESIGNER PER BLDG DESIGNER
	DP7	ALLOWASLE WALL DEFLECTIONS: EXTERIOR (CLADONG DESIGN) (BRUCK VENEER DESIGN) INTERIOR	L/380 L/600 ≤ 0.3" L/360
	DP8	ALLOWABLE FRAMING DEFLECTIONS: ROOF LVE LOAD ROOF TOTAL LOAD	L/380
		FLOOR TOTAL LOAD	L/240 L/360 L/240
	DP9	ABSUMED FUTURE CONSTRUCTION: VERTICAL HORIZONTAL	NONE
	EF1	FOUNDATION PARAMETERS SUE AND BOTTOM ELEVATIONS OF FOOTINGS NAVE BEE THE ASSUMED MINIMUM ALLOWABLE BEARING PRESSING INCREASE FOR INDIVIDUAL COLUMN FOOTINGS PER THE	N ESTABLISHED BASED ( E OF 2500 PSF WITH A 20
		INCREASE FOR NOINDUAL COLUMN FOOTNIGS PER THE PREPARED BY ALPHA-OMEGA GEDTECH, NO, DATED AU EXCAVATION PROGRESSES, UNANTICEATED SOLL COND CHANGES, CONTACT STRUCTURAL ENGINEER OF RECOR THESE CHANGES.	GEOTECNNICAL REPORT GUST 2, 2017, AB ITIONS MAY REQUIRE ID FOR EVALUATION OF
	FP2	ALLOWABLE SOL BEARING PRESSURE Continuous Footings	2500 PSF 3000 PSF
ĺ	FP3	MINIMUM FOOTING DEPTH FOR FROST PROTECTION (BELOW FINISHED GRADE)	36 IN.
	FF4	DESIGN EARTH PRESSURE COEFFICIENTS (BACKFILLS); ACTIVE	0,25 4.2 0,4
	FP5	GEOTECHNICAL ENGINEER IS SOLE JUDGE OF THE SUITA MATERIAL TO SUFFORT FOUNDATIONS, BEARING MATERI BY GEOTECHNICAL ENGINEER SEFORE FOUNDATION INST PROCEED WITHOUT APPROVAL	
	FF8	FROR TO COMMENCING EARTHWORK, CONTRACTOR SH. FAMILIAR WITH ALL ASPECTS OF THE GEOTECHNICAL REI GEOTECHNICAL ENGINEER IF QUESTIONS OR DISCREPAN	
8	ייייייייייייייייייייייייייייייייייייי	ALL FLL WITHIN THE BUILDING FOOTPRINT AND EXTENDED DIRECTED BY THE GEOTECHNICAL, ENGINEER SHALL BE PLACED IN ACCORDANCE WITH THE GEOTECHNICAL REP	
ľ	i		uni.
ļ			
1			

SHOP DRAWINGS SHALL INCLUDE CONNECTIONS AS WELL AS SIZE, SPACING, AND GRADE OF ALL MEMBERS, PLANS AND ANY DETAIL NO. MECESSARY FOR DETERMINING FIT AND PLACEMENT SHALL ALSO BE INCLUDED. IF THE SHOUD DRAWINGS OFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGATURE OF AN ENGINEER REGISTERED IN THE APPROPRIATE STATE, ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ACTURIECT AND ARE SUBJECT TO REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER OF REDORD. TT. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY THE CONTRACTOR, ROLUDING: PREMANUFACTURERED STEEL SHELTERS SUB SHALL BEAR THE SEAL AND DISINATURE OF AN ENGINEER REGISTERED IN THE APPROPRIATE STATE AND SHALL BE SUMITIED TO THE ARCHITECT PHORY TO FARMEXCATON AND CONSTRUCTION. CALCULATIONS SHALL BE INCLUDED FOR ALL CONSTRUCTION THE STRUCTURE, CONSERVING LOCALUED EFFECTS ON STRUCTURE, ELEMENTS INDUCED BY THE CONNECTION LOCAL. THEN'S THAT ARE DESIGNED BY THE CONTRACTOR SMALL BE DESIGNED TO RESIT THE LATE LOADS MUCCATED IN STRUCTURAL NOTES, LEAD LOAD, SELF WEIGHT, ANY ADDITIONAL LOADING NDICATED ON PLANS AND DETAILS, SNOW DRIFT, AND A NET WIND UPLIFT. SU7 **CN15** ITEMS THAT ARE DESIGNED BY THE CONTRACTOR SHALL NOLLIDE ANY RELEVANT TECHNICAL LITERATURE FROM MANUFACTURER. ALSO PROVIDE A CERTIFICATION FROM THE MAURACTURER SHOWNG THE PRODUCT IS IN COMPLIANCE WITH ALL APPLICABLE COMES AND STANDARDS. THE CONTRACTOR HALL COORDINATE SEISING RESITAINTS OF INSCHWICH, PUNBING, AND ELECTRICAL EQUIPMENT, MACHINEY, MA OBSOCHTED PHNO WITH THE STRUCTURE, ANY CONNECTIONS TO STRUCTURE SHALL CONFORM TO SACE 7, COMPTER 13 AND SMALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE APPROPRIATE STATE, AND SHALL BE SUBMITTED TO THE ARCHITEET PHOLO TO FARBURATION. SU9 FELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS BHALL BEAN THE SEAL AND SIGNATURE OF AN ENGINEER REGISTERED IN THE APPROPRIATE STATE AND SHALL BE SUBMITTED TO THE ARCHITECT PROG TO CONSTRUCTION. SU10 <u>BP</u> SP1 SPECIAL INSPECTION SPECIAL INSPECTION PROGRAM SHALL CONFORM TO CHAPTER 17 OF THE THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM THE REQUIRED TEATS AND SPECIAL INSPECTIONS WITH CULALIFICATIONS DESORBED FOR ISC CHAPTER 17 AND THE PROJECT SPECIFICATIONS. SP2 SPECIAL INSPECTION REPORTS SHALL BE FURNISHED TO BUILDING OFFICIAL, OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND CONTRACTOR. ICE SP3 CM4 SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING THAT THE BTRUCTURAL WORK WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, PERPORMED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. 8P4 CONCRETE NOTES CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE BC. WHERE NEW CONCRETE IS PLACED AGAIN&T EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE CLEANED AND ROUGHENED TO A MINIMUM OF 1/4" AMPLITUDE. CNZ ALL EXPOSED EDGES OF CONCRETE MEMBERS SHALL BE CHAMFERED 1/2" U.N.O. CN3 CONNECTED THE UNCU. CONNECTED THE UNCUL. THE SUBJECT OF CONTINUES THE CONTINUES BUTCH BE APPROVED BY THE STRUCTURAL DRAWINGS BUALL BE APPROVED BY THE STRUCTURAL ENGINEER PROR TO POURING CONCEPTE CONJUTE BURGEDDE IN SLASS SHALL NOT BE LARGER IN CUTSIED DMENSION THAN ONE STAGED CLOBER THAN THREE DUMMETERS ON CENTER. STAGED CLOBER THAN THREE DUMMETERS ON CENTER. CH.2 VERTY ALL BLOCK OUTS WITH ARCHITECTURAL MECHANICAL, BLECTICAL, AND PLUMBING REQUIREMENTS FRIGO TO POURING. CN4 RE-UTIONING., ELG: HIGGL, AND PILMEING REGUIREMENTS
 PRIOR TO POURING.
 CONCRETE REENCROCEMENT:
 CAS1 DETAILING, FABRICATON, AND PILACEMENT DR
 REINFORCEMENT SMALL CONFORM TO ACISIS.
 CONTROL REPORT TO BE ASTM AND SEASON WILL
 SECURITY SEASON SEASON WILL
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 SECURITY SEASON SEASON SEASON SEASON WILL
 SECURITY SEASON SEASON SEASON SEASON SEASON WILL
 SECURITY SEASON SE CN5 CMS CM6 OPENING, U.N.O. UIAJO, INSTALL WWF 11/2" FROM TOP OF ALL SLABS ON GRADE, TOPPING SLABS ON DECK, OR TOPPING SLABS ON PRECAST, LAP JOINT SWO FULL MESHES BUT NOT LESS THAN 8" AT CONSTRUCTION JOINTS, LOCATE WWF AT MID DEPTH OF SLABS. CN5.7 CM7 DEPTH OF SLAS. CNB CMB CRETE COMPRESSIVE STRENOTHS SHALL BE VERIFIED BY IQARD 28-DAY CYLINDER TESTS PER ASTM C39 AND SHALL BE AS 
 Λε = 3,000 PSI

 Γc = 4,000 PSI

 Λε = 4,000 PSI

 Λε = 4,000 PSI
 SURBS SPECIAL INSPECTOR SHALL, BE NOTIFIED IF ANY WATER (S TO BE ADDED IN FIELD, CNS

SUBMITTALS GENERAL CONTRACTOR 'TO PROVIDE A SHOP DRAWING SUBMITTAL LOG ITEMIZING ALL PROPOSED SUBMITTALS FOR APPROVAL BY STRUCTURAL ENGINEER OF RECORD.

ALL SHOP DRAWINGS SHALL BE CHECKED BY THE FABRICATOR AND APPROVED BY THE CONFELL CONTRACTOR PROC TO SHOP DRAWING SHOP THE CONFIGURATION OF THE CONFIGURATION SHOP DRAWING SHOP DRAWING TO THE CONTRACT DOCUMENTS. CONTRACTOR & DEFINISY CHECKED FOR ANY CARACTERISTIC CONTRACTOR & DEFINISY CHECKED FOR ANY CARACTERISTIC CONTRACTOR BERGING, OLI CARASTONIA DI OP DRAWINGS.

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PROR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING: CONCRETE MIX DESIGNS (5 DAYS BEFORE POUR, MIN.)

CONCRETE REINFORCEMEN

MASONRY REINFORCEMENT
 PREMANUFACTURED STEEL SHELTERS

<u>SU</u> 901

SU2

SU3

SU4

#### CONCRETE NOTES (CONT.)

- CONCRETE: NOTES (CONT.). CONCRETE: NOTES (CONT.). CONCRETE: NOTES (CONT.). CONCRETE: NOTES (CONT.). CONCRETE: NOTACINE. CONCRE

- PROVIDE MINIMUM 8000 PSI NON-SHRINK, NON-METALLIC GROUT UNDER COLUMN BASE PLATES, GROUT BHALL NOT CONTAIN GYPSUM, COLUMN BASES TO BE GROUTED BEFORE PLACEMENT OF CONCRETE TOPPING ON STEEL FORMS, CN10
- MINIMUM A" CONCRETE SLAB ON GRADE WITH WWF 8X6-W2,1 X W2,1, CYER VAPOR BARRIER, OVER A MINIMUM 6" COMPACTABLE, TRUMMABLE, GRANULAR FILL TYP, U.N.O. ON DRAWINGS AND GEGTECHNICAL REPORT CN1
- CN12
- PROVIDE SLAB CONTROL JOINTS AS FOLLOWS, U.N.O.: CN121 SLASS (CM KING LINE WHERE PRACTICAL) AND: SLAS-MARANCE. SCHEMESTER THE PRACTICAL) AND: SLASS SCHEMESTER SCHEMESTER SCHEMESTER CN123 CONTROL JOINT LAYOUT SHOULD MAINTAIN AN APPROXIMATE ASPECT RATIO OF 1.1. PESIMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES TO BE HOT-OP GALVACED ANTER HARDCRATION, U.N.O. NO LOADS ON WEDS SMULL BEFLACED ON EMBEDDED PLATES ON ANGLES FOR A MINISMM OF TAXYS AND ANGLES FOR A MINISMM OF CASTING. CN13
- EPOXY REPAIR ADHESIVE SHALL CONFORM TO ASTM C881 AND SHALL BE A TWO-COMPONENT, LIQAD EPOXY WITH NON-SAG CONSISTENCY AND A LONG POT LIFE. THE EPOXY ADHESIVE SHALL BE SULTABLE FOR USEIN DRY OR DAMP CONDITIONS, INIMIAM SHILAR STREAMST B-KALL BE GOO PER, IMINIANY TENSLE STREAMST SHORTH SHALL BE ADD REAMST SHORTH SHALLANTON SHALL BE N SHORTH SHALL BE AND REAMST SHORT SHALLANTON SHALL BE N SHORTH SHALL BE MENT. CN14
- CONCRETE MASONRY NOTES CONCRETE MASONRY WORK SHALL CONFORM TO CHAPTER 21 OF THE BC. CM CM1 CM2
- ALL WALLS ARE NOMINAL & CARU STACKED WITH A RUINNING BOND PATTERN UNLESS NOTED DTHERWISE ON THE DRAWINGB, JOINT RENFORCING SHALL NOT EXTERD THROUGH CONTROL, JOINTS, BOND BEAM REMFORCING SHALL BE CONTRUCTUS THROUGH CONTROL, JOINT INTERRECTING WALLS SHALL BE BONDED BY UVERLAPPING COURSES.
  - ALL LAIRS IN MALL BE NORMAL WEIGHT CONCETTE MASCHRYWY. MINIMUM COMPRESSIVE STRENCTI + 1,00 PSI (ASSEMBLY Pm + 1,00 PSI) AD COMPORT TO ASTIN GCI, UNITS SHALL BE BARNED AN DI STETED IN ACCOMPANE WITH ASTIM CIAD. UNIES JANL BE BARNED AND TSTEDE IN ACCOMPANE WITH ASTIM CIAD. UNIES JANL BE BARNED AND TSTEDE MOT EXCEED LOSSY. CONCRETE MASCHRYWAILS SIMUL SE REINFORCED AS SHOWN ON THE PLANS AND DETAILS AND, F NOT SHOWN, SHALL BE REINFORCED AN ONTED BELOW.

  - VERTICAL CONTROL JOINTS HALL BE SPACED USING THE LESSEN OF ETHERA 14:1 LENGTH TO HEIGHT RATIO RX 25 FT ON CENTER. COORDWATE JOINTS WARCH DRAWINGS SOND BEAM REINFORMS SHALL BE CONTINUOUS TINKOUGH CONTROL JOINTS. CONTINUOUS TINKOUGH CONTROL JOINTS. CONTRUCUS TINKOUGH CONTROL JOINTS. MIXIMUM OF S WAY FROM MEADER BEAMING LOCATIONS AUX 10 CARE VERTICAL CONTROL JOINTS ADJACENT TO CORNERS OR WALL INTERSECTIONS WITHIN ADSTANCE EQUAL TO HALF THE CONTROL JOINT SHORE SOLAL TO HALF

  - UNLESS NOTED OTHERWISE, MORTAR SHALL BE TYPE S AND SHALL CONFORM TO BC SECTION 2103 AND ASTM C270. MORTAR BED JOINT THICKNESS SHALL NOT EXCEED 5/8".
  - GROUT: CM7.1 GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI (TYPE S) AND SHALL CONFORM TO ISC SECTION 2103 AND ASTM CAR'S. GROUT SHALL CONSIST OF MOTURE OF CEMENTITIOUS MATERIALS AND AGGREGATE TO WHICH BUFFCIENT WATER HAS BEEN ADODE TO CAUSE THE MOTURE TO FLOW WITHOUT SEGREGATION OF THE CONSTITUENTS.
  - CONSTTUENTS. CONSTTUENTS. CAT 2 GROUT CELLS SOLD AT REINFORCING, BOND BEAMS, NEERTS ANGHORS, BLEVATOR GUDE FALS, AND 24 BELOW BEAMING POINT OF STEEL SECTIONS 300, UT 21 D TO BE FREE OF PROTRUBING WORKAR AND DROPPINGS. GROUT BOND BEAMS OVER OFENINGS IN OWE CONTINUOUS POUR SUPPORT AND SECURE REINFORCEMENT AGAINST DIPLICEMENT EFFORTE BROITING.
  - PROVIDE MASONRY HEADERB OVER OPENINGS LARGER THAN 12" N LENGTHAE INDICATED IN SCHEDULE BELOW, U.N.O. HEADERS SHALL BEAR 8" ON EACH SHE OF OPENING. FERMANEMITY EXPOSED STEEL HEADERS TO BE HOT-OIP GALVANIZED AFTER FABRICATION, U.N.O.
  - CM8.1 ALL STEEL HEADERS OVER 8-0" LONG SHALL HAVE #5 DEA x 2-6" SPACED TO MATCH WALL REP/FORCING.

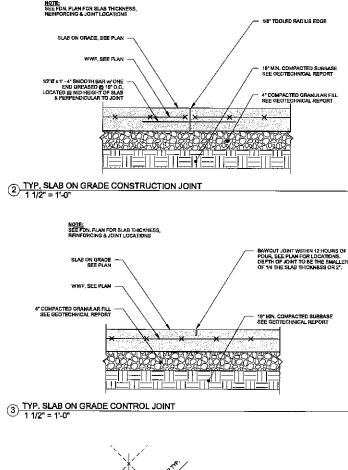
# CM CONCRETE MASIONRY NOTES (CONT.) CMB PROVIDE MASIONRY HEADERS OVER O'PENINGS LARGER THAN 12" IN LENGTH AS PRICATED IN SCHEDULE BELOW, LINAD, NEADERS SHALL BEAR 8" ON EACH SBE OF O'PENING, PERMANENTLY EXPOSED STEEL HEADERS 10 BEHOT-DIP GALVAREED AFTER FASHICATION, UNAD.

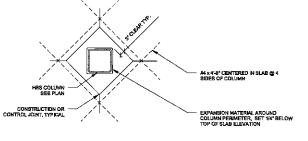
#### ALL STEEL HEADERS OVER 6'-0" LONG SHALL HAVE #3 DBA x 2-5" SPACED TO MATCH WALL REINFORCING.

		AXIMUM MASONRY OPEN	NG
	4-07	8-0	12-5
2	WT459	WT5#15	_
•	W8x10 w/ PL 5/16x7 - OR - (1) COURSE BOND BEAM	W8x10 w/ PL 5/16x7 - OR- (2) COURSE BOND BEAM	W16x01 w/ PL 5/16x7
107	W8x10 w/ PL 5/18x9 - OR - (1) COURSE BOND BEAM	W8x10 w/ PL 5/16x9 - OR- (2) COURSE BOND BEAM	W18x31 w/ PL &/16x9
12	W8x10 w/ PL 5/16x11 - OR - (1) COURSE BOND BEAM	W8x15 w/ PL 5/18x11 - OR - (2) COURSE BOND BEAM	W18x31 w/ PL 5/16x11
	WI	<u>WEW</u> PLATE	BOND BEAM
-	<u>↓</u> 3/16 → 3-6	(2) #5 CON	п. — Х. Д.

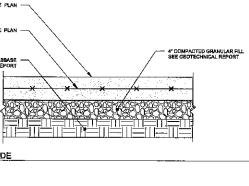
### TYPICAL STRUCTURAL ABBREVIATIONS (PERIODS w) ABBREVIATIONS MAY BE OMITTED WITHOUT CHANGING MEANING).

4	AND	1	ANGLE
69	AT	- Ēr	LMELOAD
ĂВ.	ANCHOR BOLT	ĹĿ LL,H,	LONG LEG HORIZONTAL
AC.L	ALCERON CONCEPT INFERENCE		LONG LEG VERTICAL
A.E.S.S.	AMERICAN CONCRETE INSTITUTE ARCHITECTURALLY EXPOSED	LP.	
A.E.S.J.	STRUCTURAL STEEL		LOW POINT
	STRUCTURAL STEEL	LB,	POUND
A.I.S.C.	AMERICAN INSTITUTE OF STEEL	LSH.	LONG SIDE HORIZONTAL
	CONSTRUCTION	LS.V.	LONG SIDE VERTICAL
A.S.T.M.	AMERICAN SOCIETY FOR	LONG.	LONGITUDINAL
	TESTING AND MATERIALS	LT. GA.	LIGHT GAGE
ADD.	ADDENOUM		
ADD'L	ADDITIONAL	MEP.	MECHANICAL, ELECTRICAL,
ALT.	ALTERNATE	MLEP.	PLUMBING
ARCH.	ARCHITECTURAL	M.B.J.C.	PLUMBING
Jana H,	ARCHITECTORAL	MLELJ, Li,	MASONRY STANDARDS JOINT
			COMMITTEE
B.P.	BASE PLATE	MAX	MAXIMUM
B/	BOTTOM OF	MECH	MECHANICAL
BLDG.	BULDING	MFR.	MANUFACTURER
BRG.	BEARING	MIN	MINIMUM
		MISC.	MISCELLANEOUS
CJ.P.	CAST IN PLACE	MTL	METAI
čJ.	CONSTRUCTION JOINT		me inc
C.M.U.	CONCRETE MASONRY UNIT	N.D.S.	NATIONAL DESIGN
		N.D.S.	
CP.	COMPLETE PENETRATION		SPECIFICATION
CL	CENTERLINE	N.LC.	NOT IN CONTRACT
CLO.	CEILING	N.S.	NEAR SIDE
CLR.	CLEAR	N.T.S.	NOT TO SCALE
CONC.	CONCRETE		
CONN.	CONNECTION	O.C.	ON GENTER
CONST.	CONSTRUCTION	O.S.HA	OCCUPATIONAL SAFETY AND
CONT.	CONTINUOUS	cuation.	
COMI.	CONTROUDS		HEALTH ADMINISTRATION
		0.W.J.	OPEN WEB JOIST
CONTR.	CONTRACTOR	OPNG.	OPENING
		OPP.	OPPOBITE
DBA	DEFORMED BAR ANCHOR		
DL.	DEAD LOAD	P.C.I.	PRECAST/PRESTRESSED
			CONCRETE INSTITUTE
DET.	DETAIL	P.P.	PARTIAL PENETRATION
	DIAMETER	P.S.I.	POUNDS PER SQUARE INCH
DIAG.	DIAGONAL		FOUNDO FER SQUARE INVIT
DIM.		P.S.F.	POUNDS PER SQUARE FOOT
	DIMENSION	P.C.F.	POUNDS PER CUBIC FOOT
DN.	DOWN	PL	PLATE
DWGS.	DRAWINGS	PLBG.	PLUMBING
E.F.	EACH FACE	OTY.	QUANTITY
EJ.	EXPANSION JOINT		
E.W.	EACHWAY	R or RAD,	DADD IS
EA.	EACH		REFERENCE
EL.	ELEVATION	REF. REINF.	REINFORCING
	ELEVAININ		
ELEC.	E.ECTRICAL	RECTO.	REQUIRED
EQ.	EQUAL	FUEV.	REVISION
EXT.	EXTERIOR	RRE	RAKER RHODES ENGINEERING
F.S.	FARSIDE	SD.I.	STEEL DECK INSTITUTE
FIN.	FINISH	5	STEEL JOIST INSTITUTE
FM	FOUNDATION MISCELLANEOUS	S.D.G.	SLAB ON GRADE
FNDN.	FOUNDATION	SCHED.	SCHEDULE
	FOOT/FEET		SCREDULE
FT	FOOT/FEE/	SM	SIMILAR
		SPA.	SPACING/SPACES
GA_	GAGE	SPECS.	SPECIFICATIONS
GALV.	GALVANIZED	STD.	STANDARD
GC.	GENERAL CONTRACTOR	STRUC.	STRUCTURAL
-			
HP.	HIGH POINT	T/	TOP OF
HORIZ.	HORIZONTAL	ŤΥΡ.	TYPICAL
HT.	HEIGHT	U.N.O.	
mu.	ngan)	U.NUU.	UNLESS NOTED OTHERWISE
I.B.C.	INTERNATIONAL BUILDING CODE	VERT.	VERTICAL
IN.	INCHINCHES	VJ.F.	VERIFY IN FIELD (FIELD VERIFY)
INT.	INTERIOR		
		wi	WITH
Kork	KIP	wia	WITHOUT
		W.F.	WIDE FLANGE
		W.P.	WORKPOINT
		W.W.F.	WELDED WIRE FABRIC





(1/2" = 1'-0"



NOTE: SEE FON, PLAN FOR SLAB THICKNESS REINFORCING & JOINT LOCATIONS

WWF, SEE PLAN

SLAB ON GRADE, SEE PLAN

18" MIN. COMPACTED SUBBASE -SEE GEDTECHNICAL REPORT

1 1/2" = 1'-0"

### CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816,531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTURE 2100 CENTRAL STREET, SUITE 31 KANSAS CITY, MISSOURI 64108 PH: 816,474,1397

CIVIL ENGINEER WILSON & COMPANY 800 E 101ST TER, SUITE 200 KANSAS CITY, MO 64131 PH: 818.701.3100 CERT, OF AUTHORITY #2003007599

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN STREET, SUITE 300 KANSAS CITY, MO 64108 PH: 816.663.8718

STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. 8234 ROBINSON ST. OVERLAND PARK, KANSAS 68204 PH: 913,214.2169





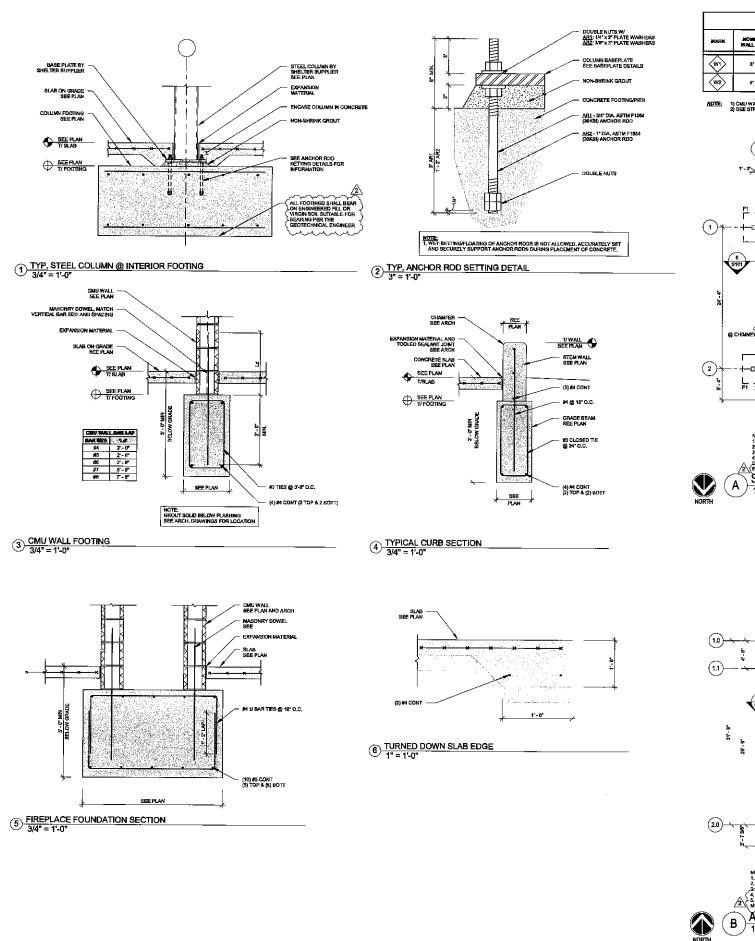


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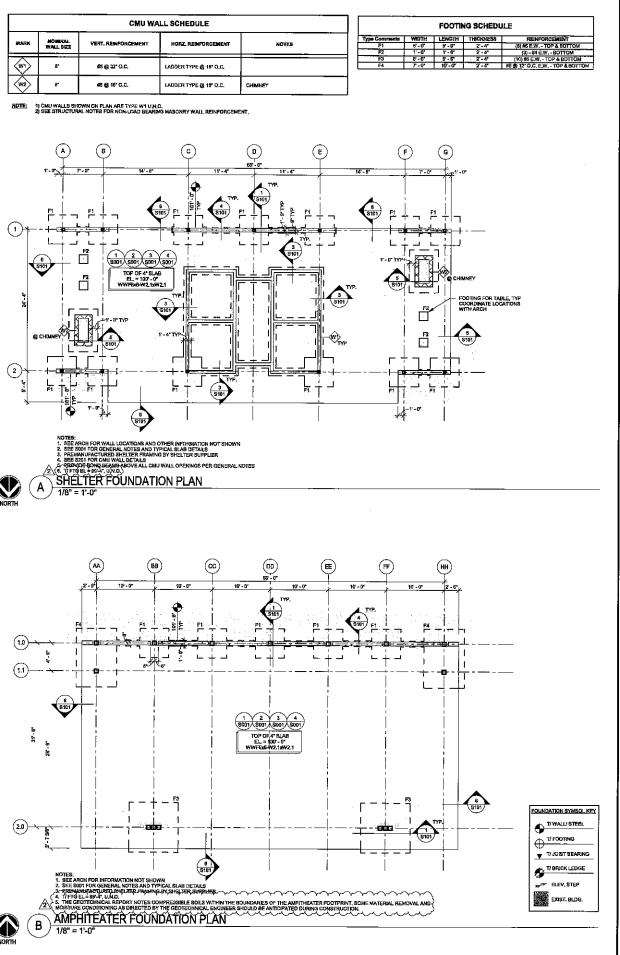
STRUCTURAL NOTES

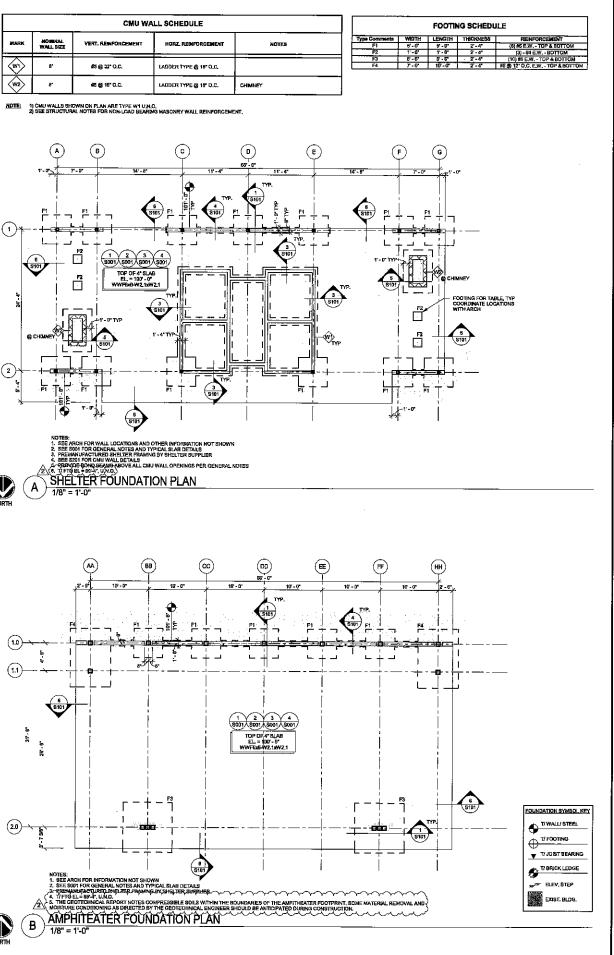
CONFLUENCE PROJECT NO: 16081KG

S001



		CMU W/	ALL SCHEDULE		1
WARK	NOMINAL WALL SIZE	VERT. REINFORCEMENT	HORZ. REINFORCEMENT	NÔTES	Type Com
<b>W1</b>	B*	#5 @ 12" O.C.	LADDER TYPE @ 18" O.C.		F3 F4
	8"	45 👩 16° D.C.	LADDER TYPE @ 15" D.C.	CHIMNEY	1





LANDSCAPE ARCHITECT COMPANY, INC. 415 DELAWARE, SUITE 400 KANSAS CITY, MISSDURI 64105 PH: 816,531.7227 FAX: 816,531.7229

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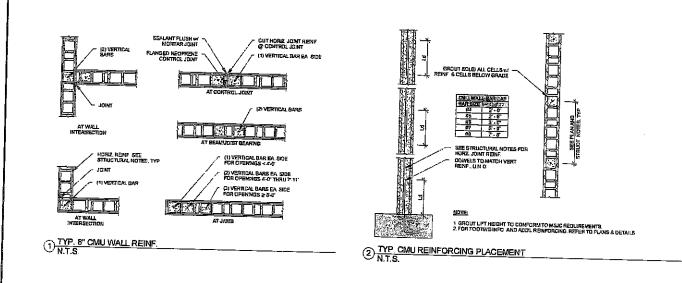






FOUNDATION PLANS
AND DETAILS

CONFLUENCE PROJECT NO: 16081KC S101



042018 12-0 00 Pbb) *standsener106, PropostsXXXX Malts RRE FilestRRERemone Projects/FRE 17072 Hork Ruge Park15 RD Olaringe RRE 17077 Hork Prige Safe Center Rrf 17 nd

### CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE COMPANY, INC. 415 DELAWARE SUITE 400 KANSAS CITY, MISSOURI 84105 PH: 816531.7227 FAX: 616.531.7229

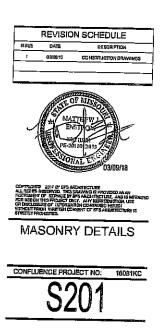
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STRUCTURAL ENGINEER STAND STRUCTURAL ENGINEERING INC. B234 ROBINSON ST. OVERLAND PARK, KANSAS 66204 PH: 913214.2169

> HAWK RIDGE PARK CITY OF RAYMORE PARKS AND RECREATION RAYMORE, MISSOURI



PLUMBING SYMBOLS	
THIS IS A MASTER LEGEND AND NOT ALL SYMBOLS OR ABBREVIATIONS	MAY BE USED. V2.00
	DOMESTIC HOT WATER (HW)
1 PLINEING EQUIPALENT DESIGNATION. (CONTRACTOR FURNISHED AND INSTALLED). REFER TO PLUMBING FUTURE OR EQUIPMENT	SUL PIPING - ABOVE FLOOR (S)
SCHEDULES	SOIL PIPING - BELOW FLOOR (5)
1 EDURMENT DESIGNATION (OWNER FURNISHED, CONTRACTOR	WASTE PIPING - ABOVE FLOOR (W) WASTE PIPING - BELOW FLOOR (W)
INSTALLED)	WASTE PIPING - BELOW FLEUR (W)
AND INSTALLED LINEED LITER CONTRACTOR FURNISHED	
AND INSTALLED UNLESS NOTED OTHERWISC)	CONDENSATE PUMP DISCHARGE (PD)
CONNECTION POINT OF NEW WORK TO EXISTING	
DETAIL REFERENCE UPPER NUMBER INDICATES DETAIL NUMBER LOWER NUMBER INDICATES SHEET NUMBER	E G FLOOR DRAW (FD), SIZE & TYPE
	D ROOF DRAW (RD), SIZE & TYPE
	BALL VALVE
STANDARD MOUNTING HEIGHTS	CONTROL VALVE
REFER TO THE ARCHITECTURAL DRAWINGS FOR PLIMBING FATURE MOUNTING HEIGHTS. UND, INSTALL PLIMBING FATURES WITH THE MOUNTING HEIGHTS AS LISTED BELOW WITH FINAL AFFRANK, BY THE ARCHITET.	SHUTOFF VALVE
	CHECK VALVE
LAVATORY OR SINK STANDARD HEIGHT 31° FLOOR TO RIM ADA ACCESSIBLE 34° FLOOR TO RIM	BALANCING VALVE WITH PRESSURE PORTS
ADA ACCESSIBLE 34" FLOOR TO RIU	WATER NETER
URINAL STANDARD HEIGHT	STRAINER
ADA ACCESSBLE 24" FLOOR TO RIM 17" FLOOR TO RIM	RELIEF/SAFETY VALVE
WATER IN DEET	
STANDARD HEIGHT 15" FLOOR TO FUM ADA ACCESSIBLE 17" TO 19 FLOOR TO TOP OF SEAT	
	BACKALOW PREVENTER
WATER COOLER OR DRINKING FOUNTAIN STANDARD HEIGHT 41° FLOOR TO SPOUT ADA ACCESSIBLE 36° FLOOR TO SPOUT	PRESSURE GAUGE
HOSE BIES 42" FLOOR TO CENTERLINE	
NON FREEZE WALL HYDRANTS 35" AFT TO GENTERLINE	FLANGE CONNECTION
	HOSE EUBB (HB)
UNLESS NOTED OTHERWISE, MOUNTING HEIGHTS LISTED ARE ABOVE FINISHED FLOOR (AFF) OR ABOVE FINISHED GRADE (AFG).	NONFREEZE WALL HYDRANT (NW)
ABBREVIATIONS	CLEANDUT
ADA AMERICANS WITH DISABILITIES MAX HAXIMUM	CAP
AFE ABOVE EINISHED D.COM MEH 1000 BTU PER HOUR	
APU AR HANDING INF	· · · · · · · · · · · · · · · · · · ·
AFU AFFRANDUNG UNUL MAN MINIMA AF ALTENDUNG UNI N, AND	ELE EXTERIOR CLEANDUT (EDD)
AP ACCESS PARS. THE ACCENT ACC	- ELBOW OP
BUS BUTTOM OF STRUCTURE PRV PRESSURE REDUCING VALVE	
DED EXTERNO OF SINUCIDIC. PRV PRESSURE REDUCING VALVE ETU EXTIREST HERMAN UNIT. PVC POLYMINI. CHLOROIS CPVC CHLORINATED FOLYMINI. RD ROOF DRAIN CHLORINE REP. NEWLYT.	TEE DOWN
CU COPPER ROLL PROCTOLINATION	
DN DOWN SF SQUARE FEET	
DID DIDUTOLE FIATURE UNIT SS STAINTESS STEEL CANITADY	TEE UP WITH SHUT-OFF VALVE (SOV)
DS DOWNSPOUT SEVER, SOL STACK EMS ENERGY MANAGEMENT SYSTEM ENS ENERGY MANAGEMENT SYSTEM ETA TO FLOOR BELOW ETA TO FLOOR BELOW	TEE DOWN WITH SKUT-OFF VALVE (SOV)
BURNESPOUT     B	
LINE DALING UNELANY FO PLOOD DAWN COLLER TTP TYPICAL FFA FRON FLOOR ABOVE UL UNDERWATERS LABORATORES, FFA FRON FLOOR ABOVE UL UNDERWATERS LABORATORES, FFF FRON FLOOR BLOW UL FF PINSHED FLOOR ULPS UNINTERNATIONE POWER FF FLOOR ULPS UNINTERNATIONE POWER FF FLOOR ULPS UNINTERNATIONE POWER	WITH PDI SIZES, (A, B, C, D, & E)
FTE FROM FLOOR BELOW UND UNLESS NOTED OTHERWISE FF PINSHED FLOOR UPS UNINTERSHED FLOOR FLOW UNC FLOW UNC DA FULL LOAD AMPS	
TA FULL WAD AMPS 1 to transfer	
AD DEAD LINE PRAISE YOU WARDED FREQUENCY DRIVE	
E INVERT ELEVATION VIR VENT THROUGH ROOF	
HER UNKTION BUX WC WATER COLUMN WW KILOWATT WSFU WASTE STACK WSFU WATER SUPPLY FATURE UNIT	

. .

AND ST
4. INSTALL THE RE MEET AJ LANDLO
5. PLANS A REQUIR
6. VERUFY BEFORE
7. REFERT HEIGHT
8. DO NOT ROUTIN
9. Rystall Possiel Ceiling Avoid C
10. VALVEE
11. РІРІМС (1 РІРІМС, 1 ПОНТТО
12. INSTALL BUILOING
13. COORDIN
14. COORDIN FOOTING FOOTING SLEEVEI STRUCT CONCRE
15. CLEAN F/ BUILDING
16. PROVIDE
17. COORDIN INSTALL (
18. VERIFY W RUST INH COORDIN
19. COORDIN MINIMUM FROM ALL
20, INSULATE 2' BATT IN
21. PROVIDE LARGER, AND VENT
22. PROVIDE TO CAST ( DIVISION 2 FIPING AN

#### GENERAL NOTES:

1. FROVIDE A CONSTRUCTION RECORD SET OF "AS-BUILT" DOCUMENTS TO THE ARCHITELT AND OWNER'S CONSTRUCTION MANAGER REFLECTING ANY VARIANCES OF INSTALLED PIPINS LOCATIONS OR EQUIPMENT CONTRARY TO THE CONSTRUCTION DOCUMENTS, REFER TO SPECIFICATIONS.

TO THE CONSTRUCTION DUBLINENTS, TOPERTS, TO SPECIFICATIONS, 2. DRAWING ARE DAGRAMMATIC ONLY AND DEPRESENT THE GENERAL SOLDE OF THE WORK, REVIEW THE GENERAL NOTES, SPECIFICATIONS AND PLANS TOP ADDITIONAL REPUIRE/REVEALS THAT MAY NOT ESSENT SCALE CALLED OUT IN THIS FORTION OF THE CONSTRUCTION DOCUMENTS CALLED OUT IN THIS FORTION OF THE CONSTRUCTION DOCUMENTS NOTIFY THE RECORDER THAN OWNERS CONSTRUCTION DOCUMENTS OF ANY CONFLICTS OR DISCREPANDIES PRIOR TO SUBMISSION OF BIL

PROVIDE TO THE ARCHITECT AND OWNER'S CONSTRUCTION MANAGER A COPY OF INSPECTION REPORTS AND APPROVAL CENTRICATES FROM LOCAL AND STATE INSPECTIONS, REFER TO SPECIFICATIONS.

INTERNISTEDITIONS, CHE LEN LE VIEL CONSTITUTED CODES AND LEMINSMENTS OF AUTHORITIES HANNS, JURSDICTION AND ALSO ALL REQUIREMENTS OF THE HANDLOR. OUTFINE ACOPY OF THE LORD'S REQUIREMENTS AND REVIEW PRIOR TO SUBMITTING BID.

AND SPECIFICATIONS GOVERN WHERE THEY EXCEED CODE REMENTS

Y LOCATION AND DEPTH OF UTILITIES AT POINTS OF CONNECTION IE START OF PIPING INSTALLATION.

TO ARCHITECTURAL FLANS FOR EXACT LOCATION AND MOUNTING TS OF FLUMEING FIXTURES. I SCALE FLOOR FLANS FOR EXACT KORIZONTAL LOCATION OF PIPE

L CONCEALED FIPING TIGHT TO THE STRUCTURE AND AS HIGHAS SLE, INSTALL EXPOSED FIPING TIGHT TO THE STRUCTURE WALL OR A AND AS HIGH AS POSSIBLE, COORDINATE WITH OTHER TRADES TO CONFLICTS.

S THALL BE LINE SIZE UNLESS OTHERWISE NOTED.

IN FINISHED AREAS EMAIL BE ROUTED CONCEALED; EXPOSED WHERE NECESSARY, SHALL BE ROUTED AS HIGH AS POSSIBLE AND FO WALLS.

L NO PLASTIC PIPE OF ANY KIND ABOVE SLAB INSIDE OR UNDER THE 16. INSTALL NO PLASTIC PIPE IN THE CELLING RETURN AIR PLENUM.

NATE ALL WORK WITH OTHER TRADES AND CONTRACTORS. NATE FING INSTALLATION WITH STRUCTURAL GRADE BEAMS. 1939. COLUMN FIERS, BTC. SLEEVE FINITO THROUGH GRADE BEAMS. 1943. COLUMN FIERS, BTC. SLEEVE FINITO THROUGH GRADE BEAMS. 1943. COLUMNER RESULTED AND AS MONTED ON FLANS. COORDINATE EINSTALLATIONS WITH THE ARCHITECT, STRUCTURAL BEADRE TIDRAL CONTRACTOR AND GENERAL CONTRACTOR BEFORE URE IS INSTALLED.

AUCET AERATORS AND PIPE STRAINERS FRIDE TO TURNING G OVER TO THE OWNER.

E TRAP PRIMERS WHERE REQUIRED BY LOCAL AUTHORITIES. NATE PIPE ROLITING AWAY FROM ELECTRICAL PANELS. DO NOT PIPING OVER ELECTRICAL PANELS.

WITH THE ARCHITECT THAT ALL DRAGED WATER PLPING USING HIGHTOR PAINT EMALL BE PAINTED. PAINT AND COLOR SHALL BE NATED WITH THE ARCHITECT AND / OR OWNER.

NATE ALL ROOF PENETRATIONS WITH OTHER TRADES. MAINTAIN 10° II CLEARANCE FROM ALL AR INTAKES MAINTAIN 2° CLEARANCE LL OTHER EQUIPMENT.

E PPING ROUTED IN EXTERIOR BUILDING WALLS WITH MINIMUM NSULATION TO PREVENT FREEZING,

THEAVY-DUTY" NO-HUB COUPLINGS ON SANITARY PIPING 3" AND SEE DIVISION 22 SPECIFICATION SECTION "SANITARY DRAINAGE IT AND PIPING SPECIALITES" FOR MORE (MPORMATIC).

TRANSITION ADAPTER COUPLINGS FOR CONNECTION OF PUC DWV IRON SANITARY, WASTE AND VENT PIPE AT SLAB ON GRADE. SEE 22 SPECIFICATION SECTION 'SANITARY DRAINAGE AND VENT ND SPECIALITIES' FOR MORE INFORMATION.

23. WATER HAMMER ARRESTORS SHALL BE SIZE "A" UNLESS NOTED OTHERWISE

### CONFLUENCE

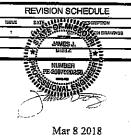
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MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MO 64108 PH: 816.6839718

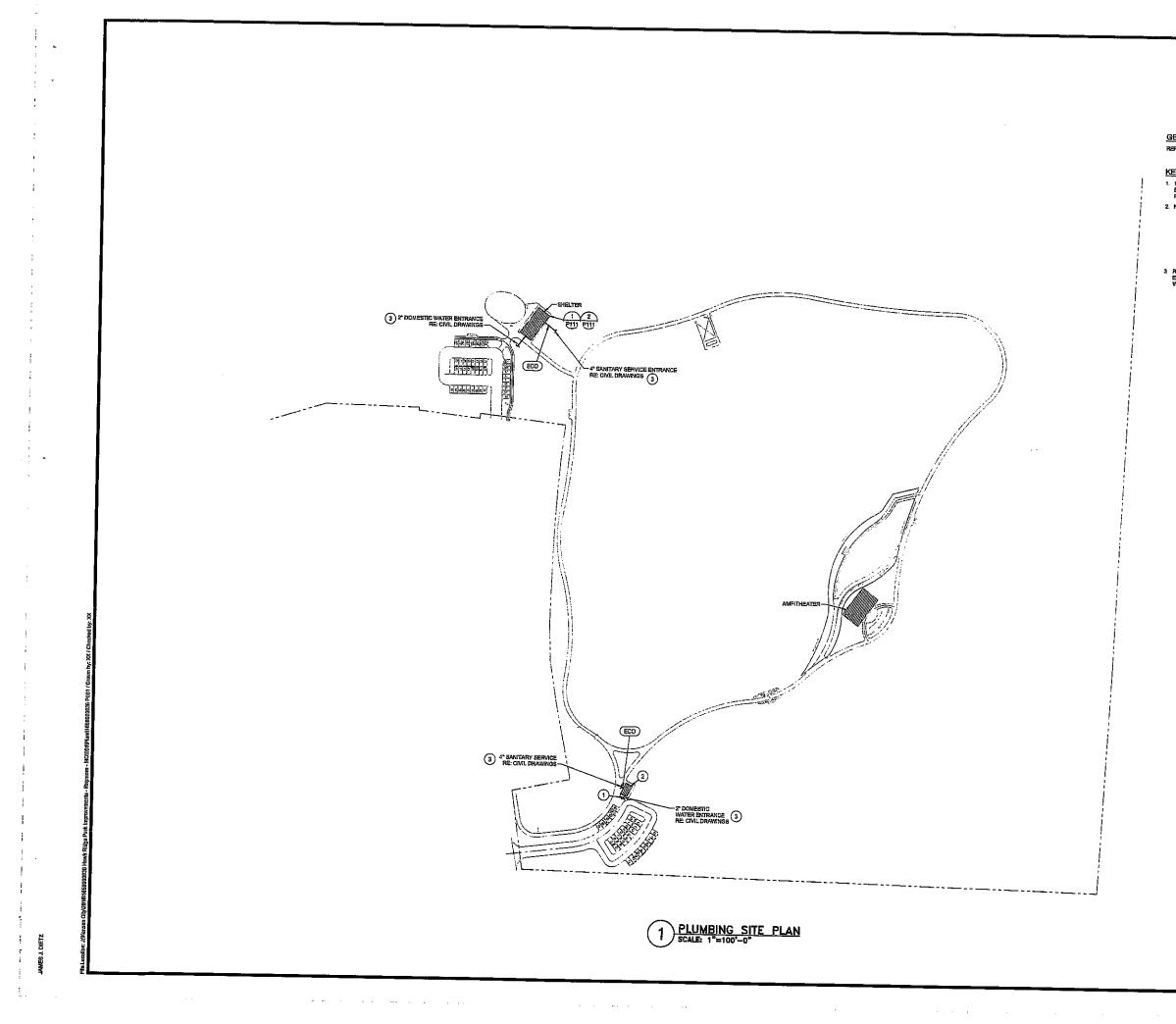




ААЛЕЗ 1. DIETZ UCENSE # РЕ-2007.020258 СОРТИЗИИ Ф 2017 ИГО ОПИСИСТВИ И КОТА 525/02010. ПАЗАЛИЗИ В ПОТОВО И КОТА 525/02010. ПАЗАЛИЗИ В ПОТОВО И КОТА 525/02010. ПОТОВОЛИТЬ И КОТОВО ОПИСИТАТИ И ПОТОВОЛИТЬ И ПОТОВОЛИТЬ ОПИСИТАТИ И ПОТОВОЛИТЬ И ПОТОВОЛИТЬ ПОТОВОЛИТЬ И ПОТОВИТЬ И ПОТОВИТИ И ПОТОВИТИ И ПОТОВИТИ И ПОТОВИТИ И ПОТОВИТИ И ПОТОВИТИ И ПОТОВИИ И ПОТОВИИ И ПОТОВИТИ И ПОТОВИИ И ПОТОВИ И ПОТОВИ И ПОТОВИ И ПОТОВИ И ПОТОВИ И ПОТОВИИ И ПОТОВИИ И ПОТОВИИ И ПОТОВИИ И

### PLUMBING LEGENDS

CONFLUENCE PROJECT NO: 16081KC P000



GENERAL NOTES:

REPER TO SHEET FOOD FOR GENERAL NOTES.

#### KEY NOTES:

1. WATER INSTER IN WATER METER PIT, REFER TO CIVIL PLANS FOR EXACT INSTALLATION LOCATION ON SITE. VERIFY AND COORDINATE FINAL LOCATION WITH THE ARCHITECT.

2. MODULAR RESTROOM SCOPE OF WORK:

MODULAR NOT RESTROYED BY OTHER: FLUMBING CONTRACTOR MODULAR UNIT IS PROVIDED BY OTHERS: FLUMBING CONTRACTOR SHALL PROVIDE ONE 4' SANITARY STUB AND ONE 2' DOMESTIC WATER STUB PER MANUFACTURER RECOMMENDATIONS. FIELD VERIFY AND COORDINATE EXACT PLUMBING PPE STUB LOCATIONS AND DIMENSIONS, PIPER BLACK PLUMBING PPE STUB LOCATIONS AND DIMENSIONS, PIPER BLACK PLUMBING PPE STUB LOCATIONS CLEANOUT LOCATIONS WITH ALL DISCIPLINES. NOTIFY ARCHITECT AND PLUMBING EXIGNEER OF ANY OTHER COORDINATION ITEMS WITHIN THE FLUMBING ENGINEERS SOCHE OF WORK

3 REFER TO CIVIL PLANS FOR EXACT LOCATION OF GERVICE EXIT / ENTRANCE. COORDINATE LOCATION OF SERVICE EXIT / ENTRANCE WITH ALL DISCIPLINES.

### CONFLUENCE

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MEP ENGINEER HENDERSON **ENGINEERING** 1801 MAIN, STE 300 KANSAS CITY, MO 64108 FH: 816.6638718

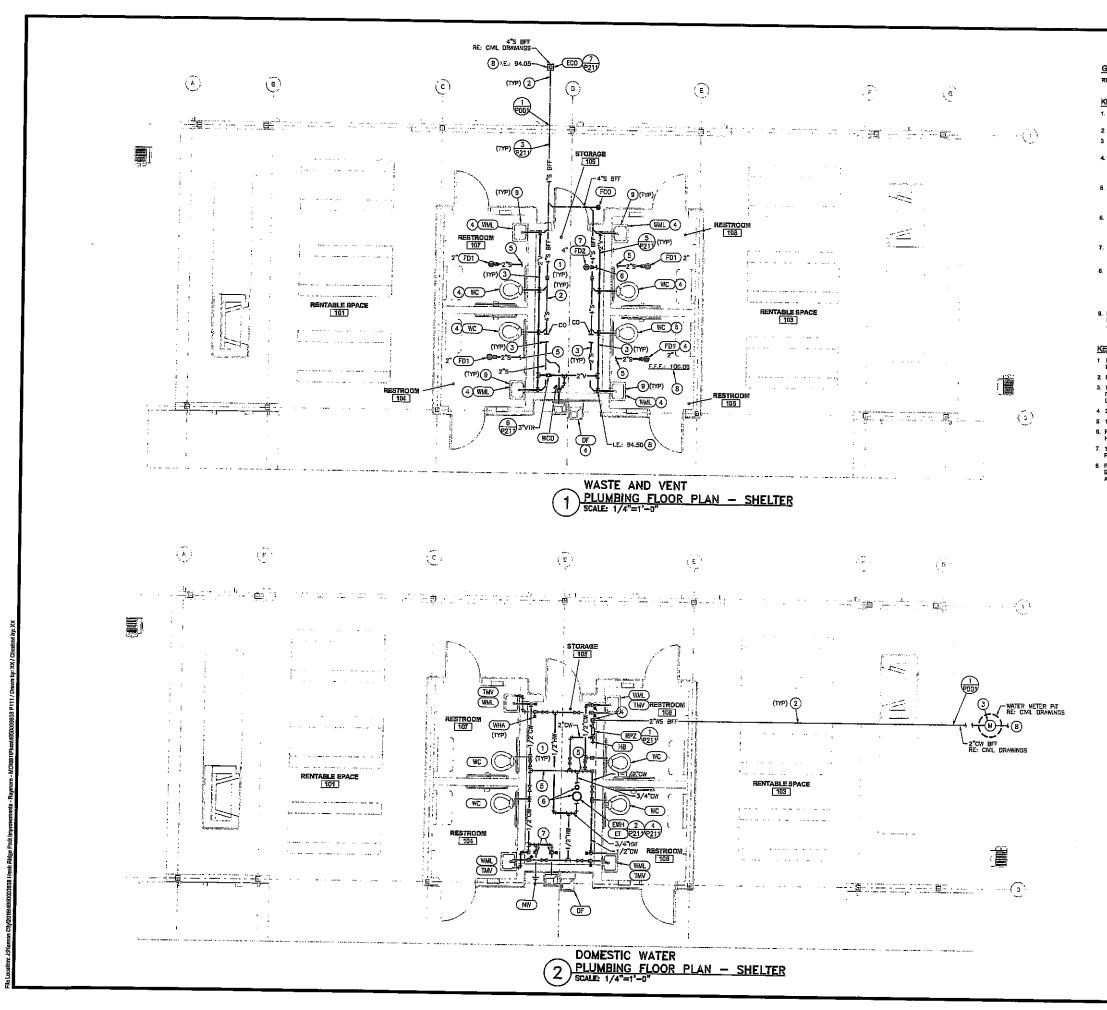




SITE PLAN

CONFLUENCE PROJECT NO: 16081KC

P001



and the second second

### GENERAL PLUMBING NOTES:

REFER TO SHEET F000 FOR GENERAL NOTES.

#### KEY NOTES WASTE AND VENT:

 DO NOT INSTALL ANY PLUMBING PIPING OVER ELECTRICAL PANELS OR EQUIPMENT.

2 PIPING SHOWN DASHED SHALL BE INSTALLED BELOW GRADE

 REFER TO WASTE AND VENT RISER DIAGRAMS FOR PIPING DETAILS AND CONTINUATION IN CHASE.
 PROVIDE PLUMBING FIXTURE AT CHASE WALL, BREED TO BUILDING

4. PROVIDE PLUMEING FORTURE AT CHASE WALL REFER TO FLUMEING Riser Diagrams for Piping Detrus and Continuation in Chase Typical for all plumeing fortures, this type, shown on this floor plan.

5 PROVIDE R.OOR URAIN AND ROUTE 2' SANITARY TO CHASE. REFER TO WASTE AND VENT RISER DIAGRAMS FOR PIPME DETAILS AND COMMUNICATION IN CHASE. TYPICAL FOR ALL RLOOR DRAINS, THIS TYPE, SHOWN ON THIS FLOOR PLAN.

6. FROMDE FLOOR DRAIN AND ROUTE 4" SANITARY TO CHASE. REFER TO WASTE AND VENI RISER DIAGRAMS FOR PIPING DETAILS AND COMINUATION IN CHASE. TYPICAL FOR ALL FLOOR DRAINS, THIS TYPE, SHOWN ON THIS FLOOR PLAN.

7. PROVIDE ALGOR URAIN FOR DOMESTIC WATER GERVICE ENTRANCE. INSTALL RELIEF VALVE UNDER REDUCED PRESSURE ZONE BACKFLOW ASSEMBLY.

REVENTION OR ARCHITECTURAL PLANS FOR EXACT FINISHED FLOOR ELEVATION. THE FINISHED FLOOR E EVATION SHOWN 10140 IS INTENDED FOR FINISH WORTH ELEVATION RESPECTIVE ONLY AND SHALL NOT BE CONSTRUCTION AS A FINAL PIPING INVERT ELEVATIONS FOR CONSTRUCTION. COURDINATE FINAL INVERT ELEVATIONS WITH THE ARCHITECT AND CAVL.

PLUMBING FORTURE CONNECTION SIZES NOTE SHOWN FOR GLARITY. REFER TO FLUMBING FORTURE CONNECTION SCHEDULE FOR CONNECTION DETRILS AND INFORMATION.

#### KEY NOTES DOMESTIC WATER:

1 DO NOT INSTALL ANY PLUMBING PIPING OVER ELECTRICAL PANELS OR EQUIPMENT.

2 PIFING SHOWN OASHED SHALL BE INSTALLED BELOW GRADE. 3. WATER METER IN WATER METER PIT, REFER TO CIVIL FLANS FOR EXACT INSTALLATION LOCATION ON SITE. VERIFY AND COORDINATE FINAL LOCATION WITH THE ARCHITECT.

4 Z' COLD WATER FROM BELOW FINISHED FLOOR 5. 1-1/2 COLD WATER

8. REFER TO WATER HEATER AND EXPANSION TANK DETAILS FOR WATER HEATING BYSTEM INSTALLATION.

 1/2" COLD WATER COWN, PROVIDE SHUT OFF VALVE IN COLD WATER RISER, DROP AT 5'-0" AFF.
 REFER TO COM DI ANNE FOR EVALUATION OF REPURCE FOR 1

 REFER TO CIVIL PLANS FOR EXACT LOCATION OF SERVICE BUT / ENTRANCE. COORDINATE LOCATION OF SERVICE EXIT / ENTRANCE WITH ALL DISCIPLINES.

### CONFLUENCE

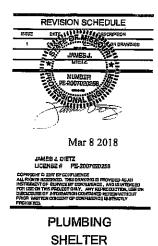
LANDSCAPE ARCHITECT CONFLUENCE 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 FH: 916.631.7227 FAX: 819.531.7229

ARCHITECT SFS ARCHITECTS 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 818.541.2288

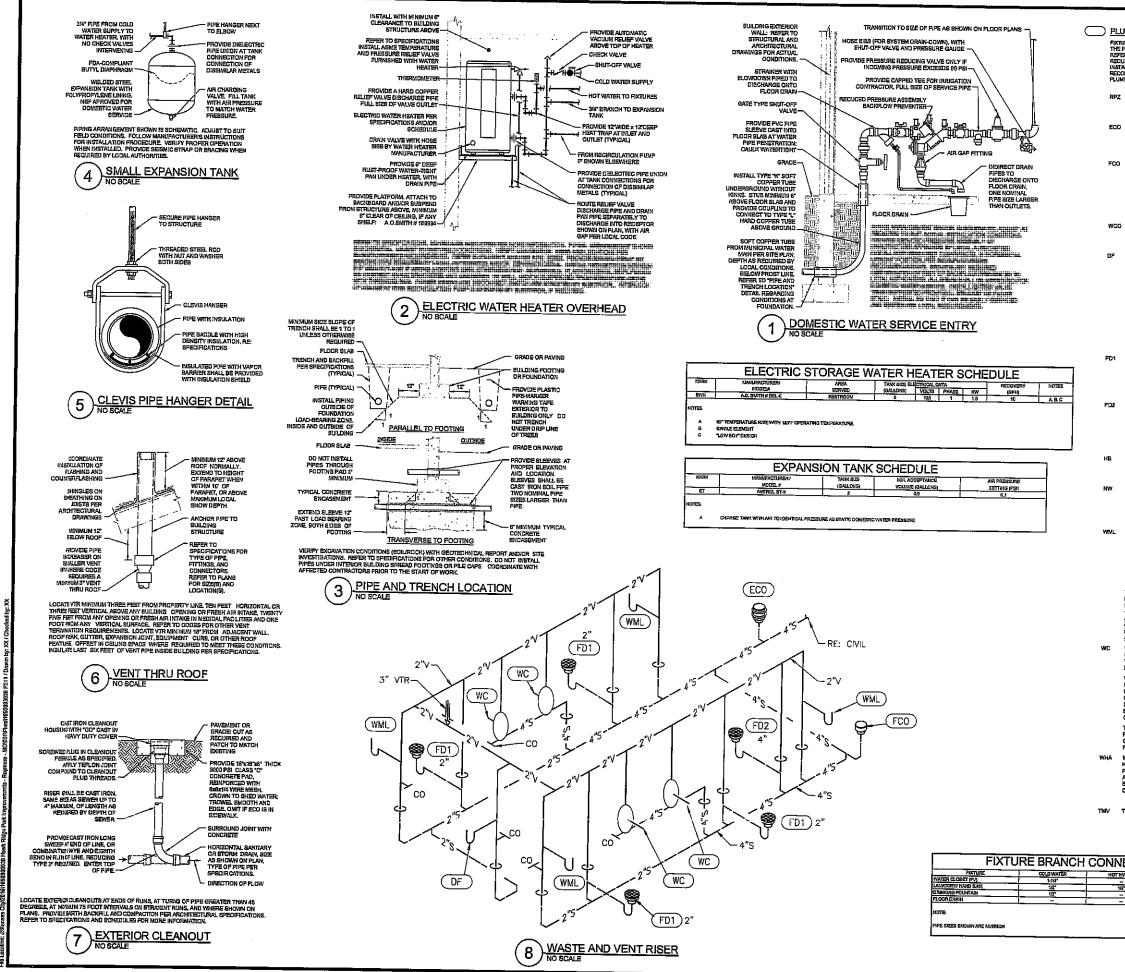
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MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CTTY, MO 84108 PH: 816,6538718





CONFLUENCE PROJECT NO: 16081KC



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#### PLUMBING FORTURE SCHEDULE:

FLEWING FOR THATLE OF THE APPRICIP ENDALERT ARE PROVIDED BY FRUTHER IN THIS CHEMICLE OF THE APPRICIP ENDALERT ARE PROVIDED BY THE FLUXENING CONTRACTOR, SUBMIT SHOP DRAWNES ON EACH OF THESE THESE INFERT TO SPECIFICATIONS FOR FUTURER FROM FORMATION AND INSTALLATION REQUIREMENTS. VERIFY ROUGH-IN REQUIREMENTS WITH INVUSATIONER REQUIREMENTS. WERTY ROUGH-IN REQUIREMENTS WITH INVUSATIONER REQUIREMENTS. REFER TO THE ARCHITECTURAL DRAWINGS FOR THE FLUXENING RAUTE MOLTING HEARTS.

- PZ REDUCED FRESSURE ZONE BACKFLOW PREVENTER: WATTS # 00507-5, METING ASSE 1019, QAST BROXZE BODY, QUARTER TURN TEST COCKS, QUARTER TURN BALL VALVES, SRONZE STRAINER, AND # 502AQ AR GAP FITTING.
- CO EXTENIOR CLEANOLT: ALY R. SMITH # 4281L GERIES DUCO CAST IRON DOUBLE FLANGED XOUSING WITH HEAVY DUTY SECURED SCORATED CAST RON COTTEN WITH WITHING DEVICE AND CLEANOUT BIDDY WITH ABS PLASTIC FLUG WITH GASKET SELL AND PUBLICAN JOINT. RESET TO SECONDATIONS FOR RESTALIATION.
- PUSK-ON JOINT, REFER TO SPECIFICATIONS FOR INSTALLATION 20 Section 2010 Se
- WCO WALL CLEANOLT: JAY R. EMITH # 4505, CAST IRON CLEANOLT THE COUNTER SUNK PLUS, STAINLESS STEEL ROUND COVER AND SCREW, AND IRON PLUS WITH GASKET SEAL, REFER TO SPECIFICATIONS FOR INSTALLATION.
- GESTIGATION OF THE REST OF THE REAL OF THE
- PROVIDE ELKAY # MP2D SURFACE MOUNTING PLATE (FOR MASDNRY WALLS ONLY) OR ELKAY # ML100 IN WALL, FLODR SUPPORT LEGS (FOR STUD WALLS ONLY).
- TRIM-MOUTENER LIZERSC LEAD FREE BRASS COMPRESSION ANGLE STOP VALVE WITH RISER AND ESCUTCHEON, MGUIRE & B8972CF 1441 72 AUGG CAST CHROME PLATED BRASS ADJUSTABLE PATROP AND WASTE ARM WITH CLEANOUT PLUG AND ESCUTCHEON.
- FLOOR DRAIN: JAY R SMITH # 2005L (A), CAST IRON BODY AND CLAMPING COLLAR, ADJUSTABLE & ROUND NICKEL BRONZE STRAINER, USE PUSH-ON JOINT OF OUTLET SIZE AS SHOWN ON FLANS.
- TRAP BEAL: PROVIDE TRAP BEAL PER SPECIFICATIONS FOR ACTUAL FLOOR DRAIN MODEL AND SIZE
- EQUIPMENT FLOOR DRAIN: JAY R. SMITH #2131L (8), 5'DEEP GAST IRON BODY, 12' ROUND, LOOSE, MEDIUMDUTY, CAST IRON GRATE. SEDIMENT BUKAT, BOTTON OUTLET, SEPAGE PAN, AND MEMERANE FLABHING CLAVP. USE FUEN-ON JOINT OF OUTLET SIZE AS STOWN ON FLANS.
- TRAP GEAL: PROVIDE TRAP SEAL PER SPECIFICATIONS FOR ACTUAL FLOOR DRAIN MODEL AND SIZE
- HOSE BIBS: FRIER PRODUCTS # C-153NP.75, ROUGH CHROME PLATED BRASS 347 FEMALE FPI INLET, 347 TIMEADED HOSE CONNECTION, METAL WHEEL HANDLE, AND ASSE 1011 INTEGRAL VACUUM BREAKER.
- NON-FREEZE WALL HYDRANT: FRIER PRODUCTS & C-B34NBX1, SATIN NICKEL P. ATED BRASS 1* MALE INLET BY 3%* FEMALE INLET, 34* THREADE NOSE CONNECTION, LOOSE KEY HANDLE, HYDRANT LENGTH AS REQUIRED FOR INSTALLED WALL THICKNESS, ADJUSTAELE WALL CLAYR BRASS BOY WITH GATIN NICKEL PLATED FINISH AND INTEGRAL ASSE 1052 COURLE CARECY VACUUM BREAKER.
- WALL-MDUNTED LAVATORY (ADA ACCESSIBLE): AMERICAN STANDARO & GISLO12 "LUCENNE" 20-12" X 18-14" RECTANGULAR WALL MOUNTED WITHE WITHENDUS CHINA FORTURE WITH FAUCET LEDGE AND FRONT OVERPLOW.
- FAUCET-CHICAGO FAUCET # 502-VE2805A80KOP 4* CENTERGET. VANDAL RESISTANT, LEAD FREE FAUCET WITH # 380 LEVER HANDLES, CERAMIC QUARTER TURN CARTRIDGES AND # E2805 0.5 GPM AERATOR.
- WALL-MOUNTED WATER CLOSET (ADA ACCESSIBLE): AMERICAN STANDARD 8 2257, 101 "AFWALL MILLENNIUM FLOWIGE" WHITE VITREDUS CHINA FIXTURE WITH ELONGATED UNIVERSAL BOYL AND DIRECT-FED SIFHON JET ACTION.
- VALVE SIGAN SUGAN SI IN A GALLON PER FLUSH, EXPOSED CHROMS-PLATED DIAPHRAGM TYPE FLUSH VALVE WITH CHLORAWINE RESISTANT DIAPHRAGM AND PROTESTED DORIFICE. GSCILLATING ADA COMPUNAT HANDLE, ESCUTCHEON, INTEGRAL SCHEWINGHEM STOP WITH VAROAL RESISTANT CAP, VACUUM BREAKGR, AND GWEAT ADAPTER ICT. INSTALL FLUSH VALVE HANDLE ON THE WIDE SUG OF THE STALL
- TRIM- CHURCH # 95008SCT WHITE OPEN-FRONT CONTOURED, SOLID PLASTIC, HEAVY DUTY, SEAT LEBB COVER WITH STELF-SUSTAINING CHECK HINGES AND BTAINLESS STEEL BOLTS. PROVIDE SUITABLE FIXTURE CARNIER
- WATER HAMMER ARRESTER: PRECISION PLUMBING PRODUCTS, MARD DRAWN COFFER BODY WITH WROUGHT COPPER FITTINGS, PISTON TYFE WITH LIBRICATED EPON TO RING SEALS, MEETING ASSE 1010 ON PDI WH 201, FROMDE PID EUSES 'A'T PROUGHT TA SHCIMM ON PLANS. PROVIDE SIZE 'A' UNLESS SHOWN OTHERWISE ON THE PLANS.
- TMV THERMOSTATIC MIXING VALVE, POWER LEG480 SERVES

ATER	WASTE	van
<u>.</u> T	4*	2
7	2	1-1/2"
-	2*	1-1/2
I	2"	ž

### CONFLUENCE

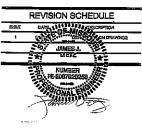
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Mar 8 2018



ELECTRICAL SYMBOLS				<u> </u>
THIS IS A MASTER LEGEND AND NOT ALL SYMBOLS OR ABBREVIATIONS ARE L STANDARD MOUNTING HEIGHTS			LIGHTING CONTROL. WIRING DEVICES & BOXES	
AUDINE APPLANCES (CENTERLINE) 64 ALFRUS 65 AURINE CAROL 65 AUR	AECHANICAL OR HRE PROTECTION FLAN CALLOUT	A LIGHT FIXTURE		ELECTRICAL ONE-LINE & RISER DIAGRAM
EXIT SIGNS (WALL MOUNTED)	D PULINBING PLAN NOTE CALLOUT		SWITCH LETTER DESIGNATIONS AS FOLLOWS:	SWITCH (RATING AS INDICATED))
FREALAR/(BEL (EXTERIOR) 120			2 ≈ TWO POLE 3 ≈ THREE-WAY 4 ≈ FOUR-WAY	DRAWDUT ORCUIT BREAKER (RATINGS AS IND(CATED)
FIRE ALAQUI CONTROL PANEL/UNIT (DISFLAY) BUT INTERCOM (AFA ONLY) 37 INTERCOM (AFA ONLY) 37	1 ELECTRICAL FLAN NOTE CALLOUT		将 D = DIAMER F = FAN SPEED CONTROL	THEFT SWITCH (RATING, POLEB AND FISE TYPE AS INDICA
			K = KEVED LV ≈ LØW VOLTAGE	2200
RECEPTACES         16           RECEPTACES(CARGES)         24           RECEPTACES(GARGES)         24           RECEPTACES(CARGES)         24	PLIABING BOUTWENT DESIGNATION, (CONTRACTOR FLERINSHED AND INSTALLED), REFER TO PLIAMENS FORTURE OR EQUIPMENT		05 = 0.05UPANCY 55US07 P = 5PSF THUST LIGHT V5 = VACANCY 55US03 WP = VV2ATHER PROCF	COMELIKATION FUEED SMITCH/STARIER AND STARTER SIZ
REACT ACCES IN EQUIPATENT ROOMS 44" REACTE INDICATING LIGHT (EQUIPATENT ROOMS) 42" REMOTE INDICATING LIGHT (FINISHED AREAS) CEILING	1 EQUIPMENT DESIGNATION (OWNER FURMSHED, CONTRACTOR		ALTOMATIC LOAD CONTROL RELAY	CONTRINCTION CIRCUIT EREAKERUSTARTER AND STARTER
SAFEITY SWITCHES (TOP OF DEVICE) STATUES (TOP OF DEVICE) SVITCHES TELEPHONE, DATA CUTTLETS SAME AS ADJACENT DEVICE, LIVO		NIGHT LIGHTEMERGENCY LIGHT FIXTURE WITHERGENCY BATTERY PACK OR CONNECTED TO EMERGENCY SQURCE		
TELEFICIAE TEMAINAL EDATO (BOTTOM) TELEFICIAE TEMAINAL EDATO (BOTTOM) VISIELE AFFUMICES (CENTERLINE) VISIELE AFFUMICES (CENTERLINE)	1	O LIGHT FORTURE VATH DUAL BALLASTB ORCUMED SEPARATELY (SHADING RIFLES BARRETERY LIGHT FORTURE)		
USE THE DEFAULT MOUNTING HEIGHTS SHOWN ABOVE UNLESS HOTED OTHERWISE IN THE SPECIFICATIONS OR ELSEWHERE MOUNTING HEIGHTS LISTED ARE ABOVE CINISHED				
FLOOR (AFF) OR ABOVE FINISHED GRADE (AFG) TO BOTTOM OF OUTLET BOX. ALL DEVICES SHALL BE INSTALLED IN COMPLIANCE WITH CURRENT ADA AND LOCAL REQUIREMENTS.		<u></u>	DOUBLE DUFLEX RECEPTAGLE - NEWA 6-20R, UNO	TRANSFORMER (TYPE AND RATINGS AS INDICATED)
ABBREVIATIONS			5-TECIAL RECEPTACLE - NEMA TYPE AS NOTED	。 アッパ 開始 SHIELDED TRANSFORMER (TYPE AND RATINGS AS INDICATE デイ・
AF ANTERS FRAME SIZE MCB MAIN CROUT BREAKER AFC ASOVEFINISTED CEILING MCC MOTOR CONTROL CENTER				ATT2
APP ABOVE FINISHED FLOOR MER MANUFACTURER APG ABOVE FINISHED GRADE MIN MINIMUM	CIRCUITING & WIRING	🕺 🕸 EXIT BIGN - CEILING / WALL MOUNTED, ARROWS AS INDICATED		AUTOMATIC TRANSFER SWITCH (RATINGS AS INDICATED)
ANI AUTORITY HAVING MLO MANULUES ONLY JUESSICTION MCCP MANULUES ONLY ANY ARVANDANG UNIT PROTECTION ALC AVECTION TREPORTING MCD MCDUNTED	753 HOMERUN TO PARELBORAL INFORMATION AT AND/WEARS CREUM OR PI-05,7 PI WATELBORD CALE FOR BRANCH CACUM COMPLETED STREET			
AB ARTERE SWITCH N/A NOT APPLICABLE		AFEA (AREA FOR EVACUATION ASSISTANCE) SIGN - CELINGWALL MOUNTED, ARROWS AS INDICATED		THE PARTY OF THE P
ATE AUTOMATIC TRANSPER SWITCH MATL NATIONALLY RECOGNIZED AV AUDIVISUAL		POWER EQUIPMENT & DEVICES		GENERATOR (RATINGS AS INCICATED)
EAS BUILING AUTOMATION OS OCCUPANCY SENSOR SYSTEM P POLE			1 _	
C CONSULT PH PHASE CAT CATÉGORY E PHASE				
CATV CASEFTELEVISION SYSTEM FILE PANEL CCTV CLOSED CRICUIT TELEVISION PRIED PANEL CARD		TERMINAL CABINET (SURFACE OR FILLEN MOUNT), TYPE AS NOTED		PANELBDARD (TYPE, RATING, DEVICES AND ACCEPT
CO CADELA PROVIDE FURMENARD INSTRUL OKT OROUT PT POTENTIAL TRANSPORMER CODE APPLICABLE CODE AD OPTED BY CIDE APPLICABLE CODE AD OPTED BY	LOW VOLTAGE CABLE (NOT ROLLED IN CONDUM)	PLINKOOD TERMINAL BOARD FOR TELEPHONE SYSTEM UND. SIZE AR NOTED		ASIND(CATED)
JURSDICTION ROPT RECEPTACLE CT CURUENT TRANSFORMER RELO RELOCATE GTR CONTR STRANSFORMER FILA RUNNINGLIAN AMPS		ELECTRICAL DISTRIEURICH PANELBOARD	RECEPTABLE LETTER DEBIGNATIONS AS FOLLOWS: C = CONTROLLED	
CVD CUMULATIVE VOLTAGE DROP FITU RODFICP UNIT DENO REKOLITICH SCCR SHCHT-CROUT CURRENT	בסאסעד דעזאאוא ער גער גער גער גער גער גער גער גער גער	SWITCHE DARD OF MOTOR CONTROL CENTER OF HOUSEKEEPING PAD	CH = CLOCK HANGER TYPE D = DEKCUSHED	
DOVELS-TREADW SD SALDKE DUCT DETECTOR			U WAAP ERTRACY = BUSTING BH = EMERGENCY POWER H = ROFIZEONTALLY MOUNTED	GFR GROUND FAULT RELAY
SPOLETUROW SPOL SNGLEPOLE (E) EXISTING DOUBLE-THROW			IG = (SOLATED GROUND R = RELOCATED	
EC ELECTRICAL CONTRACTOR SPST ENCLEPOLE EF ENVIUST FAN SINGLE-THROW EN ESENERY ST EXUMPTION		Φ Μαΐακ	S= SWITCHED TR = TAMPER RESISTANT TV = TELEVISION	
EMS EXERCY MANAGEMENT SWED SWITCHEDARD SYSTEM SWER BWITCHEFAR	CONDUCTOR TICK MARK LEGEND		USB = UBROUPLEX WP = WEATHER PROOF COVER	
ETR BUSTNETO REMAIN EVEC ELECTRID WATER COLLER EDVIDING BACKBONE FAAP FIELALMINUNCIATUR TED TO BE INTERNIED	WHERE TICK MARKS ARE SHOWN, THE FOLLOWING BRAU, GOVERN.	ET BERAKER (2017) BULLE (2017) SO FOR NEMA MÉANS BERAKER (2017) BULLE (2017) SO FOR NEMA MÉANS BTANQARD NEMA 1 HATMIG	WR = WEATHER RESISTANT	
FACP FREALASM CONTROL PANEL TOB TELECOMMUNICATIONS GROUND BUILD BUILD	SWITCHED HOT (PHASE) CONDUCTORS (SHOWN TRALING MEUTRAL)	COMBRATION DISCONNECT (SAFETY) SWITCH AND MOTOR STARTER		AM AMMETER (RANGE AS \$PEC(FED OR REQUIRED)
FCA FAILT CURRENT AMPS AVAILABLE TL. THISTLOCK FCU FAX501LUNIT THIGB TELEDONOMINICATIONS MAIN	MEUTRAL (GROUNDED) CONDUCTOR	30/3/10/10R SUBJORED BACKER ASPERES/POLICIPUSE/NEWA STARTER		VN VOLTHETER (RANGE AS SPECIFIED OR REQUIRED)
FLA FULLOAD AMPS TX TRANSPORMER	UNEWITCHED HOT (PHASE) CONDUCTORS (SHOWN LEADING NEUTRAL)	(360/CEP1), NO VALUE (250/3/150/1) FOR NEWA ENCL. RATING MEANS STANDARD NEWA 1 ENCL. RATING		
GC GRENAL CONTRACTOR UP UNDERFLOOR GEC GROWNERS ELECTRODE U/G UNDERGROUND	NOTE: HASH MARKER INDICATE CLANNITY OF CONDUCTORS	אאפאיבדוב אסרטה אזגאדפא, אפאג שוב גם אסרום: ארטוב, ואס 🔀 י		
CLEWUCTOR US UNDERLAB CES GRAINIONIC ELECTRODE SYSTEM UN UNIT NEXTER GRA CRAMIN RAULT RELAY UNO UNLESS INTEE OTHERWISE	EQUIPMENT GROUNDING CONDUCTOR IN ODNOUT (GREEN INSULATION OR BARLE)	_	ABOVE COUNTER. THE	
G GRURD UP8 UNINTERPUTIBLE POYYER	- ISOLATED GROUNDING CONDUCTOR IN CONDUIT (GREEN INSULATION WITH VELLOW TRACER)		WALL TYP	
LEAL-BOX JUNCTION BOX VED WARLARDE FREQUENCY DRIVE	BRANCH CIRCUIT CONDUCTOR TABLE	S MANUAL MOTOR STARTER DISCONNECT	FLOOR, TYP	D WATT-HOUR METER, TO DEMONS DEMAND REGISTER. "IS" DE
LRA LASTER KATOR AMPS W/ WITH LTGATE LIGHTKIGALIGHTS WP WEATHER PROOF MAU AWAUP AURINT WP WEATHER RESISTANT	WHERE TICK MARKE ARE NOT SKOWN, THE FOLLOWING SKALL GOVERN	FRACTIONAL HDRSEPOWER MANUAL CONTROLLER	A MULT-BERVICE POWER POLE WITH TELEPHONE, DATA AND POWER OUTLETS A = TYPE, REFER TO PLANS, SCHEDULEB AND SPECIFICATIONS	
NAN LANGUPANTONI YAR WENTING RESISTANT NAN KAWIMA WY WATERINGHT MCA MAMMAGROUT AMPACITY XP EXPLOSION-PROOF	# OF POLES HOT (PHASE)" NEUTRAL (GROUNDED)" GROUNDING"	SH INTEGRAL HORSEPOWER MANUAL CONTROLLER		
/	2P         (1)         (1)         (1)           2P         (2)         (1)         (1)		OUTLETS AT TYPE, REPER TO PLANS, BOYEDULES AND SPECIFICATIONS	
	3P (3) (1) UNO (1)	R RELAY OR CONTACTOR IN SCHEMATICS		SFD BURGE-PROTECTIVE DEVICE
			PORE THROUGH, A = TYPE REPER TO PLANS. SCHEDULES AND SPECIFICATIONS	
	UNAMTCHEDREM, ETC.) AS INDICATED TRADUCINOUT CONSTRUCTIONS OF UNITABLE AND AS REQUIRED FOR A COMPLETE AND WORKING BY STEAL BY DERE TO SEPERIFICATIONS FOR UNITABLE SEAL (GROUNDED) CONDUCTORS, DO NOT CROUT AS A MULTI-WIRE BRANCH CROUT, UNA.	AGRETIC CONTACTOR, EZZ, COL, VOLTAGE AND NUMBER OF POLES P AS INDICATED (BLANK = 1 TG CONTACTOR, P = POWER CONTACTOR)	THERMOSTAT	
	HIGH DE ADDITIONAL SOLATED GROUNDING CONDUCTORS WHERE INDICATED.	TS TIME SWITCH		GROUND ROD
1	REFERTIO SPECIFICATIONS, FLANS, NOTES, WIRING AND CONTROL DIAGRAMS FOR ADDITIONAL CIRCUITING REQUIREMENTS.	PC PHOTOGEL	BLANK FACE GFCI FEED THROUGH DEVICE	
		1	"MAROL DEMONSTRATED WITH DURLEX RECEPTACLE, WHEN USED IN COMBINATION WITH	
	THROUGHOUT THE DRAWINGS DIFFERENT UNE-TYPES ARE LISED IN COMBINATION WITH THE SWIRDLE TO INDICATE THE STATUR OF THEM AS EXISTING, TO BE DEMOLISHED, TO BE INCLUDED AS PART OF NEW WORK AND/OF HEMS WHICH ARE ANTIGRATED TO BE		CITIER DEVICES MEANING IS SMILLAR FOR THOSE DEVICE TYPES,	,
l í	TO THE VIEW IN WHICH THEY ARECAD. BUACAUS PHONE OF THESE LINETYPES ARE RELATIVE		REFER TO LIGHTING CONTROL, DEVICE SCHEDULE FOR ADDITIONAL DEVICE SYMBOLS AND DEFINITIONS SPECIFIC TO THIS PROJECT.	
1	TO FULLY DESIGNED ALL NECESSARY CONSTUMINATION IN ANALYMINASIS NOT INTENDED THE CONTRACTOR AS PART OF THEIR RESPONSED THE ANY BILLY HASES DESCRIBED IN THE CONSTRUCTION DOCUMENTS ARE CONSERVATION ANY INTENDED TO INDICATE A	ſ		
	BRUAD ORDER FOR THE BAKE OF DEBOREING THE FRONCET. THE FOLLOWING UNETYPES MAY BE USED ON ANY DEVICE, EQUIPMENT, NOTE, LINE, SHAPE, ETC.			
I [	EXISTING NEW			
	DEMOLICH			
	······································			* F# VOLTAGE DROP SPREADSHEET

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### GENERAL ELECTRICAL NOTES:

- READ THE SPECIFICATIONS AND REVIEW CRAWINGS OF ALL DONSIONS OF WORK. COURDINATE THIS WORK WITH ALL OTHER DIVISIONS OF WORK AND ALL SUBCONTRACTURS. PROVIDE ALL SUBCONTRACTURS. PROVIDE ALL SUBCONTRACTURS. WITH A COMPLETE SET OF BID COOLIMENTS.
- 3. DRAWINGS ARE DIAGRAMMATIC ONLY AND REPRESENT THE GENERAL SOCRE OF THE WORK. REVENT THE GENERAL NOTES, SPECIFICATIONS AND FLANS FOR ADDITIONAL RECURREMENTS THAT MAY NOT BE SPECIFICALLY CALLED OUT IN THIS PORTION OF THE CONSTRUCTION DOCUMENTS, NOTEY THE ARCHITECT OF ANY COMPLICTS OR DISCREPANCES PRIOR TO SUBMISSION OF HID BID.
- 4. FURNISH TO THE ARCHITECT A COPY OF INSPECTION REPORTS AND APPROVAL CENTRICATES FROM LOCAL AND STATE INSPECTIONS.
- 5 DRAWINGS AND SPECIFICATIONS GOVERN, WHERE THEY EXCEED CODE REQUIREMENTS
- ALL GFCI PROTECTED CIRCUITS SHALL HAVE INDMIDUAL AND DEDICATED NEUTRALS.
- 7. ALL DATA WIRING IS BY OTHERS. PROVIDE DATA CONDUITS, WITH PULL WIRES.
- REFER TO SPECIFICATIONS FOR ADDITIONAL PANELBOARD INSTALLATION REQUIREMENTS AND IDENTIFICATION.
- 10. FROVIDE ALL MOUNTING AND SUPPORT HARDWARE FOR LIGHT FORTURES TO MEET SPECIFIED MOUNTING HEIGHTS.
- 11. ALL JUNCTION BOXES SHALL BE RIGIDLY ATTACHED TO STRUCTURE OR MILLWORK
- 12. VERIFY REDUREMENTS WITH EQUIPMENT SUPPLIER AND PROVIDE ALL NECESSARY ITEMS TO MEET THE EQUIPMENTS ELECTRICAL INSTALLATION REQUIREMENTS
- 13. All site lighting branch circuit wiring Shall be routed in minimum of 1-1/4" conduit, unless noted otherwise
- 14. MINIMUM CONDUCTOR SIZE FOR ALL Exterior lighting circuits shall be fig AWG, unless noted otherwise
- 15. LOCATIONS OF UTUITIES SHOWN ON THIS PLAN HAVE EVEN OSTAINED FROM INFORMATION MAALABLE VERIFY LOCATIONS OF ALL UTUITY UNES BEFORE PERFORMING WORK. UNTEY ENGENEE INMEDIATELY OF ANY DISCREPANCIES.
- 16. IN GENERAL, DO NOT ROUTE CONDUIT UNDER POST-TENSIONED CONCRETE SLASS. ROUTE CONDUIT UNDER FORM-RIFLACE WALKWAYS OR UNDER LANDSCAPE. CONSULT WITH LANDSCAPE ARCHITECT AND STRUCTURAL ENGINEER FOR PREFERED CONDUIT ROUTING.
- 17. COORDINATE FINAL FOLE LOCATIONS WITH OWNER PRIOR TO ORDERING FOLES AND LIGHTS AND VERIFY CHARGES WITH BOORTS LIGHTING MANUFACTURER. INSTALLATION OF POLE BASES TO BE COORDINATE WITH POET-TENSIONED SLAB INSTALLATION.
- 15. CCORDINATE ROUTING OF SITE ELECTRICAL WITH EXISTING FENCE FOST LODATIONS. SIGNAGE, PAVEMENT, ETC PRIOR TO ROUGHAN OF CONDUTTS.
- 18. CONDUIT AND CABLE ROLITING SHOWN ON PLAN IS DIAGRAMMATIC ONLY. DETERMING BEST ROLITING IN FIELD AND OCCUMENT EXACT ROLITING ON FECTOR ORAWINGS.
- 20. PROVIDE GRA CONDULT ELBOW AT ALL CONDUIT SENDS.

### CONFLUENCE

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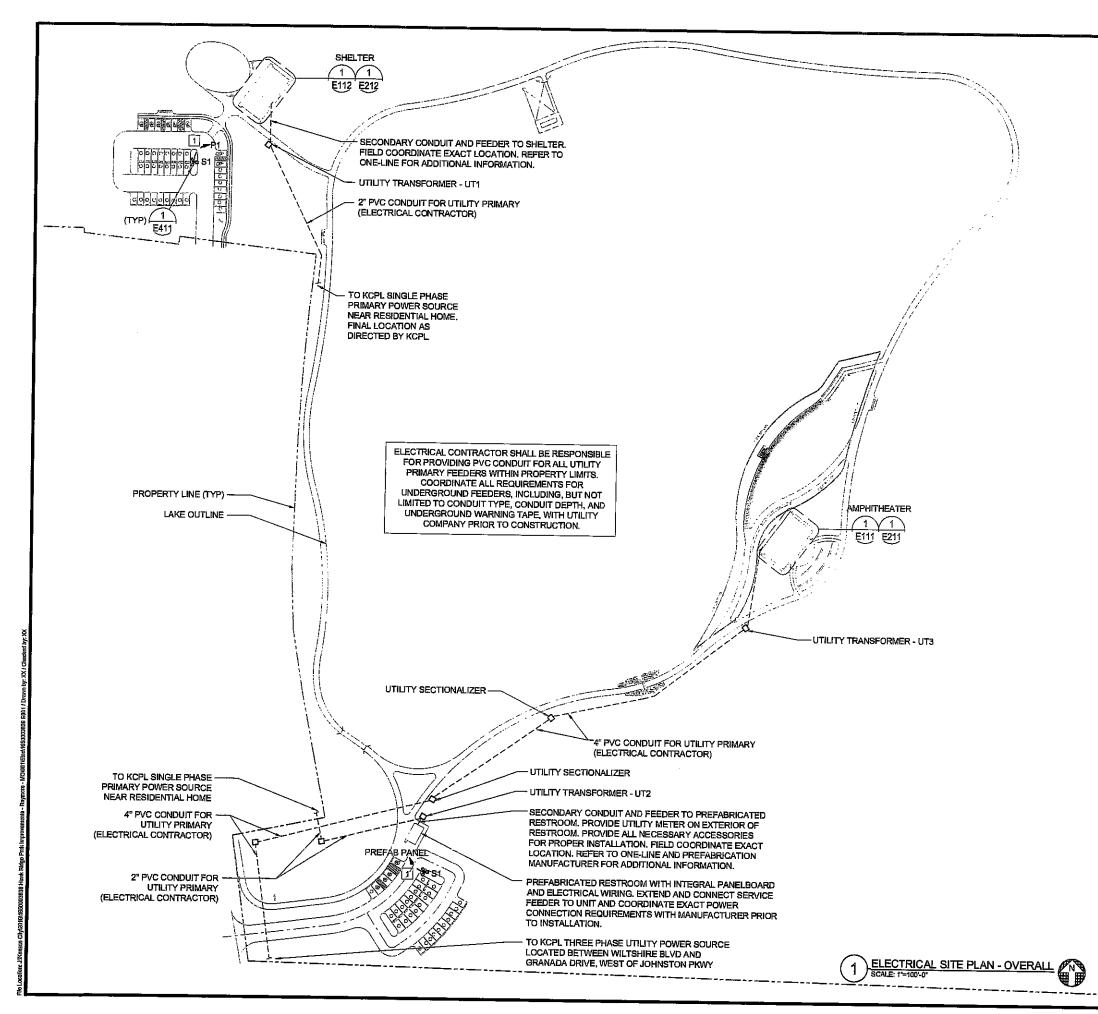


**REVISION SCHEDULE** 

LEGENDS

CONFLUENCE PROJECT ND: 16081KC





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#### GENERAL ELECTRICAL NOTES:

- 1. REFER TO SHEET E000 FOR GENERAL ELECTRICAL NOTES.
- 2. CONTRACTOR SHALL GOORDINATE TRANSPORMER CONCRETE PAD SIZES WITH KCPL PER TRANSPORMER KVA SIZE AND UTILITY CONCRETE PAD REQUIREMENTS.
- COORDINATE KOPL REQUIREMENTS FOR DUCT BANKS UNDER PAVED AREAS. PROVIDE DUCT BANKS FOR PRIMARY CONDUITS AS REQUIRED PER KOPI, STANDARDS.

#### ELECTRICAL PLAN NOTES

 ROUTE CIRCUIT THROUGH CONTACTOR AND CONTROL VIA PHOTOCELL ON/OFF.

### CONFLUENCE

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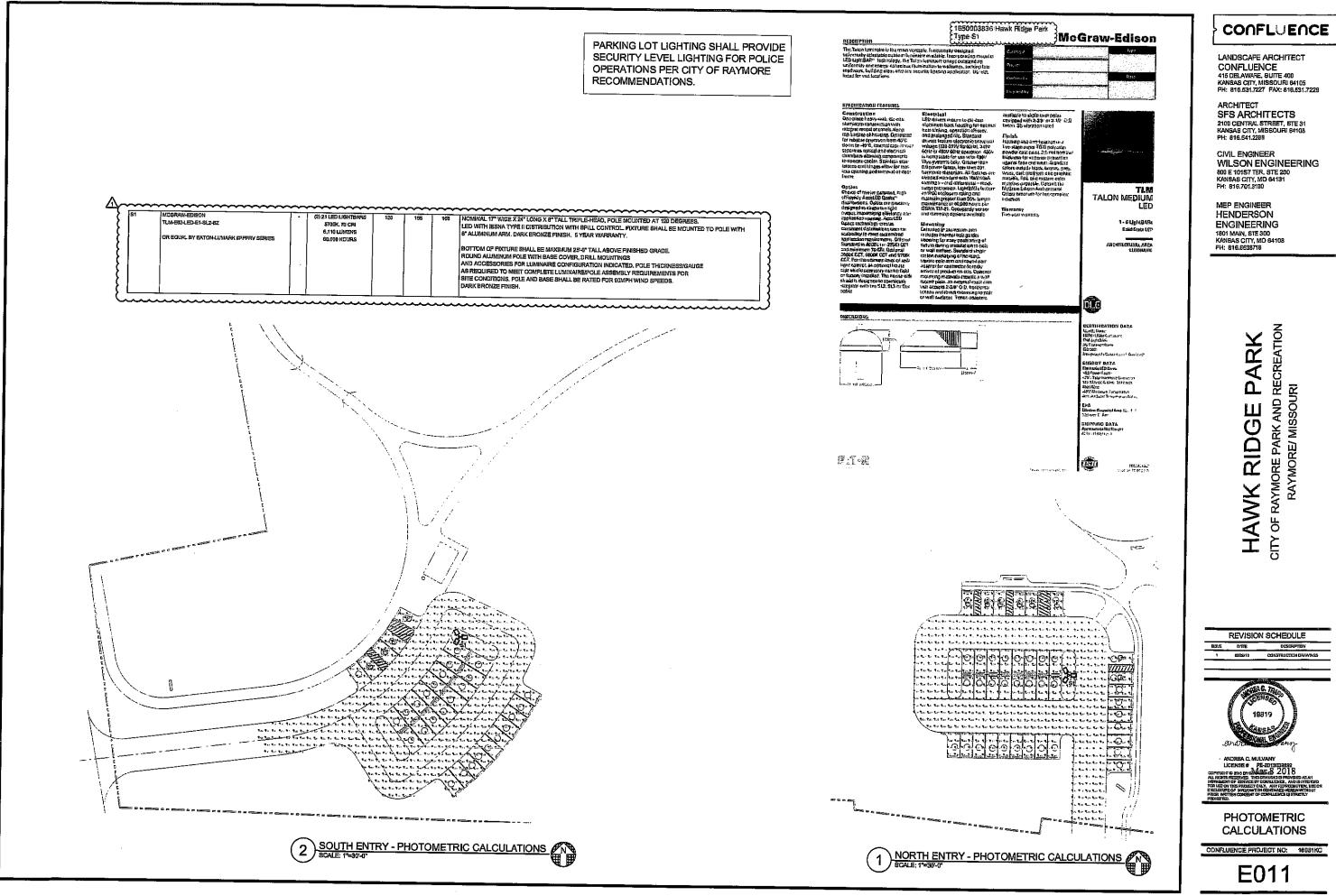
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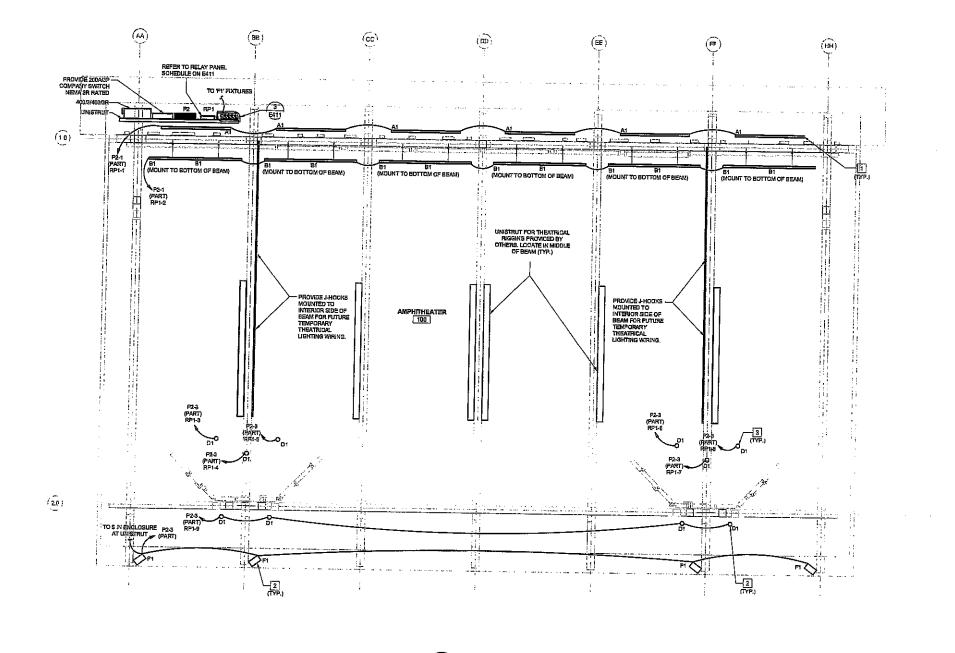
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1 LIGHTING FLOOR PLAN - AMPITHEATER

#### GENERAL ELECTRICAL NOTES:

- 1. REFER TO SHEET BOD FOR GENERAL ELECTRICAL NOTES.
- 2. REFER TO ARCHITECTURAL ELEVATIONS AND DETAILS FOR TYPICAL MOUNTING LOCATIONS AND CONFIGURATIONS.

#### ELECTRICAL PLAN NOTES

- 1. FIXTURE CENTERED BETWEEN BEAMS. CENTER FIXTURES ON TOP OF LATER BEAM BETWEEN ROOF BEAM MEMBERS LIGHT BOURGE ENALLE AR MIED AT BAME DEGREE AS ROOF FITCH. ROUTE CONCULT RACEMAY THROUGH HOLLOW METAL STELL STRUCTURE. COORDINATE WITH PRE-ENGINEEREM MANUFACTURE.
- 2. ΜΟΙΝΤ ΡΙΧΤURE ΤΟ SIDE OF BEAM STRUCTURE, RE ARCH COORDINATE CONDUIT RACERWARROUTING THROUGH STRUCTURE WTH PRE-KOMBERED DETRUCTURE MANUFACTURER, REFER TO ARCHITECTUREL MOUNTING DETAIL FOR PURTHER UNFORMATION. (77)
- 3. REFER TO ARCHITECTURAL MOUNTING DETAIL (TYP. THIS FIXTURE)

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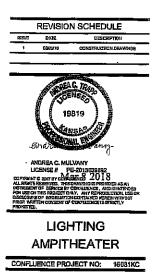
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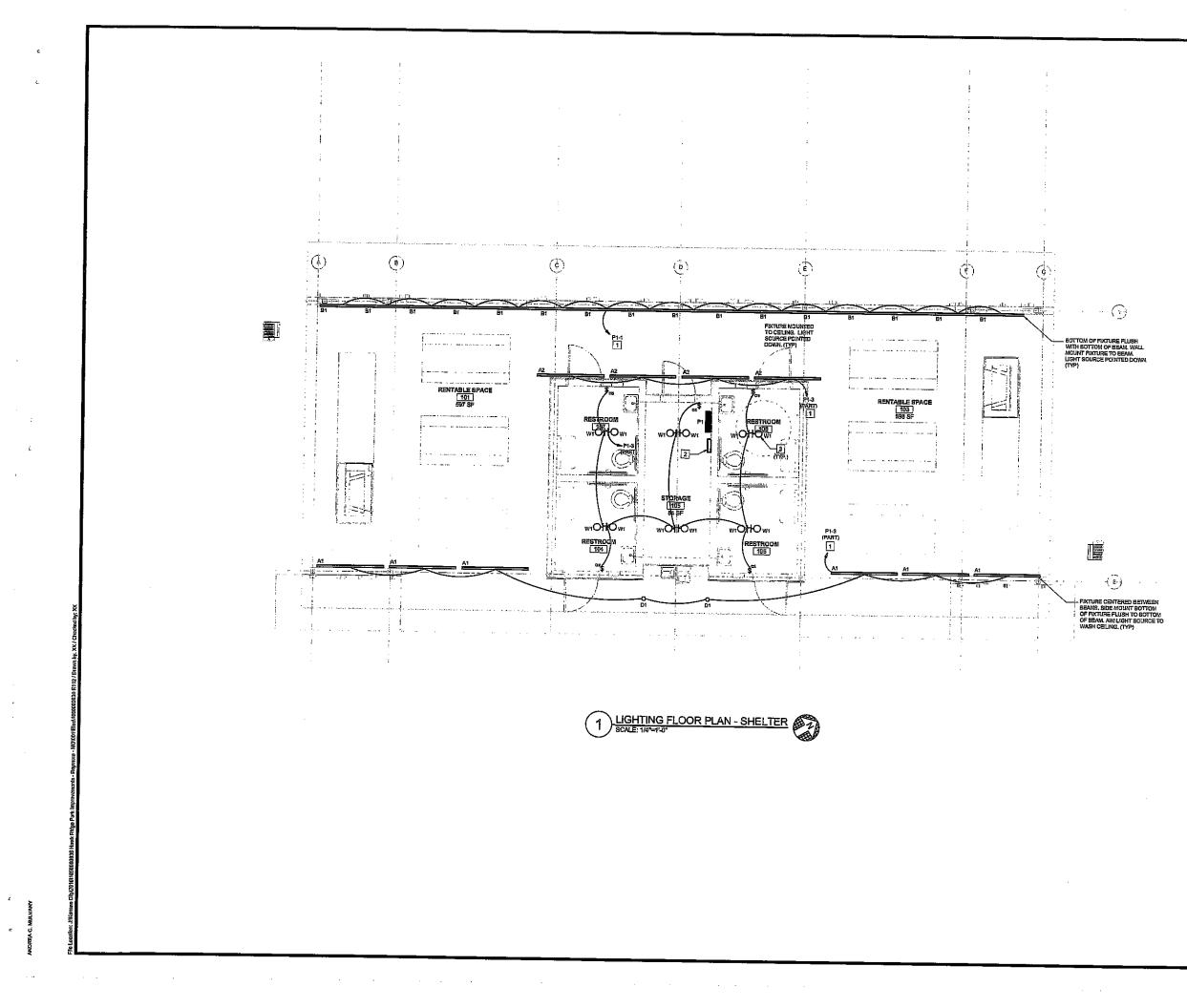
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#### GENERAL ELECTRICAL NOTES:

1 REFER TO SHEET 5000 FOR GENERAL ELECTRICAL NOTES.

### ELECTRICAL PLAN NOTES

- 1. CONTROL VIA PHOTOCELL ON/OFF.
- 2 CONTACTOR WITH INTEGRAL TIMECLOCK FOR EXTERIOR SHELTER LIGHTING, LIGHTING TO TURN OFF AT END OF DAY TIME SPECIFIED BY OWNER, REFER TO DETAIL 2. SHEET E411.
- FIXTURE MOUNTED TO SIDE OF BEAM. REFER TO ARCHITECTURAL MOUNTING DETAIL (TYP. THIS FUXTURE)

### CONFLUENCE

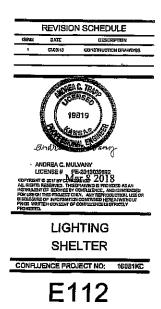
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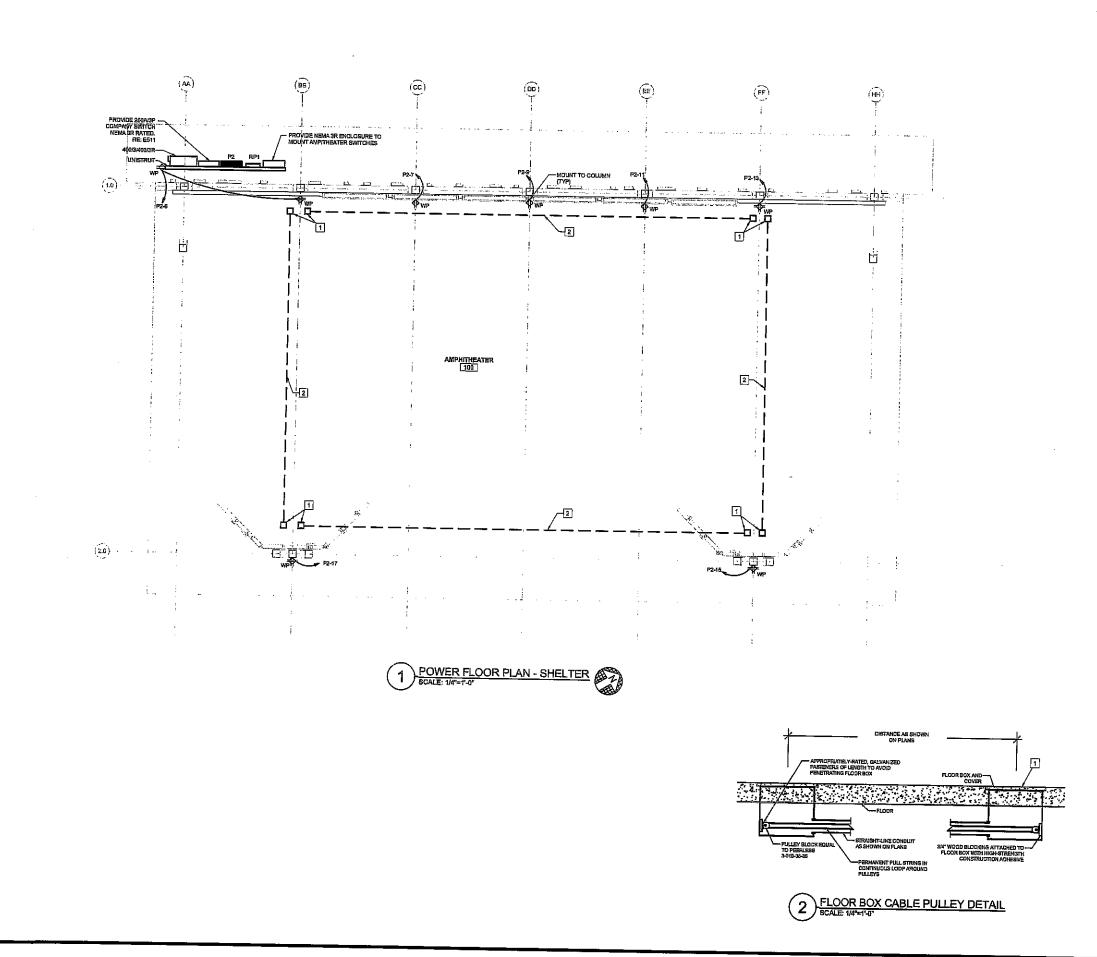
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MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, 8TE 300 KANSAS CITY, MO 64108 PH: 816,6538718







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#### GENERAL ELECTRICAL NOTES:

1 REFER TO SHEET EDDI FOR GENERAL ELECTRICAL NOTES.

#### ELECTRICAL PLAN NOTES

- 1. WIREMOLD COFD TYPE 5 BOX WITH CCFB-NS-H-C1 COVER. MOUNT WITH COVER PLUSH WITH FNISHED PLOCE. ORIENT BOX WITH SHORTER SIDEE FACING SUCH THAT CONNECTION (N ON SHORTER SIDE. REFER TO DETAIL 2 ON THIS SHEET
- 2. 4" PVC CONDUIT FOR TEMPORARY AUDID CABLING. INSTALL IN STRAIGHT LINE WITH NO BENDS. PROVIDE PERMANENT PULL STRING ON PULLEY SYSTEM. REFER TO DETAIL 2 ON THIS EMEET.

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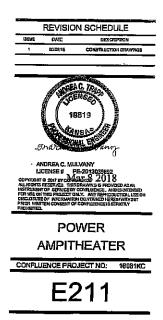
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ARCHITECT SFS ARCHITECTS 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.641.2288

CIVIL ENGINEER WILSON ENGINEERING 800 E 1015TTER, STE 200 KANSAS CITY, MD 64131 PH: 816.701,3100

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KAN9AS CITY, MO 64108 PH: 816.6638718





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 $(\mathbf{A})$ (E) (c) (i)  $(\widehat{\mathbf{E}})$ (<u></u>) (F) Ð. ÷ ė · 🗇 🖓 COURDINATE FINAL LOCATION WITH SCREEN FLEMENTS (TYP) P1-15 'PI-19 . ..... percenter and the same in the ..... _____ - ----_ v. Ľ RENTABLE SPACE RENTABLE SPACE 103 598 SF 1 ...... RESTROOM RESTROOM ····· ‴-⇒ ----------lan ar anna an an an الدري . ..... constations. 1.000 Ö STORAGE 105 84 SF 123 ₩Ż (EWH æ RESTROOM RESTROOM **M**** 4476 . . τ<del>..</del>. or Gr , <u>é</u> , <u>é</u> , <u>é</u> , 1.... ; . · ----1 1 ; · 1 : · ! . · · · · ,

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1 POWER FLOOR PLAN - SHELTER

### GENERAL ELECTRICAL NOTES:

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1. REFER TO SHEET EXID FOR GENERAL ELECTRICAL NOTES

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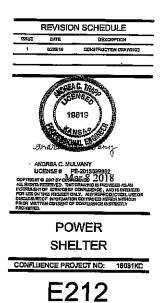
LANDSCAPE ARCHITECT CONFLUENCE 415 DELAWARE, SUITE 400 KANSAS CITY, MISSOURI 64105 PH: 816.531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTS 2400 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 816.541.2288

CTVIL ENGINEER WILSON ENGINEERING 800 E 101ST TER, STE 200 KANSAS CTY, MO 84131 PH: 816.701.3700

MEP ENGINEER HENDERSON ENGINEERING 1801 Main, STE 300 Kansas CITY, MD 64108 PH: 816.6638718





TYPE	MANUFACTURER/MODEL #		LAMPS	_	INPUT		
-		NO.	TYPE	VOLT	WATTS	VA	
A1	ECOSENSE TROV LØDASYM I JOG-49-12-30-80-MULT-ASYM OR PRE-APPROVED EQUAL		LED 3000K, 80 CRI 923 LUMENB/FT	120	12	14	NOMINAL 2" WIDE X 2" TALL X 72" LONG ASYMETRIC GRAZING. EXTRUDED ALUMINUM. WET LOCATION LISTED. PROVIDE FINE ADJUSTABLEL BRACKET. PROVIDE ALL MOUNTING, CABLES AND BOXES AS REQUIRED FOR A COMPLETE INSTALLATION.
A2	ECOSENSE TROV LSASYM LSD-E8-SA-E0-MULT-ASYM OR FRE-APPROVED EQUAL		LED Soddk, 80 CRI 482 LUMENR/FT	120	6	8	I NOMINAL 2° WIDE X 2° TÀLL X 72° LONG ASYMMETRIC GRAZING. EXTRUDED ALUMINUM. WET LÒCATION LISTED. FROMDE FINE AQUISTABLE L BRACKET. FROMDE ALL MOUNTING, CABLES AND BOXES AS REQUIRED FOR A COMPLETE INSTALLATION.
11	Lumenpulse-lumenfacade Logp Ho-120-49-30(K-30x8)-umas-cc-koerd Or Pre-Approved Equal	•	LED 3000K, 89 CRI 3,862 (JIMENS	120	61	68	NOMIAL 3" WIDE X 3.5" TALL X 49" LONG 30 DEGREE BY 60 DEGREE BEAM. EXTRUDED ALUMINUM WET LOCATIO PROVIDE UNIVERSAL ADJUSTABLE WALL MOUNTING KIT AND RADIAL LOUVER, LIGHT SOURCE POINTED TOWARL PROVIDE ALI MOUNTING, CALES AND BOXES AS RECUIRED FOR A COMPLETE INSTALLATION. PXTURE TO BE CUSTOM COLOR SPECIFIED BY ARCHITECT.
21	B-K LIGHTING DS-LED-665-87-827-12-11 IOR PRE-APPROVED EQUAL	-	LED 3000K, 80 CRL 1,974 LUMENS	120	7	9	LED FLOODLIGHT WITH A SPOT OPTIC. DIMMING, ALLIMINUM WITH BRONZE FINISH. PROVIDE WITH SOFT FOCUS LENS AND HONEYCOMB BAFFLE.
1	SPEC GRADE AFL-50W-50X135-EL-TL-FL OR PRE-APPROVED EQUAL	-	LED 2900K, 80 CRI 8,660 LUMENS 100,000 HOURS	120	80	90	NOMINAL 16 1/4" WIDE X 14 3/8" LONG X 3 1/2" DEEP. DIE-CAST ALUMINUM. LED FLOODLIGHT WITH A SPOT OFTIC.
1	MCGRAW-EDISCN TLM-EDI2-LED-E1-812-82 OR EQUAL BY EATON-LUMARK #FFFRV SERIES		2021 LED UGHTBARS	120	155	160	IOMINAL 17 WIDE X 24"LONG X 8" TALL TRIFLE-HEAD, POLE MOUNTED AT 120 DEGREES. LED WITH IESNA TYPE II DISTRIBUTION WITH SFILL CONTROL FXTURE SHALL BE MOUNTED TO POLE 8" ALL/MINUM ARM. DARK BRONZE FINISH. S YEAR WARRANTY. BOTTOM OF FIXTURE SHALL BE MAXIMUM 25"4" TALL ABOVE FINISHED GRADE. ROUND ALL/MINUM POLE WITH BASE COVER, DRILL MOUNTINGS AND ACCESSORIES FOR LUMINAIRE CONFIGURATION INDICATED. POLE THICKNESS/GAUGE AS REQUIRED TO MEET COMPLETE LUMINAIRE/POLE ASSEMBLY REQUIREMENTS FOR STET CONDITIONS. FOLE AND BASE SHALL BE RATED FOR SOMPH WIND SPEEDS. DARK BRONZE FINISH.
1	Hubeel, VBGL-1-UNV Eaton Lighting Lithonia		LED 4000K, 70 CRi LUMENS 72,000 HOURS	120	71	13	WALL MOUNT LED GLOBE. RUGGED DIE CAST ALLIMINUM HOUSING. POWDER-COAT PAINT IWTH CORROSION RES PROSTED GLASS LENS AND VANDAL REBISTANT SET \$CREWS. IP66 LISTED

### LIGHT FIXTURE SCHEDULE NOTES

#### ERAL NOTES

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BENERAL NOTES: A. REPRESENTATIVE AGENTS SHALL BE ALLOWED TO OFFER MINILOT PRICING (MLP) FOR LIGHT PIXTURES SPECIFED. B. VERIFY CELING CONDITIONS AND COORDINATE LIGHT FIXTURE MOUNTING HARDWARE AND TRIMS NEEDED TO SUIT CELING CONDITIONS FROM TO ORDERING. C. VERIFY FINISH AND COLOR WITH ARCHTECT FRIDA TO FLACING ORDER. D. VERIFY FINISH AND COLOR WITH ARCHTECT FRIDA TO FLACING ORDER. E. REFER TO ARCHTECTURAL DRAWINGS AND DETAS. FOR EXACT LOCATIONS, MOUNTING HEIGHTS AND ADDITIONAL MOUNTING INFORMATION. CONTACT ARCHITECT (IMMEDIATELY IF THERE ARE DISCREPANCIES DETIMENT THE ARCHITECTURAL DRAWINGS AND DETAS. FOR EXACT LOCATIONS, MOUNTING HEIGHTS AND ADDITIONAL MOUNTING INFORMATION. CONTACT ARCHITECT (IMMEDIATELY IF THERE ARE DISCREPANCIES DETIMENT THE ARCHITECTURAL LIGHTING FLANS. F. GATALOG NUMBERS SHALL NOT BE CONSIDERED COMPLETE AND MATERIAL SHALL NOT BE ORDERED BY MANUFACTURER AND CATALOG NUMBERS ONLY. FIRST READ THE COMPLETE DESCRIPTION. NOTES AND SPECIFICATIONS IN CONJUNCTION WITH THE CATALOG NUMBER TO DETERMINE THE EXACT MATERIAL AND ACCESSORIES TO BE ORDERED. THE MANUFACTURES LISTED ARE THE BASIS FOR THE DESIGN. G. CONTRACTOR SHALL PROVIDE ALL LIGHT FIXTURES UNLESS NOTED OTHERWISE. H. REFER TO LIGHTING CONTACTOR OR RELAY SCHEDULE AND PANELBOARD SCHEDULES FOR CONTROL INFORMATION. J. REFER TO LIGHTING CONTACTOR OR RELAY SCHEDULE AND PANELBOARD SCHEDULES FOR CONTROL INFORMATION. J. REFER TO DIVISION 26 SPECIFICATIONS IN ORDER INFORMATION REGARDING CONTROL WIRING AND COMPARIBILITY.

NOTES: 1. CATALOG NUMBER REPRESENTS MANUFACTURER LIBTED FIRST. OTHER MANUFACTURERS LISTED ARE CONSIDERED EQUIVALENT FOR SUBSTITUTION. 2. DUE TO AESTHETIC OR PERFORMANCE ORITENA, SPECIFIED MANUFACTURERS SHALL BE THE OXLY MANUFACTURER ALLOWED TO BID UXLESS OTHERWISE APPROVED BY ARCHITECT. 3. CONTRACTOR SHALL SUPPLY A COMPLETE AND OPERATIONAL SYSTEM TO COMPLY WITH DESIGN INTENT 4. FRONDE TRACTORY-INSTALLED MANUMANY WATTAGE LIBLE ON INCANDESIT, HALOGEN AND ENERGY-SAVING FLUORESCENT LANPED LIGHT FIXTURES THAT CORRESPOND TO THE MAXIMUM WATTAGE INDICATED IN THE LIGHT FIXTURE SCHEDULE 5. CONTRM FINAL FIXURE LENGTHS WITH FIELD-CONFIRMED CONDITIONS PRIOR TO CROEMING. 6. REFER TO POLE BASE DETAILS FOR ADDITIONAL INFORMATION. 7. SUBMIT SIGNED/SEALED DOLE BASE DETAILS FOR ADDITIONAL INFORMATION. 7. SUBMIT SIGNED/SEALED FOLLE FOUNDATION.

NOTES           2.5           1           2.6           2           2           2           2           2           2           2           2           2           2           2           2           2           3           3           3			7
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# CONFLUENCE

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CIVIL ENGINEER WILSON ENGINEERING 800 E 101ST TER, STE 200 KANSAB CITY, MO 64131 PH: 816.701.3100

MEP ENGINEER HENDERSON HEINDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MO 64108 FH: 816.6638718





BUS / MAIN VOLT	NELBOARD: P1 NE AMPS: 125A 18725/TYPE: 180A MCB 1876HASE: 240/120V, 1PH, 34 1709; 1		AIC R SERV MOUN	TROM: ATING: ES: SH ITING: TION: 8	ELT SUF	10 ER UFAI	000 FUL		isformer Ed					
CKT	DESCRIPTION		v	<b>dLTAMPS/PE</b>	låse	_	BKR		T P	_	Martene			_
NO.				A	8	NO.	AMP	٢	רין		ND.	A	ASE B	-
1	LTG - N. LINEAR UP & DOV	N		1.000		112	20	-	i T	20		<u>+</u>	╞═┻	-
3	LTG-S LINEAR DLRRS			1 1,000	212	12	20	1	1	20	_		<u> </u>	SPA
6	ROPT - RRS, STORAGE			900	212	12	20	<u> </u>	Ι÷	20			<u> </u>	SPA
7	RCPT - WEST CORE WALL				350	12	20	1	H	20	_		<u> </u>	SPA
9	RCPT - EAST CORE WALL			380	4 440	12	20	1	1	20	-	<u>-</u>	<u> </u>	SPA
				1	360	12	20	1	1	20			<u> </u>	SPA
	RCPT-EAST FIREPLACE W			360		12	20	1	÷	20			<u> </u>	SPA
16	RCPT-NW&N COLUMN 1				180	12	20	1	H	20			<u> </u>	SPA
17	RCPT - MID N. COLUMN & STOR DOOR			360	1.00	12	20		H	20		<u> </u>	┢───	SPA
19	RGPT-NE&N COLUMN 2				360	12	20	1	1	20			<u> </u>	SPA
21	RCPT - WATER COOLER			600	1-000-	12	20	1	1	20			<u> </u>	SPA
艺	WATER HEATER			<u> </u>	1 1.500	12	20	1	1	20	-			SPA
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41	SPARE			_	ii		20	Ħ	1	20				SPAF
	SUBTOTAL			3.580	2,972			• •				_		SPAP
TOTA	LPHASEA-VA	3,580	LOAD		CONN. VA	<u> </u>				-			_	<u> </u>
	ANPS	30	COOLING	_	CUNN. VA		DF		LO/			CONN, VA		
TOTAL	PHASE B - VA	2,972	HEATING				1.00			Ric				14
	AMPS 25 LIGHT						0			NDISP				1.3
τοτ					1,212	<b>ļ</b> .	1.25	- 6		CHEN				1.0
					3,640		1.0/.5		_	STING				1.0
		MOTORS				1.00	-		MOTO				1.2	
				SUPP HEAT 1,50 MISC BOULP		1.00			SHOW WNDW					1.2
	BDARD NOTES		Janao Beur		L		1.00	(	ΤG	TRACK				1.0

PANELBOARD: P2 (NEW)

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eus. Main Volt	NELBOARD; P2 (NE AMPS: 180A 1912E/TYFE: MLO 1917HASE: 2087/1207, 3PH, 41					AIC R Serv	ROM: ATING: E5: AM	: IPHD	100at THEAT	FULL ER	SFORM	IER UT-3 ED				EQUIPMENT GRC	UND BUS
	10N; 1						TON: A				UNIS	TRUT					
CKT NO.	DESCRIPTION	Vi	OUTAMPS/PI-	iase B		WIRE NO.	BKR	Р			WIRE NO.	TAMPS/P			1	DESCRIPTION	1
	LTG - NORTH UP AND DOW	N	900	<u></u>	<u> </u>	112	20		_	_	NO.	<u>A</u>		<u> </u>	<u> </u>		
3	LTG - SOUTH SPOTS / FLOC	DS	<u> </u>	450		12	20	1	-	20		i	<u> </u>		SPARE		
	RCPT - NORTH WALL 1		1		360	12	20	-		20			· · · · · · · · · · · · · · · · · · ·		SPARE		
7	RCPT-NORTH WALL 2		180	i		112	20	1		20	_				SPARE		
	RGPT - NORTH WALLS			180		12	_	_		20			<u> </u>		SPARE		
	ROPT - NORTH WALL 4	_	┪───-	100	180		20	1		20			l		SPARE		
	RCPT - NORTH WALL 1		360			12	20			20		-		1	SPARE		1
	RCPT - SOUTHEAST WALL			360		12	20	<u> </u>		20		L.,			SPARE		1
	RCPT - SOUTHWEST WALL		;	300		12	_ 20;	11		20					SPARE		
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<u>61</u> ]							20	7	1 2	0					SPARE		
_	SUSTOTAL		1.440	990	950						Î					SUBTOTAL	- 4
TOTAL	PHASEA - VA	1,440	LOAD		CONN. VA		ÖF 1	- ÍI	OAD	_		CONN. VA	· <u> </u>	05		JUBIOIAL	
	AMPS	12	CODLING				1.00		REFRI	-		CONTRACT AND			1		
TOTAL	PHASE B - VA	890	HEATING				0		SIGNA			··- ···	·	1.00	ļ		
	AMPS	8	LIGHTING		1,350		1,25		CTCH					1.25			
TOTAL	PHASE C - VA	900	RECEPTAC	LES	1,980		1.0/.6		DIST				·· · · ·	1.00			
_	AMPS	8	MOTORS				1.00		RGM		g }			1.00			
TOT	AL PNLED - VA	3,330	SUPP HEAT	···· ···	1		1.00		SHOW					1.25	π	TAL DEMAND	
	AMPS	9	MISC EQUIP				1.00		TG TF					1.25			3,668 VA
	Board Notes 2# - Via LTG Contactor #	_												1.00			10 A

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	EQUIPMENT GROUND BUS	_
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1.00	29 A	
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## CONFLUENCE

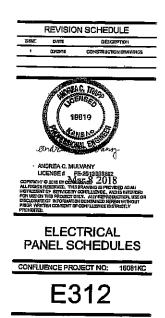
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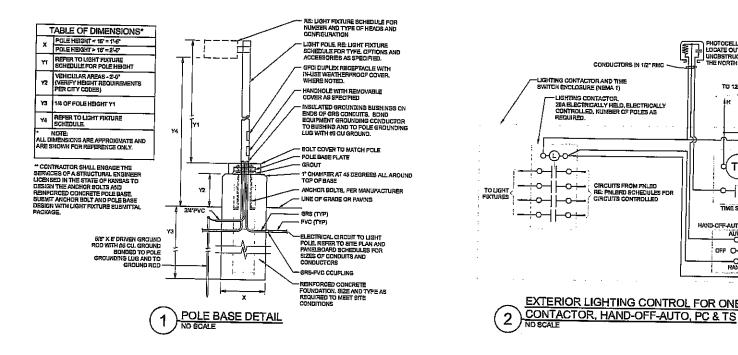
MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANEAS CITY, MO 64108 FH: 816.6338718





ZONE	DESCRIPTION	AUTOMATION SCENARS	2		
		SCHEDULE	SCHEDULEON	ASTRONOMIC	ASTRONOMIC OF
A	AMPHITIEATER GENERAL LIGHTING				GUNEDUCE OFF
9 (	ALIPHITHEATER SPOT LIGHTING	1	· · · · ·		<u> </u>
	SECURITY INPUT	IRECOMES INFORTERANT	EXCURITY SYSTEM TO TUR	And the second second	
PICAL SCHEDULE.				AVAL DISTLID ON	
CONFIRM FINAL TIL OCC	e schedule from owner phior to system programming IPED Hours.				
CONFIRM FINAL TIL OGCI	C SCHEDULE FROM OWNER PHIOR TO SYSTEM PROGRAMMING PED HOURS, MONTRU: 700 AM - 1010 PM				
CONFIRM FINAL TIL OG CI	JPED HOURS.				
CONFIRM FINAL TR. OGC	/PED HOURS. MONTER: 720 AM - 10:20 የአ				

PANELN		RP1	
LOCATIO	IN:	AMPHITHEATER	
RELAY	CIRCUIT	LOAD CONTROLLED	ZONE
1	P2-1	LTG - AMPHITHEATER NORTH UPLIGHTING	A
2	P2-1	LTG - AMPHITHEATER NORTH DOWNLIGHTING	A
3	P2-3	LTG - SW SPOTLIEHT 1	1 6
4	F2-3	LTG - SW SPOT LIGHT 2	
5	P2-3	LTG - SW SPOTLIGHT S	Ē
6	P2-3	LTG-SESPOT LIGHT 1	
7 1	P2-3	LTG-SE SPOT LIGHT 2	
8	P2-3	LTG - SE SPOT LIGHT 3	E B
9	P2-3	L'IG - SOLITH GENERAL LIGHTING	Ā
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			LIGHTING CONTROL DEVICE S					
	C. G		WALL SWITCH OCCUPANCY SENSORS	· · · · · · · · · · · · · · · · · · ·				
SYMBOL TYPE	MANUFACTURER	EDUIVALENT		COVERAGE		<u> </u>		
THE	WATISTUPPER	MANUFACTURER	DEVICE DESCRIPTION	(WXU)	ONMODE			
	Pix-102	COOPER	WALL NOUNT PASSIVE INFRARED OCCUPANCY SENSOR	NAJOR 30 x35	RANUAL	15BN	VOLTAGE	NOTES
\$ ⁶⁶⁹	Pervilua	HUSBELL	SINGLE RELAY, INTEGRAL MANUAL OVERRIDE SYNTCH.	AUDIOR 15 x20	( ICANUAL	10 10 10 10	120/277V	
+		LEVITON	LINE VOLTAGE.					1
		SENSORSWITCH				[	1	
WARDL	MANUFACTURER	EQUIVALENT	WALL SWITCHES			<u> </u>	1	
TYPE	MODEL/SERIES	MANUFACTURER						
1076	WATTSTOPPER	COOPER	DEVICE DESCRIPTION				VOLTAGE	NOTES
	LNSW IN1	HUBBEL	LOW VOLTAGEMOMENTARY SVITCHFOR MANUAL ON OFF CONTROL INTEGRAL LED	LLUNINATED WHEN LUAD				3
\$		LEVITON	ISON				240	3
-	i	SENSOREWITCH						
	WATTSTOPPER	LCAD		_				Í
	LA1559-105	COOPER	DIGITAL MULTI-BUTTON SYNTCH FOR MANUAL DRUGHT AND SCIENE CONTROL. INTEGRU	ALLED AT EACH BUTTON			24	
* ***	E	CODMER	IS BUUMWATED WHENLOAD IS ON, "MS#"INDICATES MASTER SWITCH; REFER TO SWI	TOH SCHEDULE FOR			249	
5	1		FROGRAMMING "18" INDICATES LOCAL CONTROL SWITCH, WITH & DESIGNATING BUTT	ON QUANTITY, REFER				[
· ·			TO UGHTING PLAN FOR PROGRAMMING SHALL BE FROM SAME MANUFACTURER AS R	ELAY FANEL				
			WALL DARING SWITCHES					
WINEOL	MANUFACTURER							
a set little		EQUIVALENT						
	MODEL/SERIES	EQUIVALENT MANUFACTURER	DEVICE DESCRIPTION		<u>г</u> — т	· · · · ·	<del>.</del>	·
TYPE						WWITAGE	VOLTAGE	NOTES
	MODEL/SERIES	MANUFACTURER	DEVICE DESCRIPTION 0-10/ LOW VOLTAGEPUSH BUTTON TYPE DIMMER SWITCH WITH LED STATUS INDICAT	DR	LOAD TYPE	WWITAGE 1000	VOLTAGE 120V	NOTES
1777E \$	KODELSERIES WATTSOPPER LADM 101	MANUFACTURER COOPER		DR.				NOTES
TYPE S ^{VDI} ENERAL NO A B C	MODEL/BERES WATTSCPPER LADM 101 TES. 2000/PANGY SEMIOR LAYOL AWURACTURER-SPECIFIC & WINDFATTURER CONSIGN UNITACTURERS CONSIGN 2000 DRAWINGS FOR LIGHT SIMP DRAWINGS FOR LIGHT SIMP DRAWINGS FOR LIGHT	MAKUFACTURER COOPER HUBBELL LEWTON IT BASED ON WATTETOPPE PACING CRITERIA IED EQUIVALENT FOR SULD WIGB (25 FURTHER DEFIN NG CONTRUCT DEFINCE	0-10V LOW VOLTAGE PUSH BUITTON TYPE DIMMER SWITCH WITH LED STATUS (NOICAT R COVERAGE PATTERNS ADJUST QUANTITIES AND LOCATIONS FOR BOUNAURT MANU STONARE LIFTON IN EDUVAUENT MANUFACTURES AND LOCATIONS FOR BOUNAURT MANU USBIONARE LIFTON IN EDUVAUENT MANUFACTURES AND LOCATIONS FOR BOUNAURT MANU	aaturesiusted Belowfer Uurturesiusted Belowfer			123V	NOTES
TYPE LVD: SCERAL NO A B C	KOBLIGENES WATSCHPER LXOM 101 2019ANCY SENSOR LXVOL WAUVACTURERAPPEOITES UMUVACTURERAPPEOITES UMUVACTURERAPPEOITES UMUVACTURERAPPEOITES SOP DRAWINGS FCK LIGHT SOP DRAWINGS FCK LIGHT SOP DRAWINGS FCK LIGHT	NAMERATURER COOPER HUBBLL LEWTCH TE BASED ON WATTETOPTE PACING CRITERA BED EQUIVALENT FOR SUB WHGB (XS FURTHER DETA) NG COMTULE INVICES IN COMTULE INVICES IN COMTULE INVICES IN COMTULE INVICES IN COMTULE INVICES IN	D-10V LOW VOLLTHEERUSH BUITTON TYPE DIMMER SWITCH WITH LED STATUS (NOICH R COVERAGE PATTERNS ADJUST QUANTITIES AND LOCATIONS FOR BOUNAURT MANU SUSSION ARE LIFTER IN EDUIVAURT MANUFACTURE FOLLUNG IN SIGLED LL. DIMING IN SUSSION ARE LIFTER IN EDUIVAURT MANUFACTURE FOLLUNG IN SIGLED LL. DIMING IN SUSSION ARE LIFTER IN EDUIVAURT CANTROL SYSTEM AND LIGUIDE FRAZECT EDUIST, FROME A SCALED EQUIPART PLANFOR MARCH OF EGUIVARIA INGLIZE FRAZECT EDUIST, FROME A SCALED EQUIPART PLANFOR MARCH OF EGUIVARIA	Racturers uisted Belgy/Fer Heattale provide product Specific Lightike Control Within Project Space			123V	NOTES
TYPE SV2) DIERAL NO A B C	KODELPRENES WATTSOMPER LKOM 161 JCUMANCY SENSOR LAYOU ANUMATTURERAPEORICS UTENETS AND SENSOR LOUSDE UTENETS AND SENSOR SENSOR SENSOR JLGS DISCHARTSON	INANIFACTURER COOPER HUBBELL LEWTON IT BASED ON WATTSTOPPE PAGING CRITERA BE EDUNALEN FOR SUD WIGB JAS FURTIER DESN COMTOL: DRIVER THE S. ALSO, AT ENGINEERT R D'AWAINSS BHALL INCLUDE D'AWAINSS BHALL INCLUDE	0-10V LOW VOLTAGE PUSH BUITTON TYPE DIMMER SWITCH WITH LED STATUS (NOICAT R COVERAGE PATTERNS ADJUST QUANTITIES AND LOCATIONS FOR BOUNAURT MANU STORMARE LIFTON IN EDUVAUENT MANUFACTURES AND LOCATIONS FOR BOUNAURT MANU USBIONARE LIFTON IN EDUVAUENT MANUFACTURES AND LOCATIONS FOR BOUNAURT MANU STORMARE LIFTON IN EDUVAUENT MANUFACTURES AND LOCATIONS FOR BOUNAURT MANU	actures usted Elonyfor UBATTALE PROVDE FRODUCT SFECHC LIMTING CONTROL WITHIN FROLEDT SFACE			123V	NOTES
INITE S NERAL NO A B C	KODELINERTES WATTECHTER LUDM 101 TEL SCULPANCY SENSIGR LAYOU ANU PATTURER-APPECIFICS UNITA-TETRES AND SHOP IN SUPPARAMOS FOL LIGHT SUPPARTS AND SCHEDUL CONTINUES SENSIGR SHOP I SENSIGR. LASO SICHEDUL SENSIGR. LASO SICHEDUL	NANUFACTURER COOPER HUBBELL LEVITON IT BASED ON WATTETOPTE PACING CRITERA BE BOUNLED FOR SUID WHGB K& FURTHER DESH NG CONTILL DRICLE IN CONTILL DRICLE IN CONTILL DRICLE IN CONTILL DRICLE CONTILL DRICLE CONTINUE TO THE CONTINUE CONTINUE TO THE CONTINUE CONTINUE TO THE CONTINUE CONTINUE TO THE CONTINUE CONTINUE TO THE CONTINUE TO THE CONTINUE TO THE CONTINUE CONTINUE TO THE CONTINUE TO THE CONTIN	G-10V LOW VOLTAGE PUSH BUITTON TYPE DINNER SWITCH WITH LED STATUS INDICAT R COVERAGLE ATTERNS. ADJUST QUANTITIES AND LOCATIONS FOR EQUIVALENT MANUAL USSION ARE LISTED IN EDUIVALENT MANUFACTURER FOLLING IN BCHEDLE. TURING D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN BCHEDLE. TURING D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST RATE AND LO D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN BCHEDLE. TURING D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INGUST ROLLING IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INFORMATION OF EDUINGEN IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INFORMATION OF EDUINGEN IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INFORMATION OF EDUINGEN IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INFORMATION OF EDUINGEN IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS SATERIZLE) FOR INTO IN D D & GENERAL (UTTES OF THIS	actures usted Elonyfor UBATTALE PROVDE FRODUCT SFECHC LIMTING CONTROL WITHIN FROLEDT SFACE			123V	NOTES
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TYPE LVD S ENERAL NO A B C C D C C C C C C C C C C C C C C C C	KODELINERIES WATTSCHER LMDM 101 TRL SCUPARCY SENGIA LAVO, WIUPATURSENGALAVO, WIUPATURSENGALAVO, SCIPARCH AND SENGIA LAVO, SCIPARCH AND SENGIA SENGIA CONTRACTS AND SENGIAL CONTRACTS AND SENGIAL CONTRACTS AND SENGIAL CONTRACTS AND SENGIAL CONTRACTS AND SENGIAL CONTRACTS AND SENGIAL SENGIAL CONTRACTS AND SENGIAL REVISE CONFESCIO POR ALL W. REVISE CONFESCIO POR ALL W.	MANUFACTURES COOPER HUBBELL LENTON TE ASED ON WATTETOPER ADDIS CONTROL HUBBELL LENTON ED EQUIVALENT FOR SUD ADDIS CONTROL HUBBERG ALSO, AT ENGINEERS R SAUSO, AT ENGINEERS R SAU	C-10V LOW VOLTAGE PUSH BUTTON TYPE DINNER SWITCHWITH LED STATUS INDICAT R COVERAGLE ATTERNS. ADJUST QUANTITIES AND COATIONS FOR EQUIVALENT MANUA R COVERAGLE ATTERNS. ADJUST QUANTITIES AND COATIONS FOR EQUIVALENT MANUA DISCONTRE LISTED IN EDUIVALENT MANUFACTURER COLUMN IN GCIEDLLS. TURING IN DISCONTRE LISTED IN EDUIVALENT MANUFACTURER COLUMN IN GCIEDLLS. TURING IN DISCONTRE LISTED IN EDUIVALENT MANUFACTURER COLUMN IN GCIEDLES. TURING IN DISCONTRE LISTED IN EDUIVALENT MANUFACTURER COLUMN IN GCIEDLES. TURING IN DISCONTRE LISTED IN EDUIVALENT MANUFACTURER COLUMN IN GLIENT DISCONTRENDE POLISIES AL SUBITIVE CONTROL DISTENSIONAL INCLUTE PAGE 11 VARIANTING FUNDE SCHUMENT PLANTOR REACTOR DE DISCOSS INCLUDING BUT NOT I UFACTURER. LISTITIS CONTROL DENCES WITH THE ARCHITECT IN JUSTICE PLANTS IN PLANTOR CRADING IN DISCONSE DIST.	actures usted Elonyfor UBATTALE PROVDE FRODUCT SFECHC LIMTING CONTROL WITHIN FROLEDT SFACE			123V	
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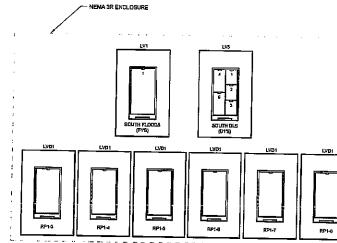


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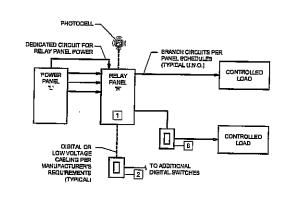
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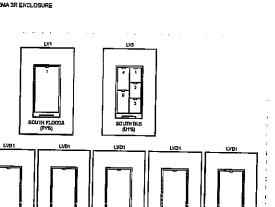
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- FORSELECTED DAVLIGHT BENROR, CONFIRM FINAL LOCATION AND INSTALLATION PER MANUFACTUREND RE

- ucation with select tablet tablet for review. Usation with select tablet tablet tablet tablet. Usation with select tablet becompared with the connected lighting control device (ie. Cocupancy Bengdr, relay Panel, etc.) Usati luga, estimation transmession arginger marked value at hongontal tablet. Tablet device tablets do does not necessaries at the review treat. Future staticge marked value at hongontal tablet and controller to adjust light donor and marked a
- EFORM. 5 ENCREMENTALIDEE NOT KIVE LITEASONG TECHNOLOOF LENSING. THEIGHTRE, THEY MAY SUBJIT ALTERNATE TECHNOLOGY SENGORS THAT CAN NOET EQUIVALEN CSIENALE NO FERTIORANDE OF LITEASONG TECHNOLOOF LENSING. THEIGHTRE, THEY MAY SUBJIT ALTERNATE TECHNOLOGY SENGORS 6. RESIELES CONSINTELE FOREE PACIETOR SWITCHING BALLASTE AND DRIVERS



AMPHITHEATER LIGHTING CONTROL





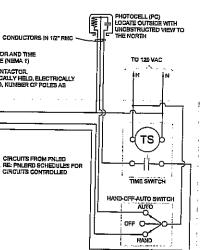
LIGHTING CONTROL DIAGRAM KEYED NOTES: 
 STAND-ALONE LIGHTING CONTROL RELAY PANEL INFORMATION:

 a.
 MANUFACTURER2: LOBD BLUE POX LT, GREENGATE LITEGEPER, WATTSTOPPER SERIES

 b.
 ENCLOSURE SURFACE MOUNT, NEMA 3R

 c.
 INTEGRAL TIME CLOCK CREFER TO CASS AND RELAY PANEL SCHEDULES ON THIS SHEET FOR TIME CLOCK AND FELAY PROGRAMMING

(**4**) NO SCALE



# EXTERIOR LIGHTING CONTROL FOR ONE

### LIGHTING CONTROL DIAGRAM GENERAL NOTES:

LIGHTING CONTROL DIAGRAM IS DIAGRAMMATIC AND REPRESENTS THE GENERAL SCOPE OF WORK AND THE LOCATION OF DEVICES IN RELATION TO BACH OTHER ALLING THE POWER CIRCUIT. ELECTROL CONTRACTOR SHALL COORDINATE WITH SELECTED MANUFACTURER. PROVIDE ALL PARTS AND PIECES REQUIRED FOR A FULLY FUNCTIONAL SYSTEM.

2 LIGHTING CONTROL SYSTEM INCLUDING, BUT NOT LIMITED TO INTEGRAL TIME CLOCK, RELAYS, AND DIGITAL LIGHTING CONTROL SWITCHES. PROVIDE SHOP DRAWINGS FOR APPROVAL PRIOR TO PURCHASE.

3 INTEGRAL TIME CLOCK SHALL BE ASTRONOMIC AND PROGRAMMABLE WITH 365 DAY AND HOLDAY SCHEDIUNG AND 24 HOUR BATTERY BACK-UP, FELAYS SHALL BE MECHANICALLY HELD, SINGLE POLE, 204 RATED, NORMALLY CLOSED CONTRACTS. LIGHTING CONTROL SYSTEM SHALL COMPLY WITH ALL LOCAL AND STATE ENERGY CODES.

4 CIRCUITING SHOWN ON THE PLAN CORRESPONDS TO THE UGHTING GONTROL SCHEME. IF CIRCUITING IS CHANGED IN THE FIELD, ENSURE THAT SYSTEM FROGRAMMING WITH REVISED CIRCUITING MEETS THE ORIGINAL UGHTING CONTROL SCHEME UFDATE RELAY PANEL SCHEDULES IN RECORD OPAVITINGS

COORDINATE WITH OWNER FOR PROGRAMMABLE TIME CLOCK SCHEDULES, PROVIDE THE GENERAL CONTRACTOR WITH OPERATIONS INANULL AFTER 408 IS COMPLETE. A COPY OF THE RECORD DRAWINGS AND RELAY SCHEDULE WITH MY PIELD CONDITION CHANGES IDENTIFIED SHALL BE LIFT IN THE DOOR OF THE PANEL.

2 DIGITAL LIGHTING CONTROL SWITCH. REFER TO DETAILS 3 ON THIS SHEET AND TO AMPITMENTER LIGHTING PLAN FOR ADDITIONAL INFORMATION

### RELAY PANEL DETAIL

CONFLUENCE

LANDSCAPE ARCHITECT CONFLUENCE 415 DELAWARE, SUITE 400 KANSAB CITY, MISSOURI 64105 PH: 816.531.7227 FAX: 816.531.7229

ARCHITECT SFS ARCHITECTS 2100 CENTRAL STREET, STE 31 KANSAS CITY, MISSOURI 64108 PH: 616.541.2288

CIVIL ENGINEER WILSON ENGINEERING 800 E 101ST TER, STE 200 KANSAS CITY, MD 64131 PH: 816.701.3100

MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MO 64108 PH: 816.6538718

> CITY OF RAYMORE PARK AND RECREATION RAYMORE/ MISSOURI PARK Ш RIDGI HAWK

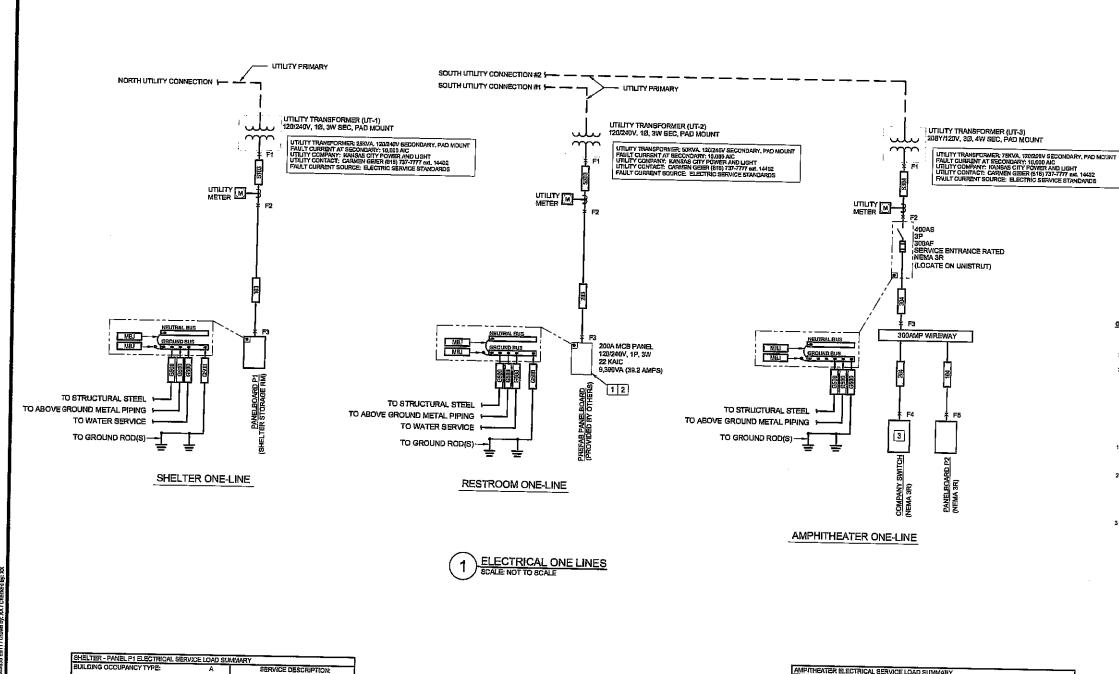


ANDREA C. MULVANY LICENSE # PE-2013/039892 RIGHT - 2018 BY CM 1017-04 2018 2018 BT LAUFERTAWING IS FROMDED AS ESERVED, THIS DRAWING IS FROMDED AS DF SERVICE BY CONFLUENCE, AND IS NIT THIS FROMELY OLLY, ANY REFRONCTION THIS FROMELY OLLY, ANY REFRONCTION THIS FROMELY OLLY ANY REFRONCTION TO THE REPORT OF THE REFORMANCE OF THE REFORMANCE TO THE REPORT OF THE REFORMANCE OF THE

> **ELECTRICAL** DETAILS

> > E411

CONFLUENCE PROJECT NO: 16081KC



	SER	240Y/120V, 1PH	TON:
LOAD DESCRIPTION	Connected KVA	Demand FACTOR	Demand KVA
HVAC - SUMMER	0.00	100%	0.001
HVAC-WINTER	0.00		0.00
LIGHTING (PER NEC-220)	0,10		0.13
RECEPTACLES	4.02	100%:50%	4.02
EXTERIOR LIGHTING	1.11	125%	1.39
TOTAL LOAD	5.23	KVA I	5.54
TOTAL AMPACITY	21.80	AMPS	23.06
SERVICE AMPACITY	100	AMPS	100.00
SPARE CAPACITY		AMPS	77

5

AMPITHEATER ELECTRICAL SERVICE LOA	D SUMMA	RY		
BUILDING OCCUPANCY TYPE	A	SEF	VICE DESCRIPT 208Y/120V, 3PH	TON:
LOAD DESCRIPTION		Connected KVA	Demand FACTOR	Damand KVA
HVAC - SUMMER		0.00	100%	0.00
HVAC - WINTER		0,00	100%	0.00
LIGHTING (PER NEC-220)		0.00	125%	0,00
RECEPTACLES		1.98	100%:50%	1.98
EXTERIOR LIGHTING		1.95	125%	1,69
TOTAL LOAD		57.33	KVA	57.67
TOTAL AMPACITY		169.13	AMPS	
SERVICE AMPACITY		300	AMPS	160.07
SPARE CAPACITY				300.00
			AMPS	140

#### CIRCUIT SCHEDULE:

CIRCUIT SCHEDULE: H. CONTRUCTION ADDA ON PROPERTY FOR THE DESIGN IN THE ADDA ON PROPERTY ADDA ON THE DESIGN OF THE DESIGN IN THE DESIGN OF THE DESIGN OF THE DESIGN OF THE DESIGN IN THE DESIGN OF THE DESIGN OF THE DESIGN OF THE DESIGN IN THE DESIGN OF THE DESIGN OF THE DESIGN OF THE DESIGN TO NEED THE DESIGN OF THE DESIGN OF THE DESIGN OF THE DESIGN TO NEED THE DESIGN OF THE DESIGN OF

_10	100A, (3)#3, (1)#8G, 1-1/4" C
104	100A. (4)#3. (1]#8G, 1-1#* C
203	200A, (3)#3/0, (1)#8G, 2* C
214	200A. (4)#3/8, (1)#5G, 2* C
_304	390A. (4)-350kcmill. (1第4G, 3° C
510	100A, (3)#3, 1-1/4" C
8703	200A. (3)//3/0, 2" C
5304	300A, (4)-350kamil, (1)#4G, 3" C
GIOD	GND, #8 COPPER GROUND, 3/4" C
<u>ESU</u>	GND, #1/J COPPER GROUND, 1" C

#### GENERAL ONE-LINE NOTES:

- 1. REFER TO SHEET EMON FOR GENERAL ELECTRICAL NOTES.
- 2. REFER TO SHEET EQ91 FOR ADDITIONAL INFORMATION.
- 3. ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR UTTLTY FRIMARY CONDUINE WITHIN PROJECT SITE BOUNDARIES. FIELD COORDINATE EXACT LOCATIONS WITH KOPL FRIGA TO CONSTRUCTION.

#### ONE-LINE PLAN NOTES

- PROVIDE ELECTRICAL CONNECTION TO FREFABRICATED RESTROOM. PROVIDE ALL ELECTRICAL CONNECTIONS REQUIRED PER MANUFACTURER REQUIREMENTS.
- PROVIDE CONTACTOR WITH INTEGRAL TIMECLOCK, AND PROTOCELL, FOR PARKING LOT LIGHTING CONTROL. LOCATE PROTOCELL ON NORTH STIDE OF BUILDING, LOCATE NEAR PREFABRICATED PANEL, AND UTILLE SPARE 20A 14POLE GROUIT N PREFABRICATED PANEL, REFER TO DETAIL 2 SHEET 5411
- 3 200A COMPANY SWITCH, 120208V, 3-PHASE, 4-WIRE, PLUS GROUND, NEMA 3R RATED, ERK AIC RATING, ACCOMMODATES BIKING LEVOLE CAN STULE CONNECTORS AND UP TO WHO CLASS IC CARLING LISING STUREMI TEXMINALS, SAFET INTERLOCK COMPONENT TO DISCONNECT POWER WHEN CABINET DOOR IS CREMED

CONFLUENCE

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MEP ENGINEER HENDERSON ENGINEERING 1801 MAIN, STE 300 KANSAS CITY, MO 64108 PH: 816.6636718





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### NORTH UTILITY CONNECTION

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The following calculations are based on the "Point" method ware?     Met 4/1-47, Met 4/1		es are for calculation purposes only and one							iy nera costalo	on the testal	2 19 2 5981	ge of test of	. Greater ci	ilcui distance									
IP = Primary bandt cycall cyrons         VP = Primary bandt cyrons          Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons         Primary bandt cyrons </th <th></th> <th>150 - , - skon current at fault point (</th> <th>יאסא-יפייזיים איז אוליים איז אוליים</th> <th>mellibö wher if(**F)</th> <th>re</th> <th></th> <th></th> <th>C⊭E ¦_{eis}≃ <u>2⊮L∋ter</u></th> <th></th> <th></th> <th>, ₁₀, -</th> <th>100,000 x Ki <u>Iffiada): Ve</u></th> <th>VA 1.647</th> <th>Ē</th> <th>18,</th> <th>= <u>Vis His ዙ.</u> 伏</th> <th>-</th> <th></th> <th>νl</th> <th>95VDs (( OLTAGE )</th> <th>(R.) (15)(11(1)) DROF (15)</th> <th></th> <th></th>		150 - , - skon current at fault point (	יאסא-יפייזיים איז אוליים איז אוליים	mellibö wher if(**F)	re			C⊭E ¦ _{eis} ≃ <u>2⊮L∋ter</u>			, ₁₀ , -	100,000 x Ki <u>Iffiada): Ve</u>	VA 1.647	Ē	18,	= <u>Vis His ዙ.</u> 伏	-		νl	95VDs (( OLTAGE )	(R.) (15)(11(1)) DROF (15)		
Point Bus/Feeder Zostangloor. Feeder Feeder Dostang of Provide Set		Vp = Reimany voltage 15: Successing strong cut ourrent Vor Secondary voltage Vor Secondary voltage Vor Secondary voltage Vor Secondary voltage La Car (C Podder form Succement toble vo Fedder Types =	nere 'C' = 1 in	pedance par	incar icos		Itanstum	19. [°]												961 T	R= 7681814	inse in ohme	DELL
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(Model Controller State						2. 1 60dsn 5894	HIT28	SE & 1189223 5:25		s dinte	(臣)	14) F4	Cto: 101;	(+abecals+)	( iRi	ili	(Redians)	Type					1
s 1004 DISC 7 1 10000 Fat 1 CU the two at the state of th		lotor Contribution			The conne	cted futi (a sc	trigior ar	ROS (Instates complet	ante on the	Circle In									 	1111 1 3 70		ge seinn burte ist - f	<u> </u>
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### SOUTH UTILITY CONNECTION #4

The failure to the state of the					in a change of tobe of greeken chorn distance		
The following associations we based on the Prof $SC_{ij} = 1SC_{ij} \times R_{ij}$ $SC_{ij} = contracticution and local point ( SC_{ij} = contracticution at four point ( SC_{ij} = contracticution)$	n-o⊶Pontaî na∌ba po watere Mi= twang	Febüer Febüer	1-10 = <u>1-732 x 1 - 947</u> 07 = 1-10 = <u>2-41 2 - 167</u> 07 = E	NFMR VFMR	(122.) = <u>ИРыстантись (172.) 462</u> 130: 6602 ж.164 110. = <u>ИРыстантись (170.)</u> 130: 6602 ж.164	ές _{επ} α <u>ταν μαγμ</u> αγ	voltase drade (ag) SVD= (ag) observatios(ag), − ) a sar (artistic)(a) lassi (aTS, - E voltase drade (ag) SVD= (ag) observatios(ag) − ) a sinterucos(ag)(a) (a) (ag) (a) - E
P = Printur; Shoth Groek burrem vf = Printary vorlage (SE Secondan en and visuit cusrem vf = Secondan voltige L = Length of circuit C = 'O' Factor ittom Subsymbol table where Peedel Typer = MM-Hon Magnetic Conduct V - Megnetic Conduct		ing, T/ - Transfor	°r∉				ಆಂ⊡್ಟಿಟ್ ಡುಂಗಬಡಿಂದ ಇಂತಿಸಿತ್ವರ ದೇವು ಗುಂಗಾ ಕರ್ಮಕರ್ ಕ್ರಿಮಿಸಿ ಕರ್ಮಕರ ಹೆಸ್ ಕರಿಯಾಗಿದ್ದ ಬೇ ರಾಗ್ಯಾ ಡಿಕ್ಟ್ ಗ ಿ ಕರಿಯಾಗದಕ್ಕೆ ಕ್ರಾಂಗ್ಗನ್ನ ವಿಕ್ಕೇರಿ
5	at 25 Strates	Teens Quanti	<u> </u>	r 8.0945 -C 1	L-1 Celuir LCBC Decur Loog DEge Length Ford Decur Loog TE L Ford Photos Photos	Contention	Depter - Transformer - Depter - Depter

### SOUTH UTILITY CONNECTION #2

# Short-Circuit and Voltage Drop Calculations

	héés élé far calculation purpáses ord, and shali ní	a of nafogo, coupaciol give out the	ing - Contemp and n	ouk Engeneer ofen, fie	ic condition that (eaching)	n e change of tidte of checker	ជាសារ ជាម្នាំងចុះ				
ويديا كميسيكم كالمتحاط ليقوه فأحقاظ	The following calculations are based on the ‡o ISC (= tSC) (> Mi ISC (= tSC) (> Mi ISC (= sTBC) (> tauti ourrent of fault boint ( ISC (= shart arcuit current of fault boint ()	มกระบุ:-Pointi สาสมหลวงศาสรร bl= 1/t1+5,	Feeder. inst	= <u>1.720 ты тыс</u> С: £ = <u>2 л1 ≠ыс</u> 07 £	ntar Vene	( ₁₂ = <u>Шинай орн 173 г 1</u> 106.000 жил ( ₁₂ = <u>Пакалина 19</u> 106.000 жила		<u>312 : 307 8≓ .</u> ≫ <u>.</u>		(D),TAGE DROF (52) %//P= (R), coelaticos(d) POLTAGE DROF (32) %//P= (R),toesarcos(d)	
	<ul> <li>IP = Pompay shan Groun current</li> <li>Vp = Pinnary votage</li> <li>Vs = Secondary Notage</li> <li>Vs = Secondary Votage</li> <li>L = Length of circuit</li> <li>C = 'C Factor from Sessman fable where</li> <li>Feeder Types =</li> <li>VM - Non Magnebic Conduit U - Magnebic Conduit</li> </ul>									SVD GUM+ Currunalive R+ resisione i b= readances i	n ohms per 🗸
Faun Poual (F#)	Bus/Feeder Description (i	ource Source Source Fault Phase Isc. Concurt	Feeder Quantity of Para	La: Sete and Busi	onductar Buswa; C V C Value Value	L-L Circui Load billigs Langth Pawer	Circuit Load (Amperage) Resistance	Conductor		Transformer	
	Utbly Service Point Notor Constitution	10.000 at the second	Phase & Phase & I lary of the using transform of full (oad coolor arous (	ner		(E) (L) Factor (pl)	(Amperage) (R)	Reactance Arccos (pl) (X) (Radians)	Type Degree EVA		Tap f Setuaç

· · · · · · · · · · · · · · · · · · ·	Conerbution 10.0	D at the secondary of the value transformer Rice KVA Xmr Z Xmv Z Vokage Setun	£
2 300A.	ISC 1 2 1000	The connected full load motor amos (includes compressors) on the system Source isc - 6 (MS CU 2) Seles of Tam Tawa 2 - 2003	iz Motor (
	NREWAY 2 3 5900 NY SWITCH 2 3 5900	Na CU 21 Setter of 140 44/06 1 34223 - 202 156 0.9 320 0.900042 0.451027	1 0 44
	EATER PANEL 3 6832	ANI CU 21 Selisi of 130 ANG 13023 - 206 5 0.9 400 000077 0080042 0.451027 1	0.01
		148 CL 21 Setts) of 130 Avid 13923 - 206 5 0.9 160 1 000077 0.000042 0.451027	0.01

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					leulatans (113 16, 243(128),	
ad Deng	r 	la .	Fault Guntum (affilier)	vonane Drop Storich	Carrelisaye Voltage Dibe IFavO;	F&101 F&101 (*#)
- <u>65  </u>	lano: Cen	ikaldada =	10032			<u> </u>
	_0.087 0.628	<u>0.95</u> 0.61	9263 8520	-0.25% -0.41%	- <u>0</u> 059. -0.4692	54.54

#19-1 @# 114 5 73, -E

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N Fault Point 4 to Pault Foint #

		Syst	em Vosage	2089/1204	5 anase
ſ	N	Fault Current (ampa)	Vottage Drop (%%/D)	Cumulative Vellage Drop (SVD)	Feat Point (F#;
eter Controutor. = 10000					
1,440	0,69	6903	1.75%	1.75%	2
0.010	ú.95	6823	-0.06%	-1 815	2
0 010	0.92	6764	-0.03%	.84	6
0010	<u>0</u> .99	6764	-0.03%	1.84%	5

### CONFLUENCE

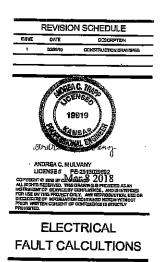
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CONFLUENCE PROJECT NO: 16081KC E512